

# Centreville Wastewater Treatment Plant ENR Upgrade and Expansion Town of Centreville

Centreville, MD

May 2024

FINAL

11/11

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Preliminary Engineering Report



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# 1 Executive Summary

The Town of Centreville Wastewater Treatment Plant (WWTP) is owned and operated by the Town of Centreville and treats flows from the entire town. The Centreville WWTP was most recently upgraded and expanded in 2003, including an upgrade of the facility to achieve biological nutrient removal (BNR) levels of treatment. The nutrient removal process at the Centreville WWTP consists of a two-tank sequencing batch reactor (SBR) with chemical addition for phosphorous precipitation and cloth media filtration. The WWTP is treating wastewater as designed and is meeting all the NPDES permit limits.

The facility has a permitted treatment capacity of 0.542 million gallons per day (MGD). The annual average daily flow (AAF) of the plant for the period 2014 through 2022 is 0.40 MGD. The most recent three calendar years, 2020-2022, averaged 0.45 MGD AAF, which is 83% of permitted capacity. The Town anticipates continued growth in the service area and may consider annexation of areas into the service area. Under the plant's current NPDES discharge permits, treated effluent is disposed into Gravel Run, a tributary to Corsica River, during the winter months and sprayed onto off-site irrigation fields during the warmer months.

The overall objective for this project is to expand the liquid and solids treatment and effluent disposal capacity to meet the needs of the anticipated growth, as well as meet enhanced nutrient removal (ENR) levels of treatment and continue compliance with the NPDES discharge permit. In concert with the treatment capacity expansion, the auxiliary systems, including the laboratory, office space, influent screening, and disinfection facilities will be upgraded and modernized. In addition, sludge handling, biosolids treatment and dewatering facilities will be included. A non-potable water system will be added to the facility to allow for the use of treated effluent for on-site uses.

The condition and performance of the existing facilities were evaluated. Almost all the facilities were found to be in good operating condition. The tertiary cloth media filter has been reported as having insufficient hydraulic throughput for wet weather flows. The cloth media was replaced, and the filter has been reviewed by a manufacturer's representative. In the evaluation of treatment alternatives, the cloth media filter would be replaced by a deep bed denitrifying (sand) filter that will both remove particulate as well as remove nitrogen.

This Preliminary Engineering Report (PER) considers three treatment alternatives to expand the design capacity of the WWTP to 1.0 MGD as well as meet ENR treatment levels:

- <u>Alternative 1 SBR</u>: Expand the existing SBR process, followed by tertiary denitrification filters,
- <u>Alternative 2 Conventional Activated Sludge</u>: Replace the existing SBR system with a 5-stage ENR activated sludge process, followed by tertiary filters (with denitrification capability),
- <u>Alternative 3 MBR Activated Sludge</u>: Replace the existing SBR system with a 5-stage ENR Membrane Bioreactor (MBR) activated sludge process.

Each alternative will require modifications to most of the existing treatment facilities, including sludge handling and expansion of the effluent disposal facilities. The expansion of the facilities capacity and upgrades to equipment will require enhancements to control and monitoring systems throughout the plant process areas. The addition of a centralized Plant Control System for monitoring and control will reduce overall operator and facilitate collection of process data. The identification and study of the effluent disposal expansion options will be conducted separately from this PER. **Table 1.1** provides a summary of the proposed modifications for Alternatives 1, 2, and 3. **Figures 1.1, 1.2, and 1.3** show the proposed process flow diagrams for Alternatives 1, 2, and 3, respectively.

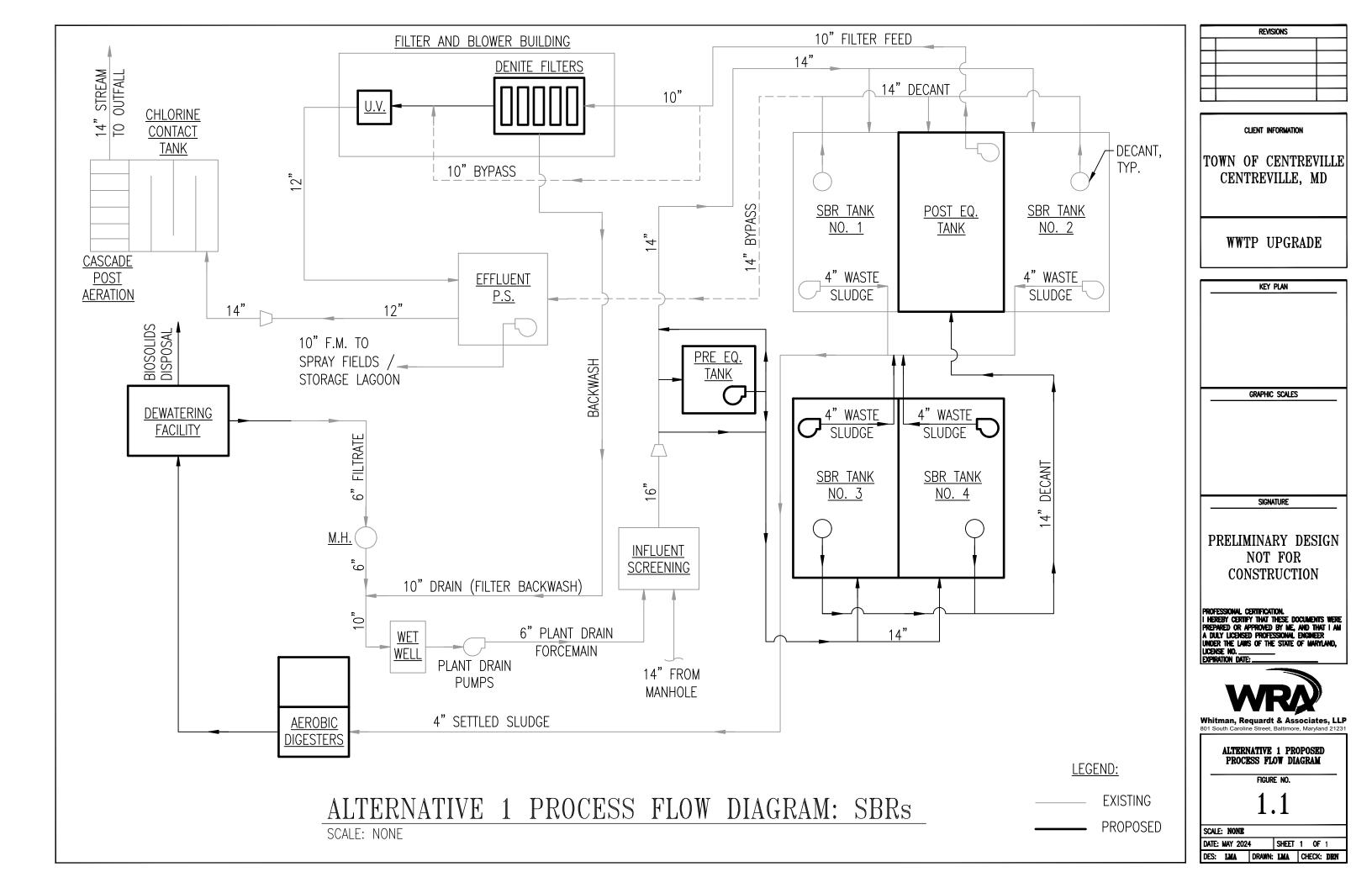
Based on the evaluations in this PER, Alternative 3 – MBR Activated Sludge is recommended for the upgrade and expansion of the Centreville WWTP.

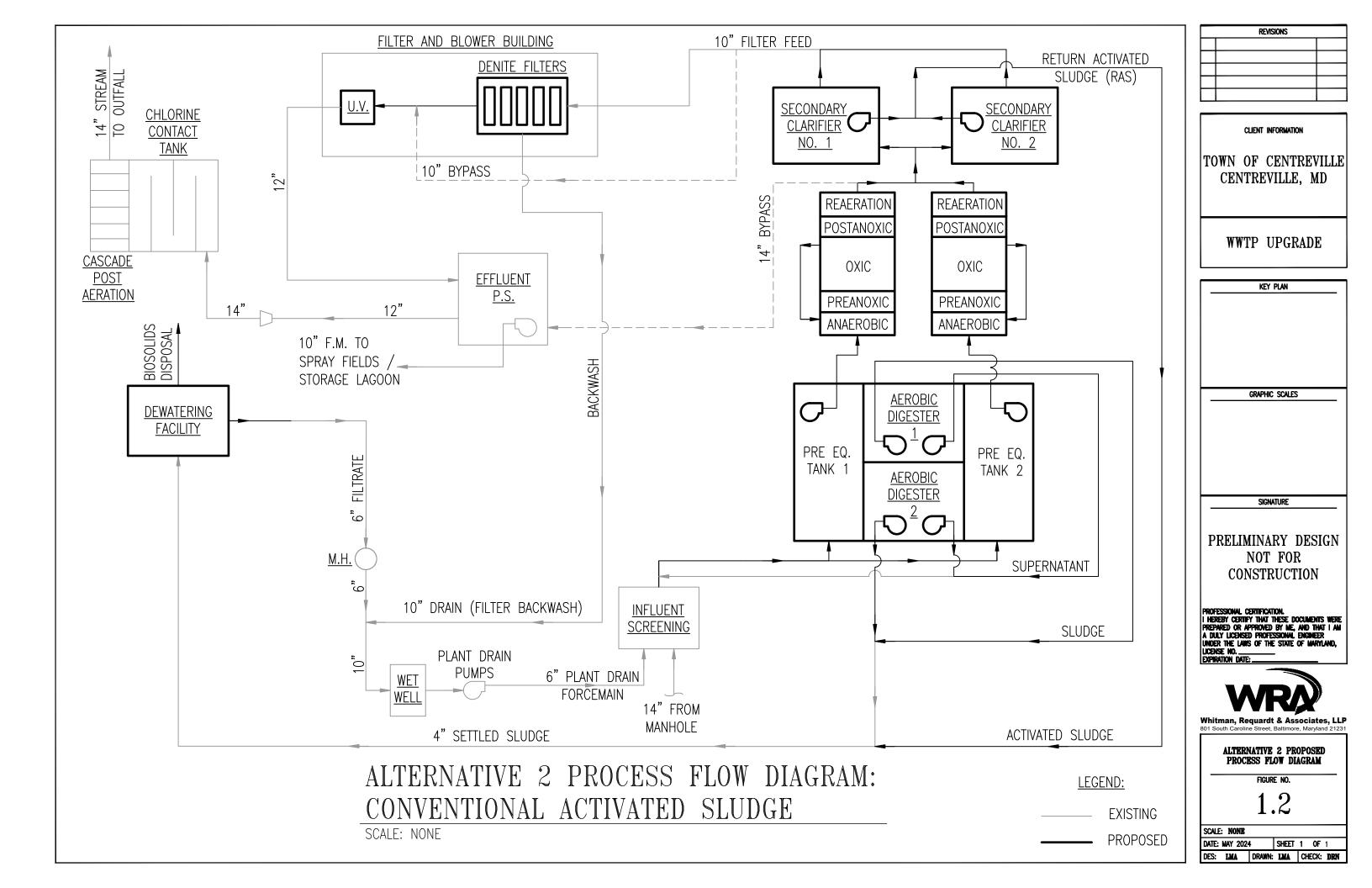


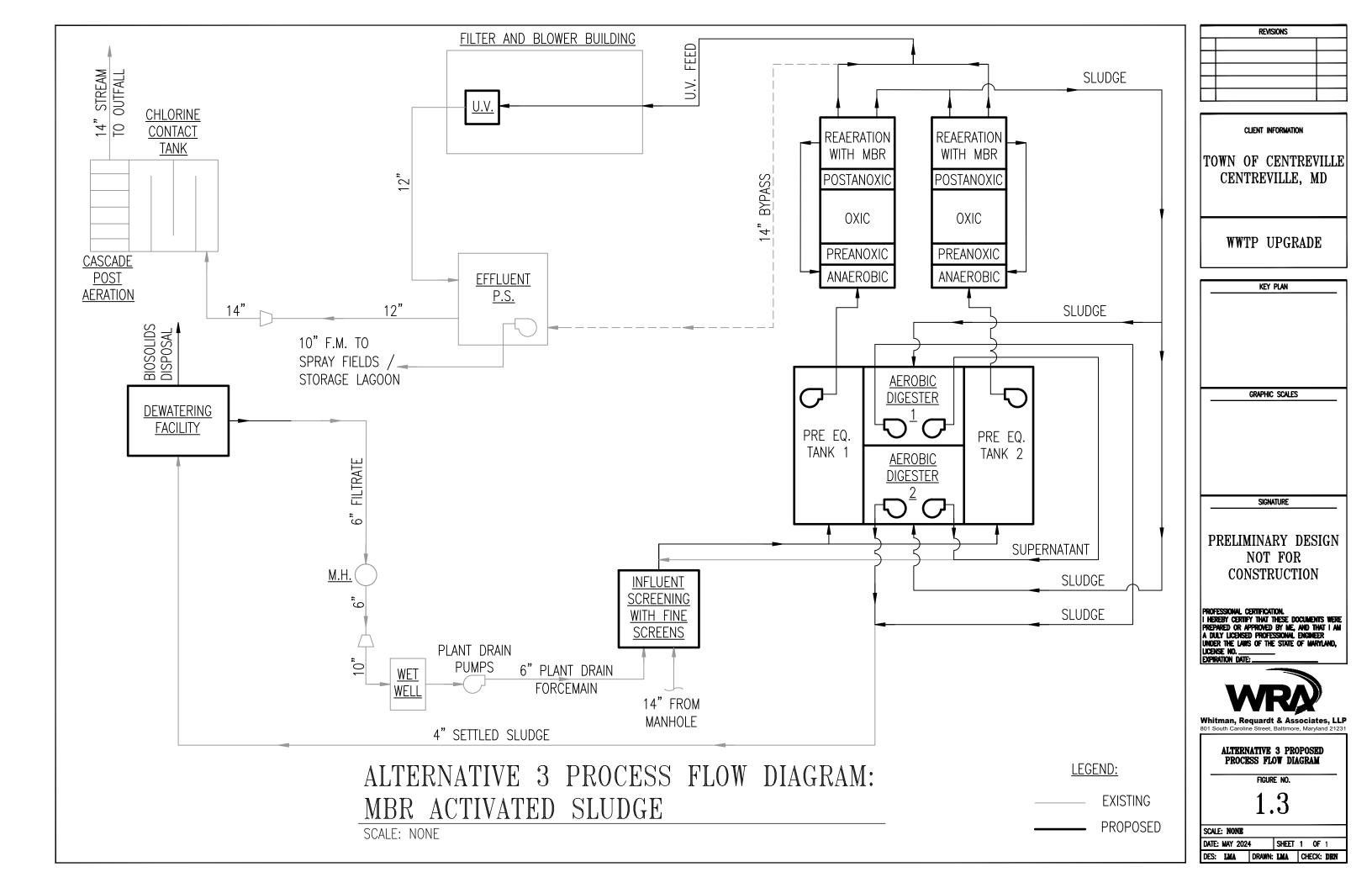
Table 1.1: Upgrade and Expansion of Facilities				
Facility	Alternative 1 – 4 SBRs	Alternative 2 – Conventional Activated Sludge	Alternative 3 – MBR Activated Sludge	
Influent Screening	Replace existing mechanical screen with similar larger model rated at 4.0 MGD (peak hydraulic flow), modify existing concrete channel to accommodate.	Same as Alternative 1	Upgrade screening to mechanically cleaned bar rack followed by two (2) redundant 2-mm opening center feed band screens.	
Influent Flow Equalization Tank	Not required for operation but requested by the Town. Construct a new 500,000-gallon working capacity flow equalization tank with surface aerator/mixers. Submersible pumps will pump flow from the influent flow equalization tank to the SBRs.	Not required for operation but requested by the Town. Convert the existing SBR process tanks to two (2) approximately 500,000-gallon working capacity each flow equalization tanks with surface aerator/mixers. Submersible pumps will pump flow from the influent flow equalization tank to the 5-stage activated sludge basins.	Same as Alternative 2 but required for operation.	
SBR	Install two (2) additional SBR tanks. Install surface mixers, removable fine bubble diffusers and decant arms. Four (4) total 50 HP blowers added to Filter and Blower Building.	Not required. The existing SBR tanks will be converted into influent flow equalization tanks.	Same as Alternative 2.	
Biological Reactors	Expand the existing SBR tanks with additional SBR tankage	Install 2 train, 5-stage conventional activated sludge process. Fine bubble diffusers to incorporate air from proposed high efficiency blowers. Anoxic and swing zones will be agitated with vertical mechanical mixers. Low head propeller pumps for internal recycle will be installed.	Install 2-train, 5-stage conventional activated sludge process with membranes to separate solids from treated effluent. Fine bubble diffusers to incorporate air from proposed high efficiency blowers. Anoxic and swing zones will be agitated with vertical mechanical mixers. Permeate pumps will draw effluent through membranes. Low head propeller pumps for internal recycle and return activated sludge will be installed. Waste sludge pumps will pull mixed liquor from the reactors and discharge into the aerobic digesters. Chemical cleaning facilities will be provided to clean the membranes.	
Secondary Clarifiers	Not required	Two (2) rectangular clarifiers with chain and flight sludge collection and submersible return activated sludge (RAS) pumps installed in a sump. Sludge will be wasted from the RAS forcemain into the aerobic digesters.	Not required	
Chemical Dosing	Provide double contained polyaluminum chloride (PACI) tank located in Filter and Blower Building for chemical phosphorus removal. Provide methanol storage and dosing facility for external carbon addition for enhanced denitrification.	Same as Alternative 1	Same as Alternative 1	
Post Equalization Tank	Construct new 250,000-gallon post equalization tank with surface agitators.	Not required	Not required	



Table 1.1: Upgrade and Expansion of Facilities				
Facility	Alternative 1 – 4 SBRs	Alternative 2 – Conventional Activated Sludge	Alternative 3 – MBR Activated Sludge	
Effluent Filter	Replace the existing cloth media filter with a continuous backwash sand filter in concrete tanks, sized to provide denitrification with the addition of external carbon. House filter mechanical equipment and controls in a new building. Include maintenance space in building.	Same as Alternative 1	Not required	
UV Disinfection	Install two UV disinfection units to replace existing.	Same as Alternative 1	Same as Alternative 1	
Effluent Disposal	<ul> <li>To be further evaluated. Options include:</li> <li>Additional spray irrigation disposal with storage lagoon,</li> <li>Relocation of the existing outfall and expand stream discharge to year round, and</li> <li>Planning for future beneficial water reuse.</li> </ul>	Same as Alternative 1	Same as Alternative 1	
Non-Potable Water System	Install non-potable water system within the Filter and Blower Building that draws from the UV effluent and pumps to an on-site distribution system for applications such as spray water for influent screens, pump seal water, wash down, or yard hydrants throughout the WWTP.	Same as Alternative 1	Same as Alternative 1	
Aerobic Digesters	Install new aerobic digesters with ability to thicken solids and decant liquid back to treatment process. Digester tank will have center wall to allow half of the tank offline. New blowers will supply air.	Retrofit existing SBR post equalization tank and sludge holding tank to two aerobic digesters with ability to thicken solids and decant liquid back to treatment process. Existing process blowers will supply air. Coarse air stainless steel diffusers will be mounted to the bottom slab.	Same as Alternative 2	
Biosolids Dewatering System	Install new biosolids handling building for dewatering process. New covered sludge cake storage area for treated biosolids.	Same as Alternative 1	Same as Alternative 1	
Plant Control System	Provide enhanced process controls with centralized monitoring and control workstation for operator interface. Provide capabilities to provide hub for Town wide SCADA system of utilities.	Same as Alternative 1	Same as Alternative 1	
Laboratory/ Administration Space	Reconfigure the Laboratory/Administration Building to better utilize the space for the laboratory uses. Provide a dedicated space for locker rooms and offices.	Same as Alternative 1	Same as Alternative 1	









# 1.1 Summary of Evaluations

To improve treatment to provide ENR level effluent water quality and to expand the treatment capacity for current and projected influent wastewater flows, three (3) alternatives were developed and evaluated. A "do nothing" alternative is not viable as the current average influent has exceeded 80% of the design treatment capacity and the effluent water quality has occasionally been adversely affected. Additionally, influent flows are projected to continue to increase in the future.

Preliminary sizing of the biological treatment process for each alternative to achieve the required effluent water quality was completed using the BioWin® process simulator (by EnviroSim).

Evaluation and comparison of each alternative is based on life cycle cost and non-monetary criteria:

- Life Cycle Cost Analysis (capital and O&M costs)
- Non-Monetary Comparison
- Energy and Water Efficiency
- Environmental Impacts

The capital costs of the three alternatives are within 8% from the least expensive Alternative 1 – SBR (\$33.0 million), and the most expensive Alternative 2 – Conventional Activated Sludge (\$35.5 million). The O&M costs were based on an assumed 20-year project life and resulted in life cycle costs of the three alternatives within 8% of each other, ranging from \$47.9 million to \$51.9 million.

Given the complexity of each alternative and the variability introduced in projecting operating and maintenance costs for a 20 year project life, the costs of the three alternatives are similar.

The non-monetary comparison indicated that Alternative 2 -Conventional Activated Sludge and Alternative 3 -MBR Activated Sludge were similar and both preferred to Alternative 1 -SBR. The ability to evolve with future regulations and technologies, and the use of the available space were key advantages for Alternatives 2 and 3, with Alternative 3 scoring higher than Alternative 2.

The treated effluent water quality of all three alternatives will be sufficient to meet off-site Class III and IV reclaimed water requirements for future consideration.

Given the smaller footprint of Alternative 3 – MBR Activated Sludge compared to the other two alternatives, the impact on the environment will be reduced. Alternative 3 will have greater flexibility to avoid the environmentally sensitive areas of the available site and have a reduced impact overall.

Alternative 3 – MBR Activated Sludge uses a permeable membrane to separate solids from the treated effluent compared to the other two alternatives using conventional sand media filter, and will produce the highest effluent quality in terms of suspended solids and turbidity.

Based on the evaluations, Alternative 3 – MBR Activated Sludge is recommended for the upgrade and expansion of the Centreville WWTP.

#### 1.2 Summary of Improvements

A site plan of the locations of the facilities that will be affected by the ENR upgrade and expansion, and approximate location of proposed facilities for the recommended Alternative 3 – MBR is shown in **Figure 1.4**.





#### Figure 1.4: Site Plan of ENR Upgrades and Expansion – Alternative 3 MBR Activated Sludge

The key scope items and rough order of magnitude (ROM) construction cost estimate for each improvement is summarized in **Table 1.2**. Given the conceptual design stage, a minus 20 percent and a plus 50 percent cost contingency are added to the estimate. Additional cost breakdown for Alternative 3 – MBR Activated Sludge is included in **Appendix A**.



Table 1.2: Preliminary Construction Cost Estimate – Alternative 3 (MBR Activated Sludge)				
Item No.	Category	Cost		
1	Interior Demolition (Lab, Control, and Filter and Blower Buildings)	\$95,000		
2	Influent Screening Expansion	\$825,000		
3	Converting Influent Flow Equalization Tanks, Aerated, with Pumping	\$2,019,000		
4	Methanol Facility	\$618,000		
5	UV Disinfection System	\$642,000		
6	Non-Potable Water System	\$54,000		
7	Dewatering Facility	\$2,413,000		
8	Covered Cake Storage Facility	\$835,000		
9	Lab, Control, and Filter and Blower Buildings Refurbishments	\$617,000		
10	Existing Tank Modifications	\$643,000		
11	Miscellaneous Process Piping and Equipment	\$784,000		
12	MBR Process Building, MBR Equipment and Controls	\$5,789,000		
13	Aerobic Digester Tank and Equipment	\$78,000		
14	Electrical	\$4,169,000		
15	Site Civil, including Yard Piping and Demolition (15% Items 1-12)	\$2,312,000		
16	Site SCADA (5% Items 1-12)	\$771,000		
	Subtotal	\$22,664,000		
	Design Contingency (30% of Subtotal)	\$6,799,000		
	Escalation to December 2026 (4%/year)	\$3,678,000		
	Total	\$33,141,000		
	Total (Low Range -20%)	\$26,513,000		
	Total (High Range +50%)	\$49,712,000		

The design and construction durations for the project were developed and presented in Figure 1.5.

ID	Task Name	Duration	Start	Finish	Q4	2023 Q1		3 04	2024 Q1 Q2	2 Q3	2025	Q2 Q3	2	026	02 Q3 Q4	2027 Q1 Q2
1	Centreville WWTP ENR Upgrade and Expansion	1655 days	Mon 3/20/23	Mon 7/23/29				<u> </u>		<u>2   U3  </u>					<u>22   Q3   Q4</u>	
2	PER Development	255 days	Mon 3/20/23				P		i							
3	Start PER Update	0 days		Mon 3/20/23			3/20									
4	Advertise for M/WBE	4 wks	Mon 3/27/23													
5	Town Assembles	4 wks	Mon 3/27/23													
	Requested Information		101011 37 277 23	111 4/21/23												
6	Develop ENR PER Amendment	2 wks	Mon 4/24/23	Fri 5/5/23			Ĩ									
7	Town Review PER Amendment	2 wks	Mon 5/8/23	Fri 5/19/23			<b>T</b>									
8	Sub Agreements	2 wks	Mon 5/22/23	Fri 6/2/23			l i									
9	Develop Draft PER	4 wks	Mon 6/5/23	Fri 6/30/23												
10	Topo Survey	4 wks	Mon 6/5/23	Fri 6/30/23												
11	Develop Alternatives	16 wks	Mon 3/20/23	Fri 7/7/23												
12	Subconsultant Field Wo	rl8 wks	Mon 5/22/23	Fri 7/14/23												
13	Develop PER	22 wks	Mon 7/17/23	Fri 12/15/23			*		<b>h</b>							
14	Draft PER to Town and MDE	0 days	Mon 12/18/23	Mon 12/18/23					12/18							
15	Review Draft PER	8 wks	Mon 12/18/2	Fri 2/9/24					ι <b>μ</b> η τη							
16	Incorporate Comments	2 wks	Mon 2/12/24	Fri 2/23/24					<b>i</b>							
17	Finalize PER	2 wks	Mon 2/26/24	Fri 3/8/24					T							
18	Develop Design Propos	al 4 wks	Mon 12/18/2	Fri 1/12/24					<b>1</b>							
19	Town Reviews Design Proposal	2 wks	Mon 1/15/24	Fri 1/26/24					1							
20	ENR Upgrade Design	390 days	Mon 1/29/24	Fri 7/25/25					F			1				
21	Project Set Up	2 wks	Mon 1/29/24	Fri 2/9/24					1 A							
22	Design Kick Off w Town	0 days	Mon 2/12/24	Mon 2/12/24					2/12							
23	30% Design	8 wks	Mon 2/12/24	Fri 4/5/24												
24	30% Design Internal QA	2 wks	Mon 4/8/24	Fri 4/19/24					<b>T</b>							
25	30% Design to Town	0 days	Mon 4/22/24	Mon 4/22/24						4/22						
26	Town Reviews 30% Des	ig2 wks	Mon 4/22/24	Fri 5/3/24					Т,							
27	30% Design Comment Review Meeting	0 days	Mon 5/6/24	Mon 5/6/24					•	5/6						
28	60% Design	10 wks	Mon 5/6/24	Fri 7/12/24												
29	60% Design Internal QA	2 wks	Mon 7/15/24	Fri 7/26/24						i,						
30	60% Design to Town	0 days	Mon 7/29/24	Mon 7/29/24						7/2	9					
31	Town Reviews 60% Des	ig2 wks	Mon 7/29/24	Fri 8/9/24						it i						
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Date:	Mon 12/4/23 M	lestone	•	Inactive	Milest	tone	$\diamond$		Manual Si	ummary Rollu	ip	E	xternal Tas	ks		Manu
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Inactive Summary

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Manual Summary

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External Milestone

Q3	Q4	2028 Q1	Q2	Q3	Q4	2029 Q1	Q2	Q3	Q4	
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C	Task Name	Duration	Start	Finish	Q4	2023 Q1	02	Q3	04	2024 Q1	Q2	Q3 Q4	2025	Q2 Q3	20	026	Q2	רז <sup> </sup> כ	2027 24 Q1
32	60% Design Comment Review Meeting	0 days	Mon 8/12/24	Mon 8/12/24			1 02					8/12							<u>!4   Q1</u>
33	Pre-Final Design	10 wks	Mon 8/12/24	Fri 10/18/24															
34	Pre-Final Design Interna QA	l 2 wks	Mon 10/21/24	Fri 11/1/24								1							
35	Pre-Final Design to Towr	n0 days	Mon 11/4/24	Mon 11/4/24								<b>*</b>	11/4						
36	Town Reviews Pre-Final Design	2 wks	Mon 11/4/24	Fri 11/15/24								, it							
37	Pre-Final Design Comment Review	0 days		Mon 11/18/24								•	11/18						
38	Design Review Meeting with MDE	2 wks	Mon 11/18/24	Fri 11/29/24								Ĩ							
39	MDE Review	8 wks	Mon 12/2/24	Fri 1/24/25								ì							
40	Permitting	12 wks	Mon 1/27/25	Fri 4/18/25										∎-j					
41	Develop Bid Ready Documents	4 wks	Mon 4/21/25	Fri 5/16/25										Ľ.					
42	Bid Ready Doc Internal C	22 wks	Mon 5/19/25	Fri 5/30/25										1 A					
43	Bid Ready Docs to Town	0 days	Mon 6/2/25	Mon 6/2/25										<b>6</b> /2					
44	Town Finalizes Funding	8 wks	Mon 6/2/25	Fri 7/25/25										tin h					
45	ENR Upgrade and Expansion Bidding	130 days	Mon 7/28/25	Fri 1/23/26										r	1				
46	Town Prepares for Advertisement	4 wks	Mon 7/28/25	Fri 8/22/25										Ľ.					
47	Advertise	12 wks	Mon 8/25/25	Fri 11/14/25															
48	Open Bids	0 days	Mon 11/17/2	Mon 11/17/2											11,	/17			
49	Bid Review	2 wks	Mon 11/17/2	Fri 11/28/25											Т,				
50	Bid Recommendation to MDE	0 days	Mon 12/1/25	Mon 12/1/25											<b>*</b> 12	2/1			
51	MDE Bid Review	4 wks	Mon 12/1/25	Fri 12/26/25											<b>1</b>				
52	Construction NTP	4 wks	Mon 12/29/2	Fri 1/23/26											Ľ.	1			
53	ENR Upgrade and Exp Construction	910 days	Mon 1/26/26	Mon 7/23/29											1				
54	Issue PO's	2 wks	Mon 1/26/26	Fri 2/6/26											ì	ĥ			
55	Shop Drawing Submittals and Review	16 wks	Mon 2/9/26	Fri 5/29/26												<b>*</b>			
56	Major Equipment Delive	r 36 wks	Mon 6/1/26	Fri 2/5/27													<b>†</b>		<b>I</b>
57	Mobilize Site	0 days	Mon 2/8/27	Mon 2/8/27															•
58	Construction	104 wks	Mon 2/8/27	Fri 2/2/29															L L
59	Substantial Completion	0 days	Mon 2/5/29	Mon 2/5/29															

	Task		Project Summary		Manual Task		Start-only	C	Deadli
Project: Centreville WWTP ENR	Split		Inactive Task		Duration-only		Finish-only	3	Progre
Date: Mon 12/4/23	Milestone	•	Inactive Milestone	$\diamond$	Manual Summary Rollup		External Tasks		Manua
	Summary	I1	Inactive Summary	0	Manual Summary	1	External Milestone	$\diamond$	

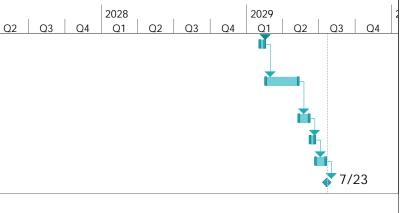
Figure 1.5: Upgrade and Expansion Schedule

	2028			2029			
Q2 Q3 (	24 Q1	Q2 Q3	Q4	Q1	Q2	Q3	Q4
						1	
′8							
				2/	'5		
Deadline	ŧ						
Progress	_		-				
Manual Progress			-				

ID	Task Name	Duration	Start	Finish		2023				2024				2025				2026				2027	
					Q4	Q1	Q2	Q3	Q4	Q1	Q2												
60	Operations Process Training	2 wks	Mon 2/5/29	Fri 2/16/29																			
61	Commissioning and Start Up	12 wks	Mon 2/19/29	Fri 5/11/29																			
62	Process Testing	4 wks	Mon 5/14/29	Fri 6/8/29																			
63	Develop Punchlist	2 wks	Mon 6/11/29	Fri 6/22/29																			
64	Project Closeout	4 wks	Mon 6/25/29	Fri 7/20/29																			
65	Final Completion	0 days	Mon 7/23/29	Mon 7/23/29																			

	Task		Project Summary		Manual Task		Start-only	C	Deadline	+
Project: Centreville WWTP ENR	Split		Inactive Task		Duration-only		Finish-only	3	Progress	
Date: Mon 12/4/23	Milestone	<b>♦</b>	Inactive Milestone	$\diamond$	Manual Summary Rollup		External Tasks		Manual Progress	
	Summary	I1	Inactive Summary	[	Manual Summary	1	External Milestone	$\diamond$		

Figure 1.5: Upgrade and Expansion Schedule





# 2 Background

### 2.1 Introduction

The Town of Centreville, established in 1782, is the county seat of Queen Anne's County and is the County's largest incorporated municipality with an estimated 2020 population of about 4,700 people. The Town, located on Corsica River is situated in the center of Queen Anne's County and is geographically positioned in the middle of Maryland's Eastern Shore.

The Town of Centreville Board of Commissioners owns the Centreville WWTP located at 116 Johnstown Lane, Centreville, MD. The Centreville WWTP has a surface water discharge permit, state number 20-DP-0116 and NPDES discharge permit number MD0020834, and a groundwater discharge permit, state number 20-DP-3323 and NPDES discharge permit number MD3323R05. Each permit allows 0.542 MGD annual average flow to be discharged. The Centreville WWTP has had an average daily flow of 0.40 MGD for the calendar years 2014 through 2022. The Town has experienced steady growth over the past several years and has been approached by multiple developers with plans to develop in the growth areas surrounding the current Town limits.

The Centreville WWTP was originally constructed in 1963, and major Biological Nutrient Removal (BNR) funded upgrades and expansion that was completed in 2005. The 2005 modifications included an upgrade of the facility to achieve BNR levels of treatment included total effluent nitrogen concentration of 5.5 mg-N/L, and total effluent phosphorus concentration of 1.0 mg-P/L. The nutrient removal process at the Centreville WWTP consists of a two-tank SBR with chemical addition for phosphorus precipitation and cloth media filtration, as can be seen in **Figure 2.1**. The facility effluent total nitrogen and total phosphorus monthly average permit concentrations when discharging to surface waters are 5.5 and 1.0 mg/L, respectively.



Figure 2.1: Centreville WWTP SBR

# 2.2 Existing Facilities

The facility provides preliminary treatment, with an activated sludge process for secondary biological nutrient removal, tertiary particulate filtration, followed by effluent disinfection with ultraviolet (UV) light, and final post aeration. Final effluent from the plant can be discharged to the Gravel Run stream December 1 to March 31, and groundwater application via spray irrigation from March 1 through December 15. A 20 million gallon (MG) working volume effluent storage lagoon is located adjacent to the spray irrigation fields. Sludge generated in the treatment process is stored in an aerated storage tank and applied and dried in reed beds. Periodically, the reed beds are removed an disposed of by land application.

Figure 2.2 provides the existing process flow diagram (PFD).

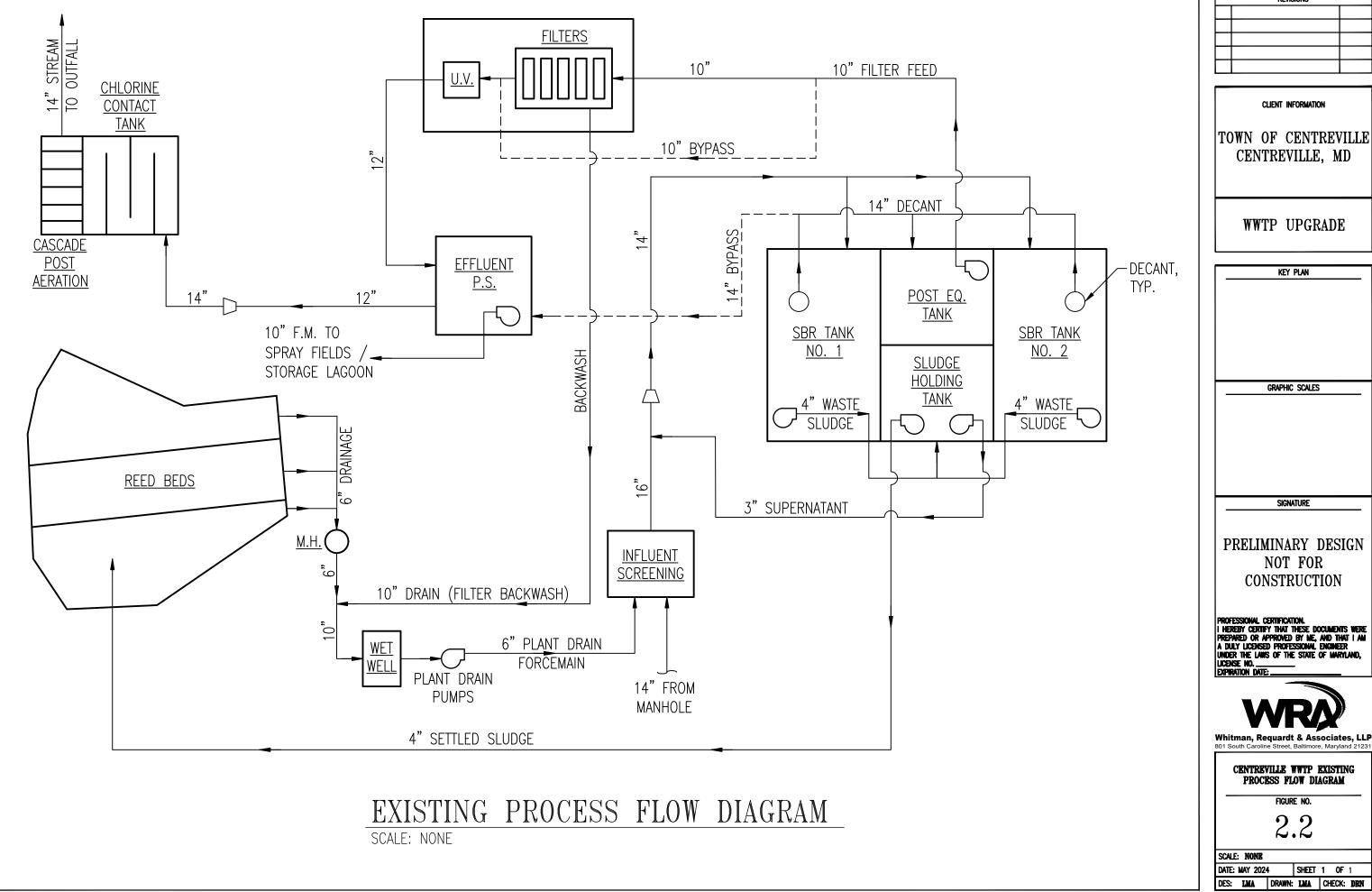




Figure 2.3 provides an overview of the existing treatment process facilities at the Centreville WWTP.

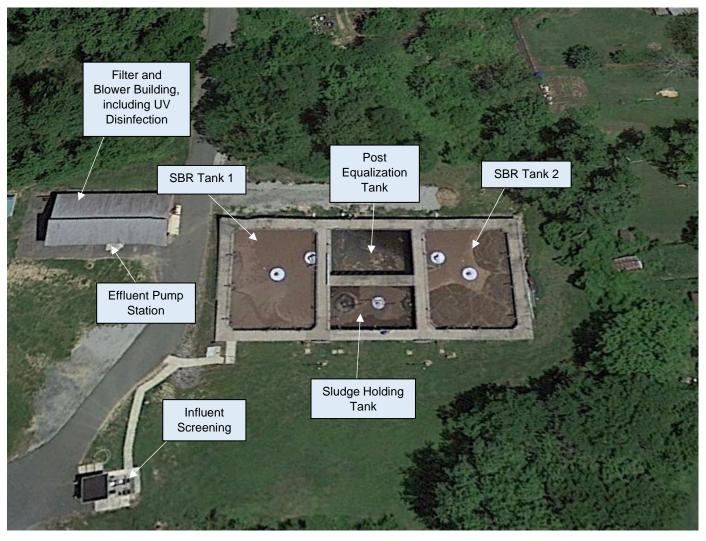


Figure 2.3: Existing Treatment Facilities (Credit: Google Earth)

**Figure 2.4** provides an overview of the existing sludge reed drying beds, chlorine contact tank, Laboratory and Administration Building, and the influent screening.



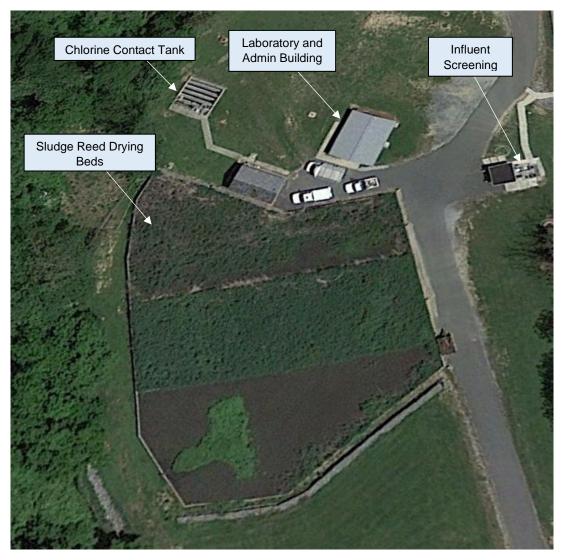


Figure 2.4: Further Existing Facilities (Credit: Google Earth)

### 2.3 Objective

The overall objective for the project is to expand the liquid and solids treatment and treated effluent disposal capacity to both meet the needs of the anticipated growth within the service area, as well as meet ENR level treatment and continue compliance with the NPDES discharge permit(s). In concert with the upgrade, the supporting facilities, including the laboratory, office space, and backup power system, will be upgraded. An on-site non-potable water system will be added to allow for the use of treated effluent for a variety of applications, which will increase the efficiency of the plant.

#### 2.3.1 Treatment Quality Goals

Under the plant's current NPDES discharge permits, treated effluent is disposed into Gravel Run, a tributary to Corsica River, during the period between December 1 through March 31. No stream discharge is permitted from April 1 through November 30 and during this period effluent up to the permitted capacity is disposed of by offsite spray irrigation to ground waters through a separate permit (both the stream permit and the ground water permit are included in **Appendix B**).



With the proposed treatment capacity expansion, the effluent disposal capacity will need to be expanded as well and the Town envisions a combination of spray irrigation and year-round stream discharge. However, year-round stream discharge will require a relocation of the current Gravel Run outfall to a new outfall location further downstream and directly into Corsica River as identified in MDE's Nutrient TMDL for Corsica River. As such, with the capacity expansion, the Town intends to replace the current BNR treatment process with enhanced nutrient removal (ENR) technology to meet TN and TP effluent levels of 3.0 mg/L and 0.3 mg/L, respectively.



# 3 Project Planning

### 3.1 Cost and Effectiveness Analysis

The development of viable treatment alternatives considers and weights monetary and non-monetary factors to deliver a project that meets the treatment objectives, is resilient, preserves natural resources, and is cost effective. The Town of Centreville WWTP ENR Upgrade and Expansion project is similar to other WWTP upgrade projects in Maryland that have implemented enhanced nutrient removal, and the alternatives propose to utilize equipment and treatment processes that have been proven successful and cost effective elsewhere.

Components that do preserve natural resources and are cost effective are incorporated into the project wherever practical. Examples include the inclusion of a treated effluent supplied non-potable water system for onsite process water uses in place of potable water, and the use of slow speed sludge dewatering equipment that draws less power than high rotational speed centrifuges.

Throughout the design process, there will be opportunities to select equipment that provides energy and water efficiency. Examples include:

- 1.) Selecting influent screens that require less wash water,
- 2.) Specifying the latest generation of UV light disinfection equipment,
- 3.) Utilizing high efficiency process blowers,
- 4.) Incorporating process controls and instrumentation that automatically maintains the treatment process and reduces energy use.

The alternatives selected will all meet the treatment goals. The non-monetary evaluation incorporates components for considering the following:

- 1.) Reuse of existing assets,
- 2.) Compatibility with future upgrades to meet ever more stringent regulations,
- 3.) Water reuse and,
- 4.) Long term project maintainability.

The Town is interested in water reuse, including treated effluent water use onsite for processes where currently potable water is used and non-potable water can be used instead, consideration for future Class III or IV reclaimed water use off-site, and planning for potential future direct or indirect potable water reuse.

### 3.2 Environmental Resources

A desktop analysis was conducted to identify environmental resources within the project study area. These resources include the United States Fish and Wildlife Service (USFWS) National Wetland Inventory, the Maryland Department of Natural Resources (MDNR) Wetlands, Maryland Department of the Environment (MDE) Wetlands of State Special Concern (WSSC), Federal Emergency Management Agency (FEMA) Floodplain data, MDE Tier II (High Quality) Waters, Chesapeake Bay Critical Areas (CBCA), Forest Interior Dwelling Species (FIDS) Habitat, MDNR Sensitive Species Project Review Areas (SSPRA), and Maryland Bird Conservation Partnership's bald eagle nest locations.

In addition to the desktop analysis, Coastal Resources, Inc. (CRI) conducted a site visit in July – August 2023 to conduct a waters of the U.S. (including wetlands) delineation and to map forest resources and other habitats. Wetlands were assessed in accordance with the *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Atlantic Gulf and Coastal Plain Region, Version 2.0* (USACE 2010). All identified waters of the U.S., including wetlands, were classified according to *A Classification of Wetland and Deep-Water Habitats in the United States* (USFWS 1979).



Terrestrial habitats within the study area were broadly assessed to document their general physical condition and quality. Forest stands were characterized by successional stage, dominant and codominant species, size class, common understory and herbaceous species, percent canopy closure, prevalence of downed woody debris, presence of invasive species, and basal area. CRI also identified specimen trees with a 30-inch diameter at breast height (DBH) or higher, or that have a diameter which is 75% of the State Champion of that species, including the location, species, size, and health of each specimen tree. A summary of the results of the site visit is included below.

#### 3.2.1 Desktop Analysis

Based on the desktop analysis, several environmental resources are present within the study area, including CBCA, FIDS habitat, MDE Tier II (High Quality) Waters, 100-year floodplains, and wetlands mapped by the NWI and DNR. No WSSC, SPRA, or bald eagle nests are mapped within the project study area. Mapped resources are depicted in **Figure 3.1**. The CBCA is located throughout the project study area and classified as an Intensely Developed Area (IDA). FIDS habitats are mapped in the forest areas surrounding the WWTP property. The entire project study area is within the Gravel Run 1 Tier II (High Quality) catchment. A 100-year floodplain is present on the northern portion of the study area associated with Gravel Run. A palustrine emergent (PEM) wetland was mapped by the NWI on the northern portion of the project area. In addition to NWI, DNR mapped palustrine forested (PFO) and estuarine intertidal emergent (E2EM) wetlands in the forested areas just north of the WWTP site.

#### 3.2.2 Waters of the U.S. (Including Wetlands) Delineation

The results of the wetland delineation indicate that there are three vegetated wetlands and two perennial streams within the project study area (see **Figure 3.1**). Wetland 1 (WL1) is a small, isolated PEM wetland located in drainage swale adjacent to the WWTP entrance road. Wetland 2 is a PFO floodplain depressions on the eastern portion of the project study area associated with Gravel Run (WC1). Wetland 3 is also a PFO floodplain depression associated with an and an unnamed tributary to Gravel Run (WC2). Watercourse 1 (WC1) is Gravel Run, a lower perennial stream that flows northwest along the eastern boundary of the project study area. Watercourse 2 (WC2) is an unnamed lower perennial tributary to Gravel Run on the north-central portion of the project area that receives water from the treatment plant discharge.

#### 3.2.3 Terrestrial Habitat

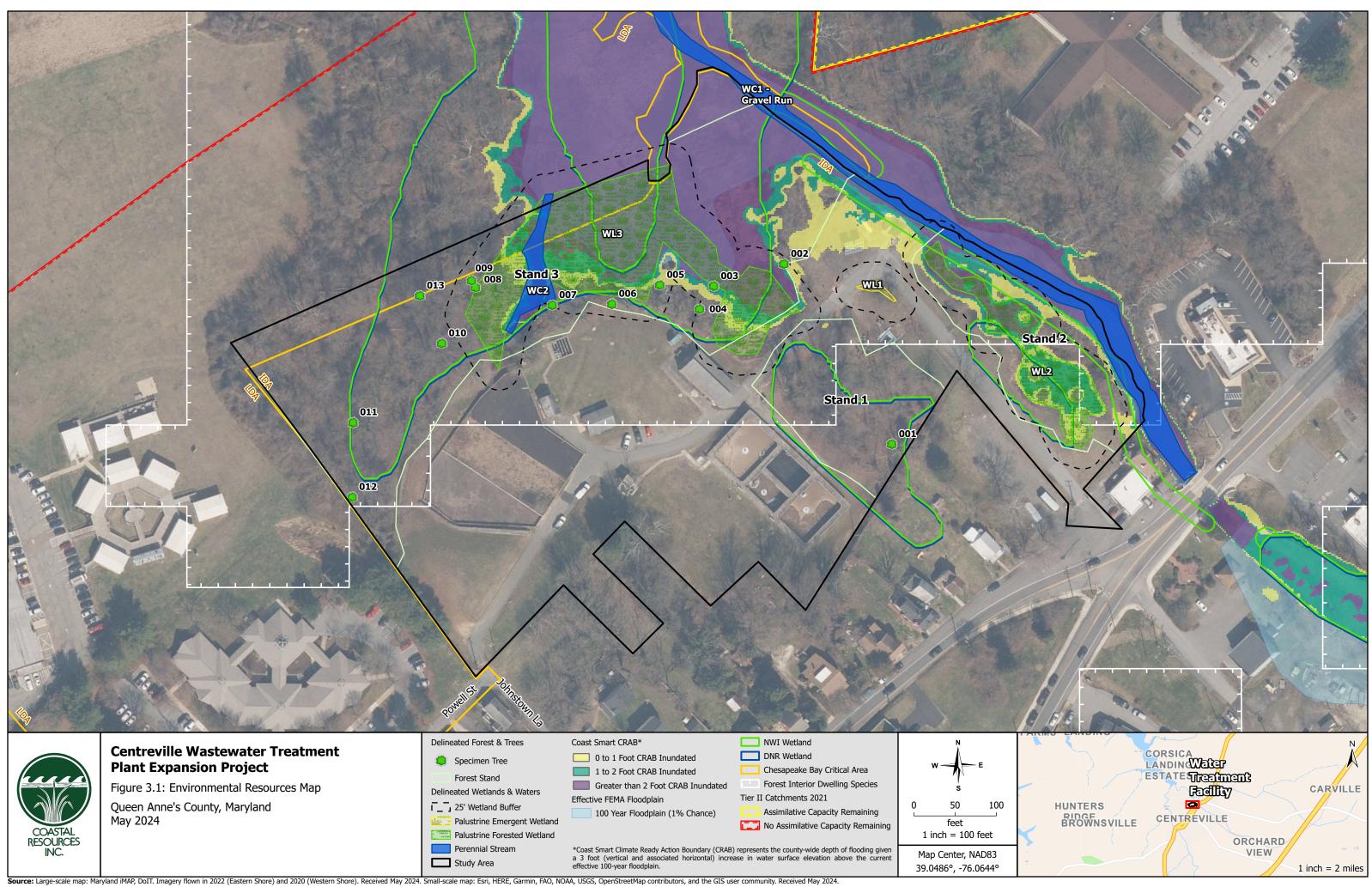
Terrestrial habitats include three forest stands on the northern portion of the study area (see **Figure 3.1**). Stand 1 consists of an early-mid succession ash-leaf maple (*Acer negundo*) – black locust (*Robinia pseudoacacia*) forest with abundant downed woody debris, high invasive plant cover, and fair structure. One specimen tree was identified in Stand 1. This stand was considered poor due to high invasive cover, fair structure, and an abundance of trash/rubble. Stand 2 consists of an early succession black willow (*Salix nigra*) – American elm (*Ulmus americana*) wetland forest. Downed woody debris was abundant in this stand, with moderate invasive plant cover, and poor structure. No specimen trees were identified in Stand 2. This stand was considered fair due to moderate invasive cover, presence of trash, and poor structure. Stand 3 consists of a mid-late succession tulip tree (*Liriodendron tulipifera*) – silver maple (*A. saccharinum*) forest with abundant downed woody debris and a total of 12 specimen trees. Due to the high invasive plant cover, abundance of dead/dying trees, and presence of trash/rubble, this stand was considered poor.

#### 3.2.4 Coast Smart Climate Ready Action Boundary (CS-CRAB)

To determine the potential impact of sea level rise on the project area, the limits of the Coast Smart Climate Ready Action Boundary (CRAB) were reviewed (Source: <u>https://mdfloodmaps.net/CRAB/</u>). The CRAB represents the county-wide depth of flooding given a 3 foot (vertical and associated horizontal) increase in water surface



elevation above the current effective 100-year floodplain. The CRAB boundaries include areas that may be inundated from 0 to 1 foot, 1 to 2 feet, and greater than 2 feet. These layers are shown on **Figure 3.1**. Based on the CRAB, inundation of 0 to 1 foot, 1 to 2 feet, and greater than 2 feet were identified on the northern and eastern portion of the study area.





### 3.3 Location

The project is located on 116 Johnstown Lane, within the town limits of the Town of Centreville which is the county seat of Queen Anne's County on the eastern shore of Maryland. The project will be on parcels currently owned by the Town Council of Centreville.

### 3.4 Population Trends

Population in the Town has grown from 2,018 in the 1980 census to 4,949 in the 2020 census, see **Table 3.1** and are expected to grow for at least several more years.

Table 3.1: Centreville Population Data											
Year	1980	1990	2000	2010	2020						
Population <sup>1</sup>	2,018	2,097	1,970	4,285	4,949						
Growth Rate	8.9%	3.9%	-6.1%	117.5%	1.5%						

https://www.census.gov/programs-surveys/popest/technical-documentation/research/evaluation-estimates/2020-evaluation-estimates/2010s-cities-and-towns-total.html

#### The data from **Table 3.1** is also depicted in **Figure 3.2**.

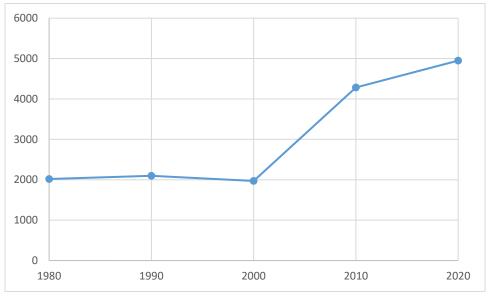


Figure 3.2: Centreville Population

According to the Queen Anne's County 2011 Comprehensive Water and Sewerage Plan:

"The Growth Areas generally include parcels of land contiguous to the east and west sides of the current Town boundaries. It is anticipated that growth pressures will occur, for the most part, on the Route 301 side of Centreville, due to the multiple road connections. Present growth patterns in and near the Town support this premise. The Town anticipates a phased approach to annexation."

The Town of Centreville Community Plan, 2009, estimated total future wastewater demand of 1.62 MGD.



# 4 Existing Performance, Facilities, and Conditions

### 4.1 Location Map

The Centreville WWTP is located within a residential area of the town. **Figure 4.1** provides a location map to show the relative distance between Centreville WWTP and the irrigation spray fields.



Figure 4.1: Location Map (Credit: Google Earth)

#### 4.2 History

The original portions of the Town's sewer collection system were installed in 1934. A primary wastewater treatment facility was constructed in the 1960's. A major Biological Nutrient Removal (BNR) upgrade was completed in 2005 that installed influent screening, a two (2) tank SBR, cloth media particulate tertiary filters, UV light disinfection, effluent pumping, treated effluent storage lagoons, spray irrigation fields, and reed drying beds. The treatment was designed to provide treatment for 0.542 MGD of annual average flow with an effluent total nitrogen of 5.5 mg/L, and effluent total phosphourus of 1.0 mg/L.

The receiving stream, Gravel Run, is a tributary of the Corsica River. At the time of the planning and design of the 2005 BNR upgrade and expansion, the total maximum daily loads for the Corsica River were being developed. To accommodate the planned flows, the Town decided to forgo year-round discharge to Gravel Run and developed a spray irrigation disposal system to provide 0.542 MGD of disposal capacity. Discharge to Gravel Run was restricted to cold weather months.



# 4.3 Financial Status

As described in **Section 3.4**, the Town has been experiencing growth within the existing water and sewer service areas. As the largest town in Queen Anne's County, it's central location on the eastern shore, and its designation as a Smart Growth area, the population is expected to continue to grow significantly for many years. The Queen Anne's County Comprehensive Water and Sewer Plan indicates future build out flows will reach 1.75 MGD of sanitary flow.

The Town has invested significant funds into upgrading its water distribution and sanitary collection systems to improve their integrity and position them for future flows.

Table 4.1: Recent Wate	r and Sewer Income	and Expenses		
	Wa	ater	Se	wer
Fiscal Year (FY)	Income	Expenses	Income	Expenses
FY20	\$844,402	\$1,115,148	\$891,258	\$1,156,565
FY21	\$1,029,299	\$1,122,767	\$1,135,004	\$1,073,320
FY22	\$1,343,266	\$1,298,048	\$1,366,296	\$1,206,366
FY23 (Through 3/28/2023)	\$604,136	\$543,029	\$591,252	\$850,908

The Town's recent and budgeted water and sewer income and expenses are summarized in Tables 4.1 and 4.2.

Table 4.2: Budgeted Wa	Table 4.2: Budgeted Water and Sewer Income and Expenses										
	Wa	ater	Se	wer							
Fiscal Year (FY)	Income	Expenses	Income	Expenses							
FY24	\$1,414,928	\$1,616,603	\$1,531,427	\$1,510,483							
FY25	\$1,485,675	\$1,697,433	\$1,607,998	\$1,586,007							
FY26	\$1,574,816	\$1,782,305	\$1,704,478	\$1,665,307							
FY27	\$1,685,053	\$1,871,420	\$1,826,791	\$1,781,878							
FY28	\$1,819,857	\$1,964,991	\$1,972,694	\$1,870,972							

The existing debt service held by the Town for the water and wastewater systems are summarized in Table 4.3.

Table 4.3: Existing Debt Service	
Area	Existing Debt (as of March 2023)
Water	\$5,075,189
Sewer	\$4,960,400



# 4.4 Current Influent Conditions

Influent conditions are not measured on a regular basis at the Centreville WWTP. Operators are able to gather composite samples of the influent when required. Composite influent sampling data from September/October 2017 and March 2023 was provided by the Town of Centreville for evaluations in this PER. **Table 4.4** lists the existing estimated average and maximum monthly influent conditions, based on the available sampling data. **Appendix C** includes all data from influent sampling.

Table 4.4: Exis	Table 4.4: Existing Influent Conditions at Centreville WWTP												
Condition	Flow	Wastewater Temperature	Biochemical Oxygen Demand (BOD)	Volatile Suspended Solids (VSS)	Total Suspended Solids (TSS)	Total Kjeldahl Nitrogen (TKN)	Total Phosphorus (TP)						
	(MGD)	(Deg C)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)						
Average	1.0	20	130	116	145	35	8						
Maximum Monthly	1.2	12	156	139	174	42	8						

### 4.5 Existing Plant Performance

Operations collects samples for analysis at the plant effluent for annual quality reporting. The effluent quality data from the monthly operating reports (MORs) that is relevant to the ENR upgrades was reviewed. The annual average, maximum monthly, and peak daily effluent flows from both stream discharge and spray discharge are summarized in **Table 4.5**.

Table 4.5: Centreville WWTP Stream and Spray Effluent Flows										
	Strea	am Effluent Flow (	MGD)	Spray Effluent Flow (MGD)						
Year	Annual Average	Maximum Monthly Average	Peak Day Flow	Annual Average	Maximum Monthly Average	Peak Day Flow				
2014	0.34	0.36	0.54	0.25	0.57	2.09				
2015	0.38	0.44	0.90	0.25	0.62	2.44				
2016	0.45	0.47	0.60	0.25	0.51	1.88				
2017	0.38	0.39	0.71	0.21	0.56	2.17				
2018	0.49	0.66	0.93	0.36	0.64	2.05				
2019	0.59	0.66	0.87	0.37	0.66	1.97				
2020	0.51	0.71	0.93	0.37	0.52	1.72				
2021	0.59	0.70	1.02	0.45	0.57	0.68				
2022	0.46	0.49	0.68	0.39	0.42	0.69				

The annual average and maximum monthly average for calendar years 2014 through 2022 for the total effluent flow, biochemical oxygen demand (BOD) concentration, total suspended solids (TSS) concentration, total kjeldahl nitrogen (TKN) concentration, ammonia (NH<sub>3</sub>) concentration, nitrate + nitrite (NO<sub>2</sub> + NO<sub>3</sub>) concentration, total nitrogen (TN) concentration, total phosphorus (TP) concentration, and E. coli concentration are included in **Tables 4.6 through 4.9**. **Tables 4.6 and 4.7** summarize the effluent quality of the stream discharge, which occurs during the winter months (December to March). **Tables 4.8 and 4.9** summarize the effluent quality of the spray discharge, which occurs throughout the warmer months of the year (April to November). **Appendix C** includes all available weekly average data for these categories.



Table 4.6: Centreville WWTP Stream Effluent BOD, TSS, TKN, and Ammonia Concentrations										
	Effluent BOD (mg/L)		Effluent TSS (mg/L)		Effluent TKN (mg/L)		Effluent Ammonia (mg/L)			
Year	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average		
2014	4.19	5.75	3.53	4.38	1.20	1.93	0.50	1.07		
2015	3.15	4.05	1.31	2.19	0.71	1.07	0.14	0.17		
2016	2.15	2.62	1.21	1.38	0.45	0.55	0.17	0.21		
2017	2.50	3.03	1.77	2.60	0.85	0.97	0.20	0.32		
2018	3.44	4.21	2.80	3.83	1.13	1.49	0.37	0.62		
2019	2.02	2.78	0.99	1.13	0.72	0.88	0.28	0.35		
2020	1.87	2.43	0.79	1.00	1.34	3.38	0.41	0.52		
2021	1.63	1.87	2.84	7.83	1.86	2.07	0.54	0.90		
2022	2.40	3.89	2.08	3.00	1.74	2.21	0.63	0.81		
Overall Average	2.76		1.77		0.91		0.29			

Table 4.7: Centreville WWTP Stream Effluent Total NO <sub>2</sub> + NO <sub>3</sub> , TN, TP, and E. Coli Concentrations										
	Effluent Total NO <sub>2</sub> + NO <sub>3</sub> (mg/L)		Effluent TN (mg/L)		Effluent TP (mg/L)		Effluent E. Coli (MPN/100 mL)			
Year	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average		
2014	2.09	2.60	3.29	4.20	0.25	0.75	1.28	1.90		
2015	1.89	2.24	2.61	2.88	0.18	0.26	4.52	12.4		
2016	2.32	2.98	2.76	3.39	0.21	0.25	4.80	41.2		
2017	1.46	1.70	2.32	2.67	0.46	0.64	1.22	9.98		
2018	1.72	2.06	2.91	3.38	0.45	0.78	20.6	42.9		
2019	1.83	2.65	2.55	3.39	0.66	1.12	31.3	91.6		
2020	1.21	1.48	2.55	4.87	0.88	1.05	118	185		
2021	1.63	2.54	3.15	3.55	0.68	0.95	461	1148		
2022	1.39	2.42	2.82	3.31	1.16	2.14	2.40	538		
Overall Average	1.79		2.71		0.44		25.93			

Table 4.8:	Table 4.8: Centreville WWTP Spray Effluent BOD, TSS, TKN, and Ammonia Concentrations										
	Effluent BOD (mg/L)		Effluent TSS (mg/L)		Effluent TKN (mg/L)		Effluent Ammonia (mg/L)				
Year	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average			
2014	2.72	3.88	4.22	4.50	0.82	1.09	0.22	0.27			
2015	3.87	6.26	3.03	6.08	0.85	1.20	0.18	0.27			
2016	3.44	5.22	1.57	3.22	0.81	1.36	0.23	0.64			
2017	3.17	4.08	1.52	3.00	0.90	1.30	0.16	0.30			
2018	2.76	4.63	1.17	1.63	1.09	1.77	0.19	0.75			
2019	1.27	1.64	0.67	0.94	0.98	1.14	0.18	0.28			
2020	1.76	2.69	0.86	1.44	1.00	1.95	0.40	1.03			
2021	2.41	3.70	1.89	3.00	1.96	3.48	0.57	2.22			
2022	2.06	2.75	2.35	4.50	1.37	1.67	0.37	0.60			
Overall Average	2.71		1.86		0.92		0.22				



Table 4.9: Centreville WWTP Spray Effluent Total NO2 + NO3, TN, TP, and E. Coli Concentrations										
	Effluent Total NO <sub>2</sub> + NO <sub>3</sub> (mg/L)		Effluent TN (mg/L)		Effluent TP (mg/L)		Effluent E. Coli (MPN/100 mL)			
Year	Annual Average	Annual Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average	Annual Average	Maximum Monthly Average		
2014	2.28	2.28	3.10	3.46	0.79	1.18	3.54	11.5		
2015	1.62	1.62	2.31	2.71	1.13	1.86	2.72	6.67		
2016	1.58	1.58	2.39	2.88	2.02	3.15	2.57	6.37		
2017	1.42	1.42	2.30	2.96	1.82	2.54	5.99	14.6		
2018	1.58	1.58	2.63	3.17	1.68	2.56	3.69	7.59		
2019	1.58	1.58	2.56	2.93	1.67	2.54	5.22	23.3		
2020	1.50	1.50	2.46	3.16	1.84	2.63	7.64	30.2		
2021	2.03	2.03	3.77	5.09	1.58	2.41	236.15	908		
2022	1.83	3.47	2.71	4.54	2.43	3.64	234.03	1223		
Overall Average	1.65		2.54		1.57		4.48			

Since January 2014, Centreville WWTP has had an average stream effluent total flow of 0.45 MGD and an average spray effluent flow of 0.29 MGD. **Figures 4.2 and 4.3** show the monthly average stream and spray effluent flows from January 2014 to December 2022.

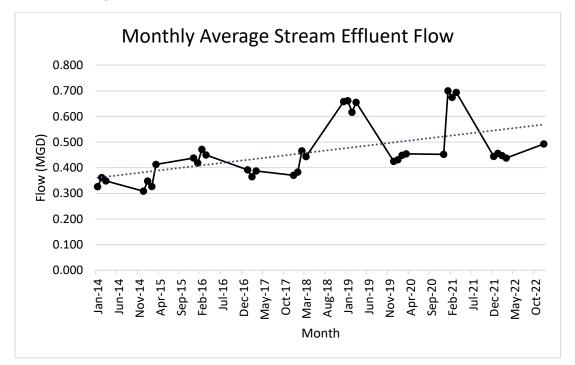
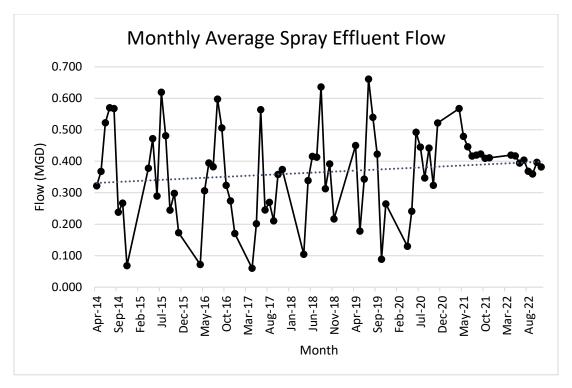


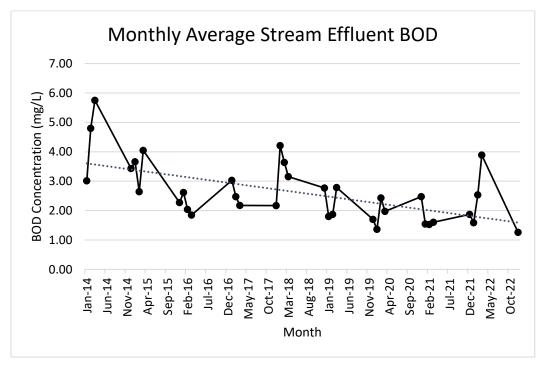
Figure 4.2: Stream Effluent Total Flow





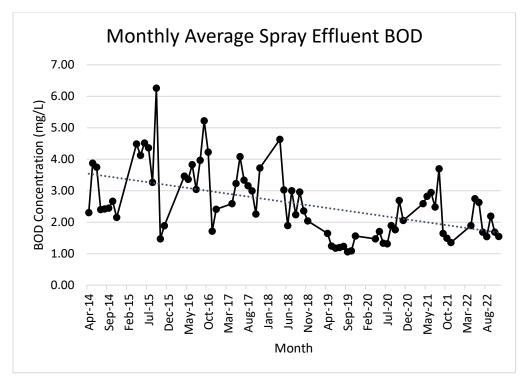
#### Figure 4.3: Spray Effluent Total Flow

**Figures 4.4 and 4.5** show the monthly averages of effluent BOD concentrations from January 2014 to December 2022. Since January 2014, the average stream effluent BOD concentration is 2.76 mg/L, and the average spray effluent BOD concentration is 2.71 mg/L.



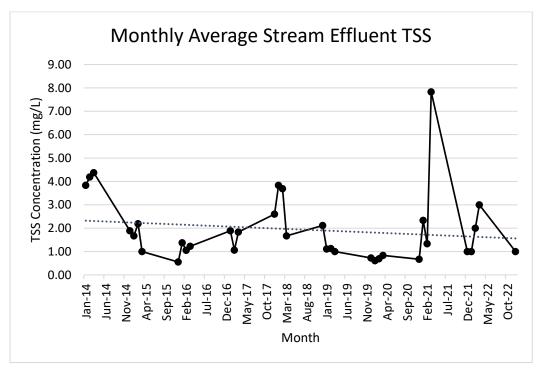






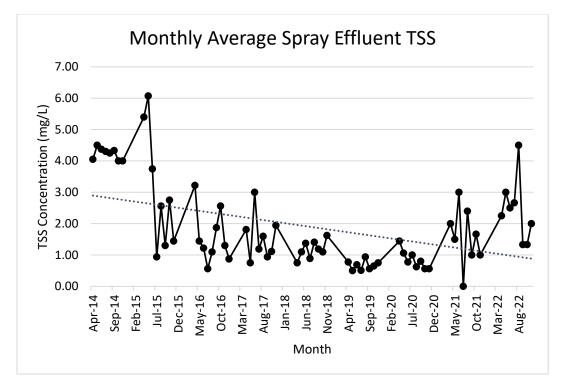
### Figure 4.5: Spray Effluent BOD Concentration

Since January 2014, the average stream and spray effluent TSS concentrations are 1.77 mg/L and 1.86 mg/L, respectively. **Figures 4.6 and 4.7** show the monthly averages of effluent TSS concentrations from January 2014 to December 2022.



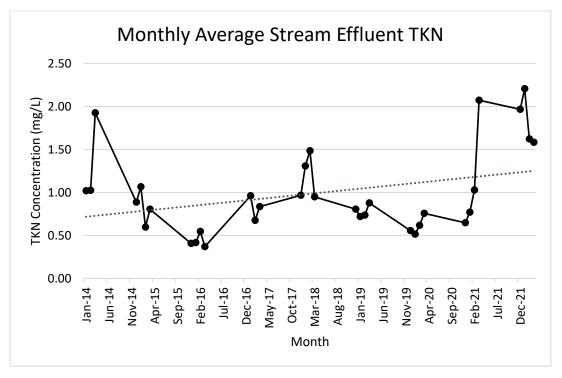






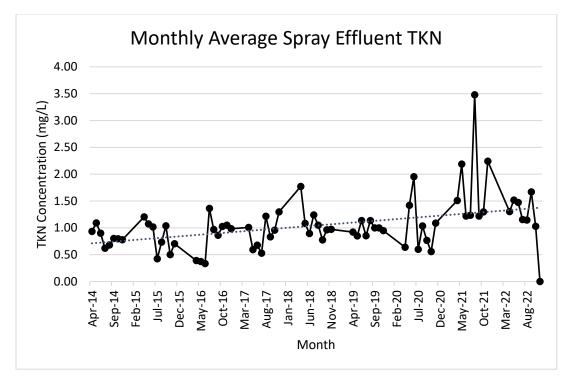
### Figure 4.7: Spray Effluent TSS Concentration

**Figures 4.8 and 4.9** show the monthly average stream and spray effluent TKN since January 2014. From January 2014 to December 2022, the average stream effluent TKN is 0.91 mg/L, and the average spray effluent TKN is 0.92 mg/L.



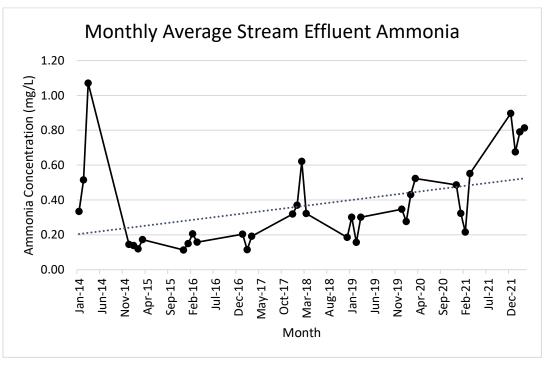






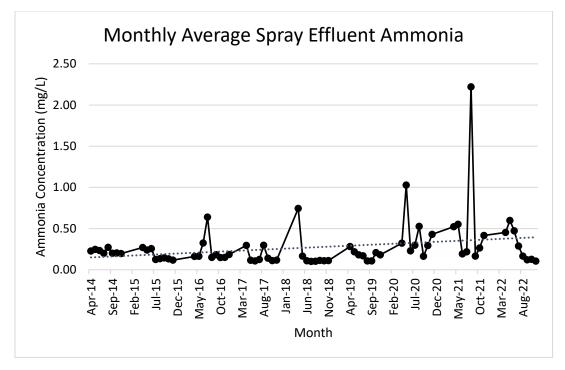
### Figure 4.9: Spray Effluent TKN Concentration

**Figures 4.10 and 4.11** show the monthly average stream and spray effluent ammonia since January 2014. Since January 2014, the average stream and spray effluent ammonia concentrations are 0.29 mg/L and 0.22 mg/L, respectively.









### Figure 4.11: Spray Effluent Ammonia Concentration

**Figures 4.12 and 4.13** show the monthly average nitrate plus nitrate  $(NO_3 + NO_2)$  concentrations in the stream and spray effluent from January 2014 to December 2022. On average, the  $NO_3 + NO_2$  concentrations in the stream and spray effluent have been 1.79 mg/L and 1.65 mg/L, respectively, since January 2014.

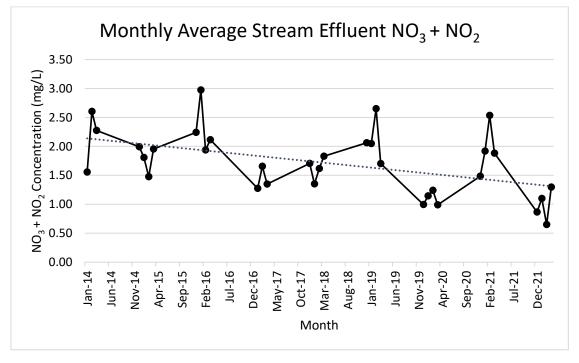
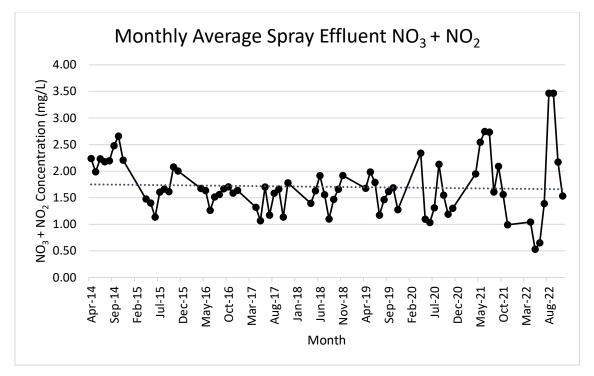


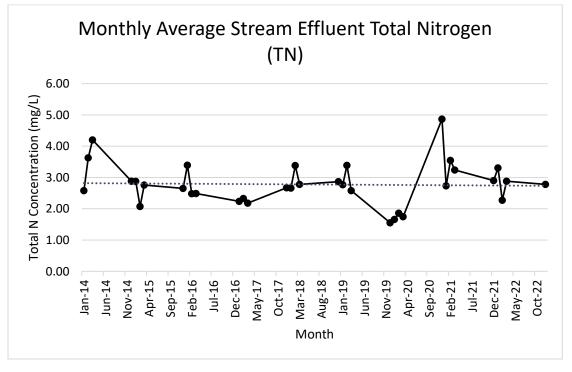
Figure 4.12: Stream Effluent NO<sub>3</sub> + NO<sub>2</sub> Concentration





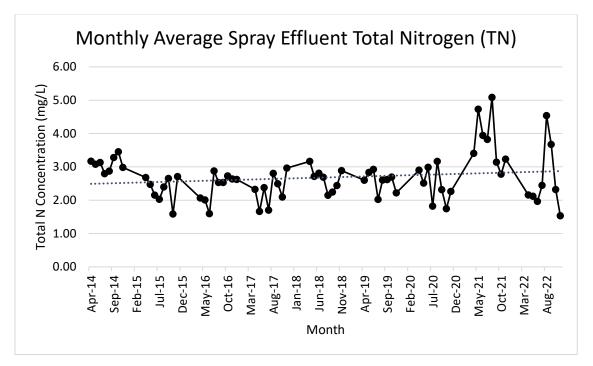
### Figure 4.13: Spray Effluent NO<sub>3</sub> + NO<sub>2</sub> Concentration

The monthly average TN concentrations in the stream and spray effluent from January 2014 to December 2022 are shown in **Figures 4.14 and 4.15**. The average TN concentration in the stream effluent is 2.71 mg/L, and the average total nitrogen concentration in the spray effluent is 2.54 mg/L.









### Figure 4.15: Spray Effluent Total Nitrogen Concentration

**Figures 4.16 and 4.17** show the monthly average TP concentrations in the stream and spray effluent from January 2014 to December 2022. The overall average TP concentration in the stream effluent is 0.44 mg/L, and the overall average TP concentration in the spray effluent is 1.57 mg/L. The monthly average TP concentrations have increased over the recent years, which is likely due to the increased flow through the WWTP.

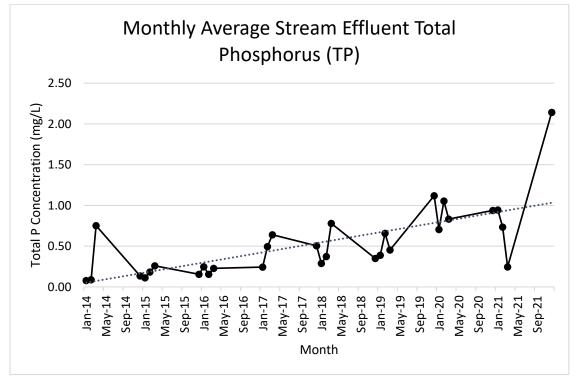
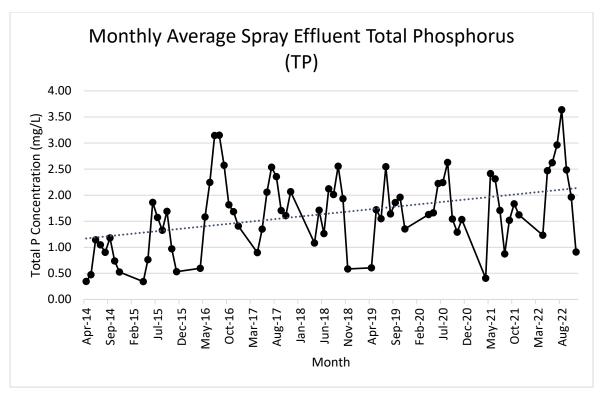


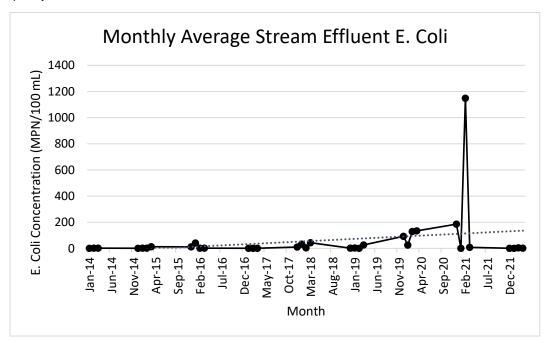
Figure 4.16: Stream Effluent Total Phosphorus Concentration





### Figure 4.17: Spray Effluent Total Phosphorus Concentration

Since January 2014, Centreville WWTP has had an average stream effluent E. coli concentration of 25.9 MPN/100 mL and an average spray effluent E. coli concentration of 4.48 MPN/100 mL. **Figure 4.18 and Figure 4.19** show the monthly average stream and spray effluent E. coli concentrations from January 2014 to December 2022. E. coli concentrations have increased in the effluent in recent years due to the WWTP operating closer to its design capacity.







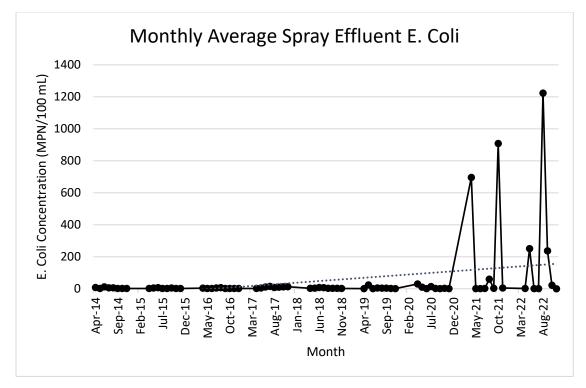


Figure 4.19: Spray Effluent Geomean E. Coli Concentration

# 4.6 Condition of Existing Facilities

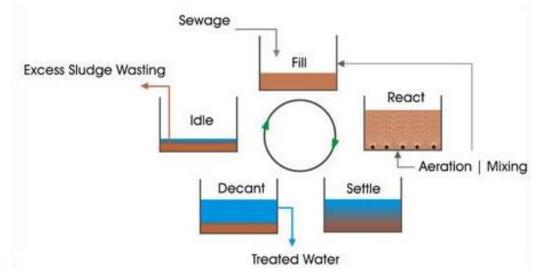
The Centreville WWTP is generally operating as intended without excessive maintenance and repair costs. The WWTP was most recently upgraded in 2003; therefore, most of the equipment is approximately 20 years old. Most mechanical equipment has a planned 20-year expected life.

# 4.6.1 Treatment Process Overview

The wastewater enters the WWTP via mostly force main, and some gravity pipelines, into a manhole, and then flows by gravity through a screening facility. The screened flow continues to the SBR tanks. Flow is directed into one (1) of the two (2) SBR tanks through two automated valves controlled by the SBR Control Panel.

The operation of an SBR is based on a fill-and-draw principle, which consists of five steps: fill, react, settle, decant, and idle. These steps can be altered for different operational applications and the general sequence is shown in **Figure 4.20**.





### Figure 4.20: Sequencing Batch Reactor Sequence of Operation (Source: Aqua-Aerobic Systems, Inc.)

#### Fill

During the fill phase, the basin receives influent wastewater. Mixing and aeration can be varied during the fill phase to create the different environments for the biomass analogous to the conditions in a traditional activated sludge basin, anaerobic, oxic, and anoxic.

#### <u>React</u>

During this phase, no wastewater enters the basin, and the mechanical mixing and aeration units are on. Most of the carbonaceous BOD removal occurs in the react phase. Further nitrification occurs by allowing the mixing and aeration to continue. Because there are no additional volume and organic loadings, the rate of organic removal increases dramatically.

#### <u>Settle</u>

During this phase, activated sludge can settle under quiescent conditions — no flow enters the basin and no aeration and mixing takes place. The activated sludge tends to settle as a flocculent mass, forming a distinctive interface with the clear supernatant. This phase is a critical part of the cycle, because if the solids do not settle rapidly, some sludge can be drawn off during the subsequent decant phase and thereby degrade effluent quality.

#### <u>Decant</u>

During this phase, a decanter is used to remove the clear supernatant effluent. The floating decanter maintains the inlet orifice slightly below the water surface to minimize the removal of solids in the effluent removed during the decant phase, an example is shown in **Figure 4.21**. Floating decanters offer the operator flexibility to vary fill and draw volumes.

#### Idle

This step occurs between the decant and the fill phases. The time varies, based on the influent flow rate and the operating strategy. During this phase, a small amount of activated sludge at the bottom of the SBR basin is pumped out.





### Figure 4.21: SBR Tank w/Surface Mixer/Aerator and Decant Device (Credit: Aqua-Aerobic Systems, Inc.)

Decant from the SBR flows into a post equalization tank, and waste sludge is pumped into an aerated sludge digestion tank. Submersible pumps send the SBR decant to a cloth media filter. The filtered effluent flows by gravity through a UV light disinfection channel, and then into the wet well of the Effluent Pump Station. Flow is either pumped to the effluent storage lagoon or flows by gravity to the chlorine contact tank.

### 4.6.2 Process/Equipment Assessment

### 4.6.2.1 Influent Screening

The influent screening is rotating drum screen manufactured by Lakeside. The screen is reported to effectively remove solids and is operating as intended. Operations also reported that this model of screen is no longer manufactured by Lakeside, and the cost of spare parts have increased significantly, as has the lead time to obtain parts. It is recommended to replace the screen, retrofitting the existing concrete channel as needed.

### 4.6.2.2 Sequencing Batch Reactor

The SBR provides treatment of the influent wastewater. BOD is removed and the influent TKN is nitrified to ammonia. The SBR is also able to partially denitrify the ammonia to nitrogen gas, to provide BNR levels of treatment, typically less than 5 mg/L of TN in the treatment plant effluent. **Table 4.10** provides the physical arrangement of the SBR's two (2) rectangular tanks.



Table 4.10: Existing SBR Tanks			
Parameter	Value	Units	
Length, each	70.5	Ft	
Width, each	53.2	Ft	
Volume at Min. Side Water Depth, each	0.402	Million Gallons	
Volume at Avg. Side Water Depth, each	0.464	Million Gallons	
Volume at Max. Side Water Depth, each	0.589	Million Gallons	

The SBR process equipment in each of the two (2) SBR basins includes:

- One (1) Influent Actuated Valve
- One (1) Surface Mixer
- Five (5) Removable Fine Bubble Aeration Diffuser assemblies
- One (1) Decant Mechanism
- One (1) Submersible Sludge Transfer Pump

The equipment is in good shape and is operating as intended. Although the equipment is nearing its expected life, it may have additional years of service left, in the range of 3-5 years with close attention to following factory advised maintenance and rebuilds.

The air for the liquid treatment process, and the post equalization tank, is supplied by three (3) 50 Horsepower (HP) blowers located in the Filter and Blower Building. Each blower has the design operating point of 525 SCFM, at a pressure of 10.7 PSIG. The existing blowers are operating as intended and appear to have many years of service life left with close attention and following the factory advised maintenance and rebuilds.

#### 4.6.2.3 Post Equalization Tank

The decant from the SBR flows into the post equalization tank for aeration and to reduce the fluctuations in the flow to the downstream processes. A summary of the physical arrangement of the post equalization tank is in **Table 4.11**.

Table 4.11: Existing Post Equalization Tank			
Parameter	Value	Units	
Length	52.9	Ft	
Width	36.7	Ft	
Min. Basin Volume	0.021	Million Gallons	
Max. Basin Volume	0.146	Million Gallons	
Working Volume	0.125	Million Gallons	



Equipment in the post equalization tank includes:

- Fine Bubble Diffuser Assemblies
- Two (2) Submersible Centrifugal Filtration Feed Pumps

The equipment is in good shape and is operating as intended. Although the equipment is nearing its expected life, it appears to have many additional years of service left with close attention to following the factory advised maintenance and rebuilds.

### 4.6.2.4 Sludge Holding Tank

Sludge wasted from the SBR is pumped into the sludge holding tank for stabilization (i.e., reduction of volatile solids). The sludge holding tank physical layout information is in **Table 4.12**.

Table 4.12: Existing Sludge Holding Tank			
Parameter	Value	Units	
Length	52.9	Ft	
Width	32.3	Ft	
Min. Basin Volume	0.138	Million Gallons	
Max. Basin Volume	0.197	Million Gallons	

Equipment in the sludge holding tank includes:

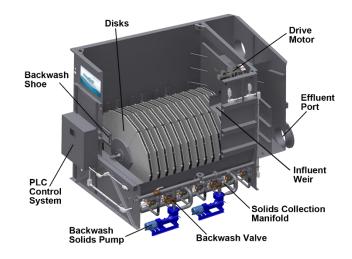
- One (1) 10 HP surface mixer
- Two (2) 30 HP floating aerators
- Supernate Pump
- Sludge Transfer Pump

The equipment is in good shape and is operating as intended. Although the equipment is nearing its expected life, it may have many additional years of service left with close attention and following factory advised maintenance and rebuilds.

#### 4.6.2.5 Cloth Media Filtration

Treated wastewater from the post equalization tank is pumped to a cloth media filter for the removal of suspended solids. The media filter is an AquaDisk unit manufactured by Aqua Aerobic Systems Inc., the same manufacturer as the SBR. The filter is a packaged unit complete with controls and backwashing and solids wasting system, see **Figure 4.22** which shows the main components of a disk filter.





### Figure 4.22: Cloth Media Filter Components (Credit: Aqua-Aerobic Systems, Inc.)

The media filter has been reported by operations to experience excessive head loss during typical wet weather flows, causing bypassing of the filter, degrading the effluent quality. The filter has produced excellent quality effluent during dry weather flows.

### 4.6.2.6 UV Light Disinfection

The filtered wastewater flows by gravity through a UV light disinfection channel. A total of twenty-four (24) low pressure high output ultraviolet lamps. The intense UV light inactivates microorganisms by destroying nucleic acids and disrupting their DNA. A typical unit can be seen in **Figure 4.23**.

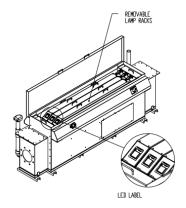


Figure 4.23: UV Disinfection Unit Isometric View (Credit: Enaqua)

The UV unit design parameters are included in Table 4.13.

Table 4.13: Existing UV Disinfection		
Parameter	Value	Units
Peak Hour Flow	0.75	MGD
Quantity of UV Reactors	1	-
Number of Total Lamps	24	-
UV Dose (Calculated)	> 40	mJ/cm <sup>2</sup>
Effluent Quality	< 84	MPN/100 mL E. Coli



The UV disinfection system was installed in 2016 and is operating as intended. Additional units will be needed for the expansion to 1.0 MGD AAF. Additionally, Class III and Class IV Water Reuse will require more stringent Fecal Coliform effluent quality.

### 4.6.2.7 Effluent Pump Station

Design information on the two (2) Goulds 20 HP effluent pumps was not available, but operations reports that the pump station can get overwhelmed during wet weather flows. The pumps were installed as part of the 2003 upgrade.

### 4.6.2.8 Chlorine Contact Tank

When the WWTP is discharging to the stream, effluent flows by gravity from the Effluent Pump Station to the chlorine contact tank, and then to the cascade steps aeration, and then the outfall.

The isolation gates in the chlorine contact tank are beyond their expected life and need replacement. The concrete tank is in fair shape.

As a back up to the UV disinfection system, when flowing to the stream, sodium hypochlorite solution can be added to the Effluent Pump Station, with subsequent dechlorination at the end of the chlorine contact tank.

#### 4.6.2.9 Chemical Addition

In the past, polyaluminum chloride (PACI) solution was added to the SBR to precipitate ortho-phosphorus for subsequent removal through settling, and in the cloth media filter. PACI dosing consisted of a dosing pump pulling solution from a drum. PACI has not been used recently since the WWTP has been able to meet their target effluent phosphorus levels by biological phosphorus (bio-P) uptake without the use of additional chemicals.

Sodium hypochlorite solution is used in cleaning the cloth media filter, and as a back up to the UV disinfection process. Sodium hypochlorite dosing consists of a dosing pump pulling solution from a tote.

### 4.6.2.10 Existing Electrical System

The existing electrical service for Centreville Water Wastewater Treatment Plant is provided from Delmarva Power (DP) company owned 500 kVA, 25 kV to 480/277-volt, 3 phase, 4 wire, pad mounted transformer. The existing transformer secondary feeders installed underground to serve an 800-amp main distribution panel (MDP) via an 800-amp enclosed circuit breaker and an 800-amp automatic transfer switch (ATS) with bypass located in the existing electrical room of the Filter and Blower Building. A 500kW engine driven generator provides backup power.

The existing MDP serves the existing WWTP buildings including the existing Panel DP in the electrical room, 600amp motor control centers (MCC) A and B in the Blower room and a 75 kVA transformer located outside of the existing Lab Building. The existing Panel DP, 400 amp, 480/277 volt, 3 phase, 4 wire, serves the lighting, 3 phase motors and a 30 kVA, 480-208/120 volt, 3 phase, 4 wire, dry type transformer mounted on the wall to serve the existing Panel PA, 100-amp, 208/120 volt, 3 phase, 4 wire for the receptacles and miscellaneous loads.

The existing Lab Building Is served by a 75 kVA, 480-208/120-volt, 3 phase, 4 wire pad mounted dry type transformer outside of the building. The existing transformer feeders enter the building via old CT cabinet to a 400-amp distribution panel which in turn serves the existing MCC, Panel PC and Panel PD. The existing MCC is located in the existing Lab Building workroom and the Panel PD is located in the Pump Building (old Admin Building).



### 4.6.2.11 Existing Controls System

The plant currently has no centralized monitoring or control system for operation of the plant. Currently all operations are performed manually by operators physically going to the process areas and starting and stopping equipment as required. Operators must go to the different instruments throughout the plant to take readings and manually collect process data.

The SBR system is provided with the manufacturer's Programmable Logic Control (PLC) based control panel to provide for automatic operations of the system. However, the system must be started locally at the control panel and process data is only able to be monitored at panel mounted interface screen. The manufacturers control panel is corroded beyond its useful life. The floor stands for the panel enclosure are almost entirely decayed by rust with the weight of the panel seemingly supported by the conduits entering at the bottom of the panel.

The PLC controller in the SBR control panel is also at the end of its supported lifecycle. The manufacturer of this product has ceased manufacturing this model and no longer supports it for technical assistance, maintenance, or software interface.

Alarms throughout the plant are relayed through an alarm notification system called Mission. The Mission system provides a common trouble alarm for a process area without providing any amplifying information to allow for advanced troubleshooting or prioritizing response. The Mission system is also used at the Town's pump stations, tanks and water treatment plants for monitoring alarms at these facilities. When an alarm is active, the Mission system will use a cellular connection to notify operators based on a pre-programmed calling list.

### 4.6.2.12 Existing Spray Irrigation Effluent Disposal

Currently the Town has an MDE discharge permit to spray irrigate 0.542 million gallons per day (MGD) of treated effluent on 223.7 acres of suitable spray area from March 1 to December 15. They have mainly center pivot spray rigs, but also have three fixed head spray nozzles located near the control building. There are ten (10) spray fields with individual approved spray rates that vary from 0.3 inches per week to 2 inches per week, per field.

Currently, they spray at approximately 0.5 MGD among 173.44 acres of irrigation fields. Often the existing fields are sprayed 8 to 12 hours per day.

The Town is not permitted to use chicken manure on the spray fields. They apply chemical fertilizer because more nitrogen is required per the annual Nutrient Management Report than is in the effluent. The nitrogen concentration in the effluent is typically 2 mg/L or lower. At the spray fields the main crop grown is corn, with some soybeans also.

The existing storage pond's original design was not specified large enough. The pond was built in 2001 when the spray rigs were built.

Water levels and groundwater quality are monitored in ten monitoring wells on a quarterly basis. Water quality is monitored at three stream sites quarterly. On a weekly basis, water levels are measured in ten piezometers which are located near the various center pivots. Permission to spray effluent is conditional on water levels observed in the piezometers.

The surficial geologic formation that underlies the area is Upland Deposits according to the Geologic Map of Maryland (1968). The Upland Deposits are gravel, sand, silt, and clay that were deposited in the Quaternary Period of earth history.

Underneath the Quaternary layer are sediments of the Calvert Formation, which formed in the Tertiary Period. The upper part of the Calvert Formation is exposed in stream valleys in the Centreville area. At the Town spray field property, the Calvert Formation is represented by a layer of green silty sand exposed in the stream valley.



According to the Natural Resources Conservation Space (NRCS) Soil Survey of Queen Anne's County, Maryland the Town spray fields are underlain by soils of the Matapeke-Mattapex-Nassawango map unit. The farms located to the northwest and to the northeast of the Town spray fields are underlain by soils of the Ingleside-Pineyneck-Unicorn map unit. Soils at both the existing spray fields and the considered expansion farms are both well-drained which is advantageous. Both can also include soils that have a wet substratum.

# 4.7 Water and Energy Audits

The Centreville WWTP is supplied with potable water from the Town's water treatment and distribution system. The Town does not currently meter the potable water use. The WWTP does not have an onsite treated effluent water reuse system, and therefore all water used in the treatment and maintenance operations is potable water.

A water audit of current significant uses of potable water was conducted at the site and is summarized in **Table 4.14**.

Table 4.14: Existing Significant Water Uses				
Use	Estimated Instantaneous Flow	Estimated Average Daily Usage		
	gpm	gpd		
Influent Screen Spray Water	2	1,000		
SBR Scum Spray Down	5	50		
Miscellaneous Cleaning During Maintenance	10	100		

The electric usage of the WWTP is only metered for the entire site. The current major energy demands are summarized in **Table 4.15**.

Table 4.15: Existing Major Electrical Demands			
Major Electrical Demands	Quantity	Electric Demand, Each	
Aeration Blowers	3	50 HP	
Sludge Holding Aeration Blowers	2	30 HP	
SBR Mixers	2	20 HP	
UV Disinfection	1	15 kW	
Effluent Pumps	2	20 HP	
Buil/ding Electric Heat, Total	-	35 kW, total	

For the period January 2020 through March of 2023 the WWTP used an average of 1,091 kWh each day. The annual average daily electric usage for 2020 through 2022 is summarized in **Table 4.16**.



Table 4.16: Recent Electrical Usage			
Year	Annual Average Electric Demand (kWh / Day)		
2020	1,107		
2021	1,057		
2022	1,048		



# 5 Need for Project

The annual average daily flows from the Centreville WWTP from 2014 to 2022 are shown in **Table 5.1**, with an annual average daily flow of 0.40 MGD.

Table 5.1: H	listorical Ce	entreville Ef	fluent Flow	1					
	2014	2015	2016	2017	2018	2019	2020	2021	2022
Annual Average Flow (MGD)	0.36	0.38	0.36	0.32	0.40	0.44	0.42	0.51	0.41

The annual average effluent flows have recently exceeded 80% of the existing facility's permitted flow (0.542 MGD), with the average of the last three calendar years (2020-2022) averaging 0.45 MGD, which is 83% of the permitted flow. As discussed in **Section 3.4**, the population is expected to continue to grow.

# 5.1 Health, Sanitation and Security

Maintaining the health, sanitation, and safety of the population served, as well as the areas impacted by the disposal of the treated effluent are key drivers for the project.

The influent screen, treatment system aeration capacity, tertiary filter, UV disinfection, and Effluent Pump Station regularly reach their practical operating limits during wet weather events. The systems are operating as designed, and maintenance is timely, but these systems do not have sufficient capacity to handle the full range of flows and loads the WWTP is currently experiencing.

As discussed in **Section 5.2**, the majority of the treatment plant is reaching 20 years old. The concrete tanks are in excellent condition. The mechanical process systems are reaching the end of their useful life, and the control systems have exceeded their useful life. The SBR main control panel is no longer supported by the manufacturer. Without the control system operating, a sequencing batch reactor requires a dedicated operator to provide manual operation 24/7. This represents a significant risk to maintaining treatment.

The spray irrigation system is also showing signs that maintaining compliance with the discharge permit requirements will become more challenging as flows increase. During extended wet periods the treated effluent storage lagoon has approached its capacity, and the spray irrigation fields are also approaching their practical limits.

Due to the stress on the treatment plant and disposal sites, and despite the efforts of the Town's operations department, the NPDES discharge permit has been violated multiple times over the past three years. Further information regarding these violations can be found in the Town's response to MDE. Regardless, the system continues to age and flows increase the system will approach a tipping point where it is unable to reliably meet the discharge permits.

# 5.2 Aging Infrastructure

The majority of the WWTP, the effluent storage lagoon and spray irrigation system were brought online in 2005. Mechanical process equipment at WWTPs has a generally accepted expected life of 20 years. As the equipment exceeds this, the cost to repair the equipment starts to outweigh the cost of replacement. More importantly, when equipment is offline waiting for repairs, it is not available to contribute to the treatment capacity.

Control systems have the shortest expected life of equipment at a WWTP. The specific expected life will depend on the manufacturer continuing to support the hardware and software. With the constant changes and upgrades in processor based systems, the manufacturers must use the currently proven technology to keep costs



competitive and can only support so many different systems with spare parts, programming, and updates. The primary hardware in the Centreville sequencing batch reactor's main control panel is no longer supported by the manufacturer. As such replacement parts are no longer readily available, can take months to find and can be many times the cost of supported systems.

# 5.3 Reasonable Growth

The project is consistent with the Maryland "Smart Growth" legislation which established Priority Funding Areas (PFA). The wastewater treatment plant lies completely within a Maryland Department of Planning PFA. The Queen Anne's County Comprehensive Plan for Water and Sewer identifies the planned growth for the Town of Centreville and projects a buildout sanitary flow of 1.75 MGD.

As indicated previously, the annual average flows to the WWTP are currently above 80% of the permitted flow, at approximately 0.4 MGD. Developers have approached the Town with conceptual plans for significant housing developments within the Town. Those developments are not practical without the expansion of the WWTP. Considering the recent average flows, and the known potential for development, increasing the design capacity to 1.0 MGD was selected. The expansion to 1.0 MGD will give the Town many years of planned growth without having a WWTP that is excessively large to treat the recent flows. Projecting out when the new developments will be brought online is difficult, but it is expected the Town could reach 80% of the 1.0 MGD capacity sometime between 2040 and 2050.



# 6 Upgrade and Expansion Alternatives

The primary goals for the upgraded and expanded treatment process are:

- 1. Provide ENR levels of treatment and continue compliance with the NPDES discharge permit (refer to **Section 2.3.1**).
- Provide liquid and solids treatment for 1.0 MGD annual average influent flow and associated wet weather flows.

There are several facilities that require expansion and/or upgrades to accommodate the increased level of treatment and hydraulic throughput regardless of the treatment alternative that is selected. Equipment catalogs for the major equipment described in this section are included in **Appendix D**.

The three alternatives considered are:

<u>Alternative 1</u> – Expand the SBR process, expand the post flow equalization tank, and add a denitrifying tertiary filter.

Installing two additional SBR tanks will expand the current process that has been proven to be effective. The SBR operation is already understood, operating, and maintaining the system will remain straightforward. To accommodate for increased flow, the post equalization tank would also be expanded, and a tertiary denitrifying filter would be added downstream of the SBR. The process will require more space and need to deal with the aging SBR infrastructure. The SBR's main control panel is no longer supported and finding replacement parts is difficult.

<u>Alternative 2</u> – Replace the existing SBR system with a 5-stage conventional activated sludge process and add a tertiary filter with denitrifying capability.

This biological process reduces nitrogen and phosphorus compounds by switching between both high and low oxygen environments. Flow passes through distinct anaerobic, anoxic, aeration, post anoxic, and reaeration stages, similar to a "Bardenpho" configuration. The anaerobic zone enriches phosphorusaccumulating organisms that help remove phosphorus in later stages. Denitrification occurs in both anoxic zones, where denitrifying bacteria converts nitrates into nitrogen gas. In the aeration stage, nitrification occurs, converting ammonia to nitrate, and is recycled to the first anoxic zone. The reaeration stage helps release any more nitrogen gas minimize denitrification occurring within the subsequent clarifier. RAS from the secondary clarifier is pumped to the dewatering facility and the rest of the flow is further treated by a denitrifying filter. Due to the change in treatment technology, additional operator knowledge will be required to maintain and operate the system.

<u>Alternative 3</u> – Replace the existing SBR system with to a 5-stage MBR activated sludge process.

The process configuration is similar to Alternative 2 utilizing 5-stages, however solid separation is facilitated via the membrane and not via clarifiers. The membrane has a pore size of approximately 1 micron, that allows water to permeate while retaining the activated sludge in the reactor. The effluent is pulled through the membranes, which are commonly either a tube or plate style, the resulting permeate has a low turbidity with the excess sludge being removed from the reactor basins. This results in a high-quality effluent without the need for a tertiary filter, and results in a compact process that also has a longer sludge retention time. The membrane does limit the hydraulic throughput of the treatment process and an influent flow equalization tank is required upstream of the MBR to ensure flux through the MBR does not exceed its capacity. The existing SBR tanks would be converted into influent flow equalization tanks. Additionally, knowledge to operate and maintain the new system as well as the additional process equipment compared to the other alternatives, will be required.

In addition to the three alternatives, a 'do nothing' option was also initially considered but is not a practical option. The current flows to the WWTP exceed 80% of the design capacity, and as indicated in the recent performance,



the WWTP has occasionally exceeded the permit limits. Therefore, the existing plant is only marginally capable of treating the existing flows. As the plant equipment ages the repairs and associated downtime will increase, which will degrade the effective treatment capacity. Refer to **Section 5**.

# 6.1 Common Upgrades

Upgrades that are common to all three alternatives include:

- 1. Expand influent screening. Alternatives 1 and 2 will be a 6-mm effective opening screen, and Alternative 3 will require a 2-mm opening screen.
- 2. Construct/convert influent flow equalization tank(s).
- 3. Expand UV disinfection.
- 4. Expand chemical dosing.
- 5. Review options for expansion of treated effluent disposal.
- 6. Install sludge treatment and dewatering.
- 7. Install an on-site non-potable water system.
- 8. Miscellaneous refurbishment of reused facilities.
- 9. Electrical and control system upgrades.

### 6.1.1 Influent Screening

Due to the age of the existing influent screen, difficulty in procuring replacement parts, and to accommodate higher peak flows, it is recommended to replace the screen. The existing screen has performed well, and the operations staff is familiar with operating and maintaining this style of screen. Additionally, the existing concrete channel appears in fair condition and can be reused with some modification and refurbishment (e.g. spalling and crack repair).

For Alternatives 1 and 2 it is recommended that the replacement screen be the same style as the existing screen. The existing mechanical screen bypass channel and manual bar rack would remain in place to serve as back up to the additional mechanical screen.

Table 6.1: Influent Mechanical Screen Design Basis – Alternatives 1 and 2				
Parameter	Value	Units		
Quantity, Duty/Standby	1/0	Unit		
Peak Flow	4	MGD		
Screen Opening	6	mm		
Screen Basket Diameter	40	Inches		
Ancillary Equipment	Integrated Screenings Washer/Compactor with Bagger	-		
Basis of Design	Lakeside – Raptor	-		

The influent screen for Alternatives 1 and 2 would comply with the design criteria in **Table 6.1**.

Alternative 3 – MBR Activated Sludge requires a more robust screening system with smaller openings to protect the membranes. For Alternative 3, it is recommended to provide both coarse and fine screens in series, with 1 duty and 1 standby screen for each size. The existing screen channels will be expanded to add a channel for a second coarse screen. The existing screen will be replaced. A dual channel fine screen facility will be constructed to the north of the existing screens. The influent screen for Alternative 3 would comply with the design criteria in **Table 6.2**.



Table 6.2: Influent Mechanical Screen Design Basis – Alternative 3				
Parameter	Value	Units		
Coarse Screens				
Quantity, Duty/Standby	1/1	Unit		
Peak Flow, Each Screen	4	MGD		
Screen Opening	1/2	inch		
Ancillary Equipment	Screenings Washer/Compactor with Bagger	-		
Basis of Design	Duperon Low Flow	-		
Fine Screens				
Quantity, Duty/Standby	1/1	Unit		
Peak Flow, Each Screen	4	MGD		
Screen Opening	2	mm		
Ancillary Equipment	Screenings Washer/Compactor with Bagger	-		
Basis of Design	Huber Band Screen	-		

# 6.1.2 Influent Flow Equalization Tank

To accommodate fluctuations in influent flows during wet weather events, an influent flow equalization (EQ) tank is recommended for each alternative. The equalization tank will reduce the peak flows (peak shaving) to the SBRs, activated sludge basins, or MBRs and therefore improve the effluent quality during wet weather events. The influent flow EQ tank will be designed to provide wet weather peak flow shaving at the 1.0 MGD annual average flow conditions. Since the current influent flows are not measured, the tank will be sized based on typical municipal peak flow characteristics. A conservative peak day peaking factor of 3 will be used, therefore a peak day flow of 3.0 MGD is expected. A 500,000-gallon working volume concrete tank is recommended to reduce the peak day flow through the treatment process to 2.5 MG. Redundant 750 gpm submersible pumps with variable frequency drives (VFD) will be mounted in the EQ tank to pump flow that is diverted from the EQ tank to the SBRs, activated sludge basins, or MBRs.

For Alternative 1 – SBR, a new 500,000-gallon working volume tank would need to be constructed onsite. For Alternative 2 – Conventional Activated Sludge and Alternative 3 – MBR Activated Sludge, the existing SBR tanks can be converted into two (2) influent flow EQ tanks. As shown in **Table 4.9**, each existing SBR tank has a maximum working capacity of 0.589 MG. Refer to **Section 6.3** for site plan layouts for each alternative.

# 6.1.3 UV Disinfection

The existing UV system was upgraded after 2005 and is operating well; however, it is undersized to handle the peak day flow following the expansion to 1.0 MG annual average influent flow and associated wet weather flows. To better utilize the space available in the Filter and Blower Building, the existing UV system will be replaced with an enclosed low pressure high output inline system. **Table 6.3** summarizes the design basis for the replacement system.



Table 6.3: UV Disinfection Replacement System Design Basis				
Parameter	Value	Units		
Quantity, Duty/Standby	1/1	-		
Configuration	Parallel	-		
Peak Day Flow	3.3	MGD		
Design Transmittance	65	%		
UV Dose	> 40	mJ/cm <sup>2</sup>		
Effluent Quality	< 116 <sup>(1)</sup>	MPN/100 mL E. Coli		
Number of Lamps, each unit	20	800W each, LPHO		
	ETS – UV System,			
Basis of Design	manufactured by	-		
	Evoqua			

<sup>(1)</sup> – Note that effluent quality limit would be 14 MPN/100 mL E. Coli if the new outfall extension is used, per **Section 6.1.5**.

### 6.1.4 Chemical Dosing

For the ENR upgrade of the Centreville WWTP, chemical addition for increased phosphorus removal will be required. The existing chemical dosing system will be expanded for additional polyaluminum chloride (PACI) and methanol dosing. Increased dosing of PACI is required to precipitate higher concentrations of ortho-phosphorus in order to meet a TP concentration less than 0.3 mg/L, as required by ENR. To meet the target TP concentration, an estimated 240 gallons per day (gpd) of PACI will be required. PACI will be dosed upstream of the denitrification filters for Alternatives 1 and 2, and downstream of the MBR process for Alternative 3. PACI dosing capacity will be increased by replacing the existing PACI drums with an 8,000-gallon capacity double contained PACI bulk storage tank located in the Filter and Blower Building. Alternatively, two (2) 4,000-gallon double contained tanks may be installed for PACI storage, if the Filter and Blower Building cannot accommodate a single larger tank. 8,000 gallons of PACI storage will provide over 30 days of chemical storage. The existing PACI dosing pumps will likely need to be replaced to accommodate a higher capacity.

Each alternative will also include methanol dosing to aid in additional nitrate removal and subsequent reductions in TN concentration less than 3 mg/L, as required by ENR. To meet the target TN concentration, an estimated 65 gpd of methanol will be required. For Alternatives 1 and 2, methanol would be dosed just upstream of the denitrification filters. For Alternative 3, methanol would be dosed within the MBR tank. Methanol will be stored in a 4,000-gallon double contained bulk storage tank, within or outside of the Filter and Blower Building for Alternatives 1 and 2, or within the MBR Process Building for Alternative 3. 4,000 gallons of methanol storage will provide over 40 days of chemical storage.

# 6.1.5 Review of Effluent Disposal Options

As previously mentioned, Centreville WWTP currently discharges to Gravel Run through an existing outfall during the cold weather months (December 1 to March 31). During the warmer weather months (April 1 to November 30), effluent is discharged to the Town's spray irrigation site. Previously, Centreville WWTP was permitted to utilize spray disposal year round; however, with the most recent permit update in 2010 and updated MDE requirements, spray disposal is restricted to March 1 to December 15.

At the Town's current spray irrigation disposal site, there is a total usable disposal area of 173.44 acres. The disposal site is reported to be near capacity at current flows. Concurrently with the ENR expansion and upgrade of the Centreville WWTP, the Town is actively pursuing expansion of the effluent disposal capacity to accommodate the expected increase in WWTP influent flows.

The Town has unsuccessfully pursued expanding its spray irrigation area, despite years of searching for suitable land. Other water reuse options, such as indirect potable reuse (IPR), have been discussed but are not



considered feasible in the near term. Therefore, expanding the surface water discharge to allow year-round discharge is currently the most viable approach. This section provides a brief overview of the proposed work to expand the surface water discharge effluent disposal.

### 6.1.5.1 Year Round Stream Discharge

To allow for year-round surface discharge, the Town is proposing to relocate the plant outfall to Corsica River at a location downstream of the Watson Road Bridge, which would be consistent with MDE's approved report for TMDL of Nutrients for Corsica River (May 2000).

The Town is proposing to manage the plant's expanded effluent flow by maximizing use of the existing spray irrigation field capacity in combination with discharge to a new Corsica River outfall within the TMDL nutrient limits. During the irrigation season, the current permitted flow (0.542 MGD) will be applied to the fields with the remaining effluent flows (0.458 MGD) to Corsica River. Outside the irrigation period, all flows would be discharged to Corsica River. The assumption for this scenario is ENR effluent quality with a consistent treatment performance of 3 mg/L and 0.3 mg/L for TN and TP, respectively. It should be noted that MDE has recently proposed a more stringent TP limit of 0.15 mg/L (i.e. 50% further reduction) if year-round stream discharge is selected. This would allow for a nutrient loading to Corsica River that is well below the established TMDL limits for low flow periods and for the total annual limit. These assumptions provide a good overall nutrient load margin of safety, especially during low flow (warm) periods where nutrient loads to Corsica River are most critical and where the utilization of the spray fields is greater.

With year-round surface discharge, upgrades to the existing Effluent Pump Station will be required to send additional flow through the new outfall pipe and further into the Corsica River. Additionally, a shellfish protection tank will be required. The tank will need to be sized for 24 hours of holding of the design average flow, or 1,000,000 gallons. The tank will be used to stop all wastewater from flowing to the stream if the effluent quality is poor or the disinfection system is offline. Upgrades to the Effluent Pump Station and construction of the shellfish protection tank are not recommended at this time as part of the ENR upgrade and expansion.

### 6.1.6 Biosolids Handling

The dewatered biosolids are currently disposed of in a landfill. By providing sufficient solids retention time in an aerobic digester, a Class B biosolids would be produced. This potentially could allow for land application of the dewatered biosolids.

For Alternative 1 – SBR, a new aerobic digester would be constructed in the footprint of the existing reed drying beds. For Alternative 2 – Conventional Activated Sludge and Alternative 3 – MBR Activated Sludge, the existing post equalization and sludge storage tanks will be retrofitted to be aerobic digesters. Refer to **Section 6.3** for site plan layouts for each alternative. The design criteria for the aerobic digester are provided in **Table 6.4**.

Table 6.4: Aerobic Digester Design Criteria		
Parameter	Value	Units
Design Waste Sludge	16,000	gallons/day
	1,300	Lbs dry solids/day
	10,000	Mg-TSS/L
Digester Solids Concentration with Settling and Decant	20,000	Mg-TSS/L
Solids Retention Time	60	days
Number of Tanks	2	-
Working Volume, each	250,000 <sup>(1)</sup>	gallons
Working Volume, Total	500,000 <sup>(2)</sup>	gallons
Surface Aerator Mixers (3 in each Tank)	20	HP/each

<sup>(1)</sup> – For Alternatives 2 and 3, the retrofitted aerobic digesters will have an approximate working volume of 175,000 gallons each.



<sup>(2)</sup> – For Alternatives 2 and 3, the retrofitted aerobic digesters will have an approximate total working volume of 350,000 gallons.

There are several proven, reliable sludge dewatering methods that can be applied to Centreville WWTP. Similar to the treatment process, a primary consideration should be that the equipment is straightforward to operate and maintain. One widely utilized technology across the wastewater industry is the belt filter press (BFP). See **Figure 6.1** for a BFP. BPFs have many advantages, including:

- Low capital cost,
- Low energy consumption,
- Simple operation and maintenance, and
- Ability to handle stringy solids (i.e. rags) and plastics.

Prior to sludge being deposited on the BFP, the sludge is conditioned with polymer to promote the coagulation of solids. The polymers would be received in a concentrated liquid format in 55-gallon drums. Polymer would be pumped by peristaltic pump to a make down tank, a 300-500-gallon fiberglass tank where potable water is added to condition the polymer and get the proper concentration for dosing. The polymer solution is then pumped into an injection ring located in the belt filter press feed pipeline and mixed in-line with the sludge.

During the dewatering operation, as the dewatered cake is discharged, the press belts are continuously washed with spray water. A wash water skid equipped with a booster pump will provide the pressure to adequately wash the belts.

The cake will be discharged from the belt filter press onto a belt conveyor which conveys the cake into a roll off storage container until it is taken for disposal.

Another technology considered is the volute dewatering press. The volute dewatering press is similar in overall configuration to a screw press, with a center conveying screw pushing the solids that are larger than the openings in the dewatering drum towards the discharge end. See **Figure 6.2** for a volute dewatering press, and **Figure 6.3** for a typical screw press. The screw press uses a static perforated, or slotted drum which separates the solids. The volute press utilizes the annular space between donut shaped plates to separate out the solids. The screw and volute press both have low capital costs and low energy consumption.

The volute dewatering press and screw press have fewer moving parts than the belt filter press which should translate to lower maintenance costs.

The BFP has low capital cost and low energy consumption as advantages that have led to numerous installations. The dewatering equipment will be further evaluated during the design of the expansion.

A dewatered cake storage area will be provided with a permeable asphalt floor and a pre-engineered clear span roof.



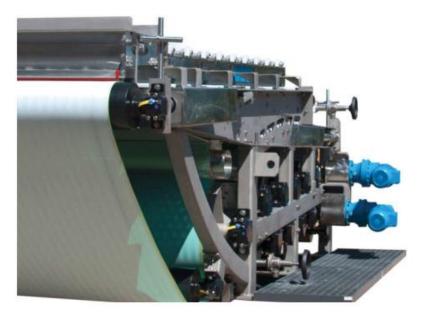
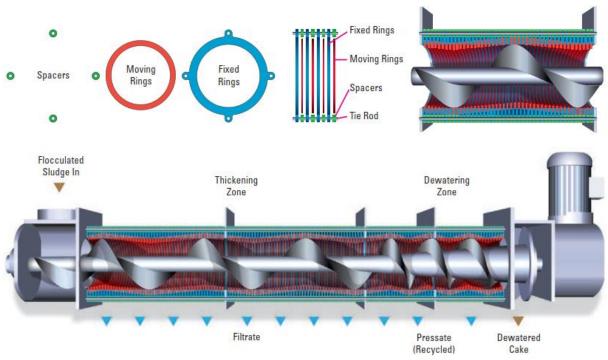


Figure 6.1: Belt Filter Press (Credit: Andritz)



\*Volute is registered with the U.S. Patent and Tradenark Office as a registered trademark of AMCON, Inc., Yokohama, Japan.

### Figure 6.2: Volute Dewatering Press (Credit: Process Wastewater Technologies, LLC)





Figure 6.3: Screw Press (Credit: Schwing Bioset, Inc.)

To expand the biosolids handling process and to house additional equipment, a Dewatering Facility and Covered Cake Storage Facility will be constructed onsite. Refer to **Section 6.3** for site plan layouts for each alternative.

The Dewatering Facility will consist of the following architectural characteristics:

- 4-inch Brick veneer with CMU block bearing wall, with steel roof trusses, and standing seam metal roof.
- Fiberglass reinforced plastic (FRP) doors, windows, frames, etc., as required, with new reinforced concrete slab.
- This building will be an enclosure for covering sludge tanks.
- One bay will be two stories for covering the sludge tanks, and the other bay will be one story for vehicles.

The Covered Cake Storage Facility will consist of the following architectural characteristics:

• Pre-engineered steel portal framed building with standing seam metal roof, with all four sides open to the exterior, over a new reinforced concrete slab.

### 6.1.7 Non-Potable Water System

A non-potable water system will be installed to be used in a variety of applications throughout the WWTP. The non-potable water system will allow for onsite reuse of the treated plant water and subsequent reduction in potable water demand at the WWTP. Currently, the WWTP utilizes potable water for all its water needs. Installation of a non-potable water system will result in a cost savings for plant operation, as well as an increase in efficiency of the WWTP. Refer to **Section 6.5.1** for additional water and energy efficiency considerations for this project.

The non-potable water system will be installed within the Filter and Blower Building. The system will be skid mounted and have an approximate capacity of 200 gpm. The system will draw non-potable water supply from the UV effluent and have non-potable storage in an approximately 5,000-gallon capacity tank. Pumps mounted on the skid will draw non-potable water from the tank and pump to a distribution system throughout the WWTP. Non-potable water can be used for applications such as spray water for the influent screens, pump seal water, wash down, or yard hydrants throughout the WWTP.



# 6.1.8 Laboratory and Administration Building

The current Laboratory and Administration Building was not designed to accommodate the number of current operators. For example, the building does not have a designated office space. Instead desks are placed in the electrical distribution room, and in the entryway. The restroom was designed for single occupancy and is serving as the locker/changing room. In addition, the plant expansion, and the move to more stringent ENR effluent quality will result in an increase in the quantity and type of laboratory tests that are needed to maintain process control.

The expansion project will include the renovation of the existing space including the demolition of the existing laboratory cabinets and restroom. The available space will be re-allocated to provide separate spaces for:

- Laboratory
- Office Space
- Separate locker room with shower and bathroom
- Electrical Distribution and Control Room

The existing Lab Building consists of 4-inch Brick veneer with CMU block bearing walls, with steel roof trusses, asphalt fiberglass roof, and existing hollow metal doors, windows, frames. The existing structure is to remain and be painted as required. The interior spaces will be renovated with new finishes, including acoustical ceilings, painted walls, doors, and frames, casework, fixtures, etc. A roof leak was discovered at a portion of the existing asphalt fiberglass roof while onsite, which will be repaired or replaced as necessary. **Figure 6.4** shows the existing Lab Building.



Figure 6.4: Existing Lab Building

### 6.1.9 Filter and Blower Building

The existing Filter and Blower Building consists of 4-inch Brick veneer with CMU block bearing walls, with steel roof trusses, asphalt fiberglass roof, and existing hollow metal doors, windows, frames. The existing structure is to remain and be painted as required. Interior work will include removal of existing process equipment, expansion of the electrical room, replacement of the existing filters and UV system, expansion of the PACI chemical storage and dosing system, and replacement of the MCC's. New finishes, including acoustical ceilings, painted walls, doors, and frames, casework, fixtures, etc., will be provided. **Figure 6.5** shows the existing Filter and Blower Building.





Figure 6.5: Existing Filter and Blower Building

# 6.1.10 Control Building

The existing Control Building consists of 4-inch Brick veneer with CMU block bearing walls, with steel roof trusses, asphalt fiberglass roof, and existing hollow metal doors, windows, frames. The existing structure is to remain and be painted as required. The interior spaces will be removed and refurbished for other uses. New finishes, including acoustical ceilings, painted walls, doors, and frames, casework, fixtures, etc., will be provided. **Figure 6.6** shows the existing Control Building.



Figure 6.6: Existing Control Building



# 6.1.11 Electrical System Upgrades

The electrical system upgrades proposed herein include all three treatment alternatives, with the exception of the MBR Process Building (see **Section 6.1.11.7**), which would only be required for Alternative 3 (MBR Activated Sludge). The existing electrical loads for the WWTP utilize approximately 25% (peak demand of 101 kW and average demand of 90 kW) of the existing 500 kVA transformer capacity, and the existing incoming electrical equipment in the Filter and Blower Building will not require electrical upgrade. The existing 500kW engine driven generator is sufficiently sized for the planned expansions.

#### 6.1.11.1 Filter and Blower Building

The existing electrical equipment, including an enclosed circuit breaker, automatic transfer switch, panelboards MDP, DP and PA, and low voltage transformer, in the electrical room is in fair condition. However, the existing MCC's in the Blower Room should be replaced with new MCC's. The existing circuit breakers in the existing MCC's located have been overheated and tripped in the summer months even with portable fans blowing directly towards the MCC's. High ambient temperature is the worst enemy for the electrical equipment and shortens the life of the electrical equipment.

Therefore, the existing MCC's should be replaced with new MCC's in a new conditioned space in the Filter and Blower Building to prolong the equipment's life and avoid any nuisance tripping from the heat. The new MCC will be sized per the motor list, shall be bigger than the previous two MCC's, and will consolidate the existing as well as new process loads. All the branch circuits from this MCC will be new with a new feeder circuit from panelboard MDP.

#### 6.1.11.2 Lab Building

The existing electrical equipment in the Lab Building is antiquated and should be replaced with new electrical equipment, including the transformer outside (which has been damaged and moved), switchboard, and Panel PC. Moreover, this equipment is original equipment that was not updated to properly protect the electrical loads/equipment and do not have proper working clearance in accordance with the National Electrical Code (NEC) due to the existing work benches, microwave oven, and refrigerator. All existing feeders and branch circuit wiring in the building should be replaced with new conduit and wires. All new LED lighting and receptacles will also be provided based on the new building layout.

### 6.1.11.3 Pump Building (Old Control/Admin Building)

The existing Panel PD is a relatively new panel in good condition and has proper working clearance. The existing panel will remain. However, the existing feeder from the Lab Building shall be replaced from a new distribution panel.

#### 6.1.11.4 Replacement Influent Screening

The screens are being replaced, and a new control panel complete with variable frequency drives (VFDs), circuit breakers, and controls will be provided outside mounted on a strut frame.

### 6.1.11.5 New Dewatering Facility

A new feeder will be run to this building, and new distribution equipment will be provided, including panelboards and dry type transformers. Electrical fixtures including receptacles, lighting, and switches will also be provided.



### 6.1.11.6 New MBR Process Building

For Alternative 3 – MBR Activated Sludge, a new feeder will be run to the MBR Process Building, and new distribution equipment will be provided, including panelboards and dry type transformers. Electrical fixtures including receptacles, lighting, and switches will also be provided.

### 6.1.11.7 General Site Electrical

The new electrical loads are anticipated to double, and a 150 kVA pad mounted transformer and a 600-amp distribution panel are proposed to accommodate the proposed treatment facility electrical loads and spare capacity. All new feeders and branch circuit breakers shall be properly protected. Site lighting shall be provided per revised layout plan. All outside feeders shall run in underground ductbank system.

In summary, the following electrical upgrades are proposed:

- 1. Existing service is adequate for all three alternatives and shall be retained.
- 2. Replace interior lighting for the whole plant with LED lighting.
- 3. Provide new lighting and controls for new proposed building.
- 4. Provide new LED site lighting for the whole plant.
- 5. Replace existing MCCs with new in the Filter and Blower Building.
- 6. New feeders and branch circuits for proposed upgrades.
- 7. New panelboards, feeders, branch circuits and fixtures for Lab building, MBR Process building and Dewatering Facility.
- 8. Provide new ductbank system.
- 9. Provide new site lighting.

### 6.1.12 Control System Upgrades

To help achieve the operational goals of the advance treatment systems proposed, a centralized Plant Control System (PCS) should be developed to provide the ability for centralized monitoring and supervisory control. Individual processes should be provided with a dedicated PLC control panel that will provide the local control for the individual process equipment and collect process data from local instruments.

Centralized supervisory control would allow operators to interface with the local PLC based control systems that are providing process control at the different process facilities. The distributed nature of this type of system builds reliability into the system by not relying on a single processor to remotely control a process. If there is a failure in communications or a local control panel, the remaining system will continue to operate based on programming and commands issued locally by the dedicated processor. This type of system saves on the installation of conduit and wire by locating the controller near the process area and also allows for the use of less expensive control equipment that has lower total memory and input/output point capability to control a limited scope of equipment.

The PCS system will collect monitored process data from field instrumentation and archive these data in a historian function. The data historian will allow for review of operations through historical trends and creating daily/monthly or annual reports. The automated and centralized collection of these data will facilitate in optimizing the process control resulting in possible savings in energy, chemicals and reduced workloads while providing the data trail to ensure regulatory compliance.

The PCS should be extended to provide remote monitoring of the pump stations, tanks and water treatment plants throughout the town. Similar type of local control panels should be located at these facilities to replace the aged control equipment and to communicate with the centralized PCS. Using the PCS to interface with these other facilities is practical in the sense that it makes full use of the software and hardware that will be purchased for the wastewater treatment plant. These systems are scalable to allow for additional capacity without impeding on the overall efficiency or functions for the plant.



Communications between the WWTP centralized PCS and the remote sites will most efficiently be performed through the use of cellular network technology. Using a third-party cellular provider for remote communications is a low-cost solution that makes use of the providers existing infrastructure and security practices at established low-cost GSA pricing.

# 6.2 Treatment Alternative Upgrades

Three (3) alternatives to upgrade and expand the Centreville WWTP have been developed. The different upgrades that are required for each treatment alternative are detailed below. Preliminary hydraulic profiles for each of the three alternatives is included in **Appendix E**.

# 6.2.1 Alternative 1 – Expand the Sequencing Batch Reactor

### 6.2.1.1 SBR Process

Expanding the existing SBR process from 0.542 MGD to 1.0 MGD to provide the design effluent quality would require the following major scope components:

- Construct a 1.0 MG influent flow equalization basin
- Construct two (2) additional SBR tanks outside of the existing SBR tank structure with floating mixers, removable fine bubble aeration grids, and decant arms.
- Double the capacity of the existing post equalization (post EQ) tank by removing the dividing wall between the existing post EQ tank and sludge holding tank and replacing the equipment.
- Install denitrification filters and a denitrification filter control building and pump station.
- Install three (3) additional blowers in the existing blower room.
- Add storage and handling for the addition of an external carbon source (methanol) to SBR.

Refer to **Section 4.5.1** for a description of the SBR treatment process. **Table 6.5** outlines the design specifications for the SBRs.



Table 6.5: SBR Design Ba	isis	
Parameter	Value	Units
No. of Basins	4	-
Length, each	70.5	Ft
Width, each	53.2	Ft
Volume each at Avg. Side Water Depth	0.47	Million Gallons
Cycles/day	5	Per day/basin
Cycle Duration	4.8	Hr/cycle
Food to Mass Ratio	0.064	Lbs BOD5/lb MLSS-Day
Mixed Liquor Suspended Solids (MLSS) Concentration	4,000	Mg/I at Min. Water Depth
Hydraulic Retention Time	1.17	Days at Avg Water Depth
Solids Retention Time	17.9	Days
Estimated Dry Sludge Produced	1,984	Lbs WAS/Day
Actual Oxygen Required	7,453	Lbs O2/Day
Air Flowrate per Basin	1,670	SCFM

### 6.2.1.2 SBR with Aerobic Granular Sludge

If Alternative 1 is pursued, there is an option to decrease the required SBR tank capacity by installing an AquaNereda® Aerobic Granular Sludge process, manufactured by Aqua-Aerobic Systems, within the SBRs. The AquaNereda® Aerobic Granular Sludge process uses an optimized batch cycle structure with granular sludge to decrease settling time. Therefore, it can operate at higher concentrations, allowing for more treatment capacity within the existing tank volume.

The main benefit of the SBR with aerobic granular sludge option is that only three (3) total SBR tanks would be required, rather than four (4) SBR tanks that are required for the standard SBR Alternative 1. This would increase and optimize the amount of available site space at the WWTP. However, costs of the SBR with aerobic granular sludge option include the cost of the SBR granular sludge equipment, which is estimated to be approximately 25% higher than the cost of the standard SBR Alternative 1 equipment, and there have been limited installation of aerobic granular sludge in the U.S. Therefore, other alternatives were pursued instead.

### 6.2.1.3 Effluent Filtering

Due to the hydraulic limitations the existing cloth media filter will be replaced with a deep bed downflow intermittent backwash sand media filter (IBF). As an additional benefit, with the addition of a carbon source, the IBF will be able to simultaneously denitrify the secondary effluent from the SBR, as well as remove particulate solids.

The IBFs are deep mono media type filters where the influent wastewater flows into the filter by overflowing a weir at the top of the filter. The water flows downward through the sand media, support gravel and underdrain. The treated effluent flows out of the bottom of the filter into the effluent pipeline and to the treated effluent clearwell. The bed is backwashed by pumping water from the clearwell into the bottom of the filter where the underdrain distributes the treated effluent across the filter. The backwash water overflows the influent weir and is discharged, by automatic valves and associated piping, into the mudwell. The biofilm develops on the sand media and sufficient biomass remains in place through the backwash process.



In the denitrifying mode, a carbon source is required. Methanol or glycerin will be added to the filter influent and the nitrate in the influent is converted to nitrogen gas that escapes the process. In the IBF process the removal of solids and excess biomass produced in the denitrifying mode is accomplished through the intermittent backwash of the sand media bed.

In denitrifying mode, the IBF can reliably achieve less than 1.0 mg/L nitrate, even at high influent nitrate concentrations. During maximum month design conditions, the nitrate levels will be less than 12 mg/L, making the IBF a reliable treatment process.

The IBF systems also remove total phosphorus as a result of the particulate solids capture and from a small fraction of biological consumption of soluble phosphorus, typically less than 0.02 mg/L per mg/L nitrate removed. While simultaneous denitrification and phosphorus removal through chemical addition and precipitation can be achieved in the filter, chemical phosphorus removal will be performed upstream of the DN filter stage, i.e. in the SBR or activated sludge process.

The number of filter cells required depends on both hydraulic (peak) flow (relative to filter headloss) and nitrate loading (performance efficiency). Based on the 1.0 MGD design conditions (average and maximum month) the IBF process would consist of three (3) filter cells (144 SF of filtration area per filter cell), for a total of 2,592 CF of active filtration volume.

The IBF filters would consist of concrete above grade tanks with influent channels integrated into the structure.

An online nutrient analyzer will be installed in a small building adjacent to the filters. The analyzer will have centrifugal pumps recirculating flow from the sample points (filter influent and effluent) to the analyzer and back to just downstream of the sample point. One sample pump will draw water from the denitrification filter pump station discharge pipeline (influent). The sample point will be up stream of the methanol injection point. A second sample pump will draw water from the denitrification filter discharge pipeline (effluent).

A control system for controlling the filtration and backwashing operations of three filters will be located in the new building, roughly 28' by 10', including level transmitters, sensors, control panels, analyzers, and a magnetic flow meter (i.e. magmeter). The external carbon storage tank and dosing system will be located adjacent to the IBF. Two (2) submersible backwash pumps (25 HP each), two (2) submersible mudwell pumps (6.5 HP each), and two (2) positive displacement blowers will be provided.

The design specifications for the IBF are shown in **Table 6.6**.



Table 6.6: Denitrifying Filter De	sign Basis	
Parameter	Value	Units
Quantity	3 Total	Unit
Average Design Flow, total	1.0	MGD
Maximum Month Flow, total	1.2	MGD
Peak Flow, total	3.3	MGD
Avg TSS to Filter	30	mg/l
Nitrate and Nitrite to Filter	< 8.0	mg/l
Filtration Area, Each	144	sq feet
Filtration Area, Total	432	sq feet
Filter Media Depth	72	inches
Avg TSS from Filter	< 5.0	mg/l
Nitrate and Nitrite from Filter	< 1.0	mg/l
Hydraulic Loading Rate (average)	< 2.2	gpm/sq feet
Hydraulic Loading Rate (maximum month)	< 3.0	gpm/sq feet
Hydraulic Loading Rate (peak flow)	< 8.0	gpm/sq feet
Backwash Frequency	24	Hrs
Backwash rate	5-6	gpm/sq feet
Backwash Cycle Duration	20-25	min
Backwash Volume	17,280	gallons
Ancillary Equipment	Integrated Controls and Backwash System	-
Basis of Design	elimi-NITE Denitrification System, manufactured by Leopold	-

### 6.2.1.4 Post Equalization Tank

Additional post equalization tank capacity will be required for the expansion. For Alternative 1, the existing sludge holding tank would be converted for additional post equalization tank capacity. The concrete wall that currently separates the existing post equalization tank and existing sludge holding tank would be demolished to effectively double the capacity of the existing post equalization tank. New surface agitators will be installed in the post equalization tank.

### 6.2.2 Alternative 2 – Conventional Activated Sludge

### 6.2.2.1 Activated Sludge Process

Conversion of the existing SBR process to a conventional activated sludge process to provide the design effluent quality, as well as expand the design capacity from 0.542 MGD to 1.0 MGD would require the following major scope components:

- Construct two (2) 5-stage activated sludge basins and two (2) rectangular secondary clarifiers with doublesided weirs.
- Convert the existing SBR tanks to two (2) separate influent flow equalization tanks, as described in **Section 6.1.2**, and convert the existing post EQ tank and the existing sludge holding tank into two (2) aerobic digesters, as described in **Section 6.1.6**.



• Install denitrification filters and a denitrification filter control building and pump station.

Most of the site is steeply sloped and is constrained on all sides from expansion. The rectangular clarifiers can integrate the RAS pumping into the footprint of the clarifiers, and with common wall construction occupy less area compared to circular clarifiers with separate RAS pump stations. Therefore, rectangular secondary clarifiers were selected over the more common circular clarifiers.

**Figure 6.4** depicts a schematic of the 5-stage process. Influent first flows through an anerobic tank, where oxygen devoid conditions are conducive to phosphorus-accumulating organisms to release phosphate into the wastewater, ensuring it is more readily available to be removed in the further stages than if it remained in biomass.

Flow then enters the first anoxic zone where the majority of denitrification occurs. Denitrifying bacteria use nitrate as an electron acceptor to covert its nitrogen through a series of steps, ultimately becoming nitrogen gas.

The water then enters the aerobic zone where nitrification primarily occurs. Through aerobic bacteria, ammonium is oxidized to nitrite and ultimately nitrate, where it is then recycled, called mixed liquor, to the previous stage for denitrification.

Next, the post anoxic zone aids with removing nitrates from the previous aerobic zone that are not recycled back to the first anoxic zone. A carbon source may be supplemented here to aid in nitrification. Methanol is one ideal substance; however, due to Maryland's colder climate, it may only prove effective with attached-growth media. Other carbon sources such as acetate, ethanol, or sugar are suitable carbon sources for ordinary bacteria (methanol requires slow growing methylotrophic bacteria) that can still be effective to cooler climates.

Fine bubble diffusers incorporate air in the final rearation zone to help release any more nitrogen gas that has formed as well as minimize inhibit any more denitrification from occurring in the following secondary clarifiers. This will allow the sludge to settle better and ensure no potential nitrogen gas bubbles form and rise and mixing the water in the process.

As mentioned, water flows to secondary clarifiers following the 5-stage process, where it then flows to a denitrification filter for further nitrogen removal. RAS from the secondary clarifiers is pumped to the Dewatering Facility by submersible pumps within the secondary clarifiers. Design specifications for the denitrification filter are discussed in **Section 6.2.1.3**.

Some key differences between a 5-stage activated sludge process and SBR include that 5-stage activated sludge process will have continuous flow while an SBR delivers flow in batches, which could play a role in treatment efficacy of other treatment technologies like UV disinfection. 5-stage processes are typically favored for nutrient removal, as it can simultaneously remove nitrogen and phosphorus. However, it has a higher footprint and has a higher energy consumption. A schematic overview of the 5-stage process is depicted in **Figure 6.7**.

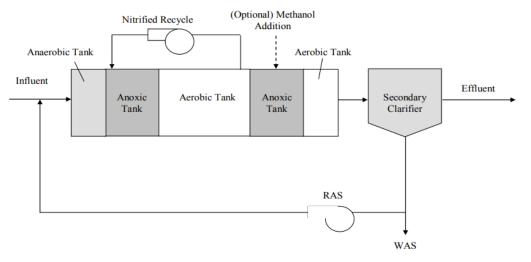


Figure 6.7: Schematic of 5-Stage Process (Credit: EPA)



Design criteria for the 5-stage activated sludge basins and the secondary clarifiers are listed in **Tables 6.7 and 6.8**, respectively.

Table 6.7: 5-Stage Activated Sludge Basin Design Basis						
Parameter	Value	Units				
No. of Trains	2	Trains				
No. of Stages	5	-				
Length of Train, each	95	ft				
Width of Train, each	35	ft				
Average Side Water Depth	18.1	ft				
Volume each at Average Side Water Depth	0.45	Million Gallons				
Solids Retention Time	10-20	Days				
RAS Recycle Ratio	50-100	%				
Internal Nitrate Recycle	300	%				
MLSS Concentration	3000-4000	mg/L				
HRT 1 <sup>st</sup> Zone (Anaerobic)	0.5-1.5 (MMF Design: 1)	Hr				
Anaerobic Zone Working Volume (each train)	25,000	Gallons				
HRT 2 <sup>nd</sup> Zone (Pre Anoxic)	1-3 (MMF Design: 2)	Hr				
Pre Anoxic Zone Working Volume (each train)	50,000	Gallons				
HRT 3 <sup>rd</sup> (Aerobic)	4-12 (MMF Design: 10)	Hr				
Aerobic Zone Working Volume (each train)	250,000	Gallons				
HRT 4 <sup>th</sup> Stage (Post Anoxic)	2-4 (MMF Design: 4)	Hr				
Post Anoxic Zone Working Volume (each train)	100,000	Gallons				
HRT 5 <sup>th</sup> Stage (Reaeration)	0.5-1 (MMF Design: 1)	Hr				
Reaeration Zone Working Volume (each train)	25,000	Gallons				
Total Design HRT	18	Hr				

• HRT = Hydraulic Retention Time

• MMF = Maximum Monthly Flow



Table 6.8: Secondary Clarifier Design Basis						
Parameter	Value	Units				
No. of Clarifiers	2	-				
Length, each	60	ft				
Width, each	35	ft				
Average Side Water Depth	14	ft				
Average SOR	250	gpd/sq feet				
Peak SOR	800	gpd/sq feet				
Average SOR with one Clarifier Offline	500	gpd/sq feet				
Average SLR at 100% RAS	14	Lbs/day/sq feet				
Peak SLR at 50% RAS	35	Lbs/day/sq feet				

• SOR = Surface Overflow Rate

• SLR = Solids Loading Rate

#### 6.2.2.2 Effluent Filtering

Similar to Alternative 1 (SBR Expansion), the existing cloth media filter will be replaced with a deep bed downflow IBF. Refer to the description and design basis in **Section 6.2.1.3** for the proposed filter upgrades.

#### 6.2.3 Alternative 3 – Membrane Bioreactor (MBR) Activated Sludge

#### 6.2.3.1 MBR Process

Conversion of the existing SBR process to a MBR activated sludge process to provide the design effluent quality, as well as expand the design capacity from 0.542 MGD to 1.0 MGD would require the following major scope components:

- Construct a two (2) train 5-stage activated sludge facility, with a larger 5<sup>th</sup> zone to install MBR equipment.
- Construct an MBR Process Building to house blowers and storage/equipment for the addition of an external carbon source (methanol) to the MBR.
- Convert the existing SBR tanks to two (2) separate influent flow equalization tanks, as described in **Section 6.1.2**, and convert the existing post EQ tank and the existing sludge holding tank into two (2) aerobic digesters, as described in **Section 6.1.6**.

The four stages prior to the MBR system typically include anaerobic, preanoxic, aerobic, and postanoxic stages, similar to the 5-stage activated sludge process. The fifth stage includes the MBR system, which uses a membrane filter with a pore size of approximately 1 micron to allow water to pass through while leaving behind the activated sludge. The effluent is pulled through the membranes, which are commonly either a tube or plate style, the resulting permeate has a low turbidity with the excess sludge being removed from the reactor basins. Oftentimes an external carbon source is utilized to aid in nutrient removal like methanol, with other substitutes like ethanol to be evaluated.

This results in a high-quality effluent without the need for a tertiary filter and results in a compact process that also has a longer sludge retention time. Since there is no need for settling, MBR's can also operate at higher mixed liquor suspended solids (MLSS) concentrations.

The membrane limits the hydraulic throughput of the treatment process, so an influent flow equalization tank is required upstream of the MBR to ensure flux through the MBR does not exceed its capacity. MBRs also incur fouling and would need to be cleaned 2-4 times a year with Citric Acid or sodium hypochlorite, although reducing sludge age can reduce fouling.



The hollow fiber membrane units are cleaned in place which can undergo either a maintenance clean 1-2 times a week or more thorough recovery clean occurring twice a year. Maintenance clean leave the train out of operation for around 30 minutes are done without draining the tank with cleaning solutions reversed through the fibers. Recovery cleans has the tank filled with permeate and cleaning solution to be soaked for 6-8 hours, which is then neutralized and drained.

MBR's smaller footprint and high-quality effluent are due to higher volumetric loading rates resulting in lower hydraulic retention times compared to conventional activated sludge (CAS) systems. Although MBRs are energetically more expensive than CAS systems, they have become significantly more efficient in the past 10 years compared to the only slight improvements in CAS technology, becoming 14% less expensive, in relation to CAS systems.

These advancements are due to primarily the reduction in air scour energy for membrane cleaning due to new diffuser technology, greater membrane packing density, decreased maintenance costs, longer operating life, and increased use of gravity permeation from membranes. MBR's higher energy costs are due membrane aeration and permeate pumps that CAS do not have, as well as the cost of RAS pumping being four times higher in the MBRs than in a CAS system.

**Table 6.9** lists the design specifications of the 5-stage activated sludge basins with the MBR system.



Table 6.9: 5-Stage Activated Slue	dge Basin with MBR Design Basis	
Parameter	Value	Units
No. of Trains	2	Trains
No. of Stages	5	-
Length of Train, each	89	ft
Width of Train, each	35	ft
Average Side Water Depth	18.1	ft
Volume each at Average Side Water Depth	0.42	Million Gallons
Solids Retention Time	10-20	Days
RAS Recycle Ratio	50-100	%
Internal Nitrate Recycle	300	%
MLSS Concentration	8000	mg/L
HRT 1 <sup>st</sup> Zone (Anaerobic)	0.5-1.5 (MMF Design: 1)	Hr
Anaerobic Zone Working Volume (each train)	25,000	Gallons
HRT 2 <sup>nd</sup> Zone (Pre Anoxic)	1-2 (MMF Design: 1.8)	Hr
Pre Anoxic Tank Working Volume (each train)	45,000	Gallons
HRT 3 <sup>rd</sup> Zone (Aerobic)	4-8 (MMF Design: 7.2)	Hr
Aerobic Zone Working Volume (each train)	180,000	Gallons
HRT 4 <sup>th</sup> Zone (Post Anoxic)	2-3 (MMF Design: 2.8)	Hr
Post Anoxic Tank Working Volume (each train)	71,000	Gallons
Total Design HRT (including bioreactors and excluding membrane Tanks)	12	Hr
No. of Cassettes per Train	3	Cassettes
No. of Modules installed per Train	132	Modules
Cassette Internal Dimensions L x W x H	21.7' x 9' x 13'	Ft
Reaeration Zone Working Volume (including membranes, each train)	103,000	Gallons
Membrane Surface Area	113,520	Sq ft
Net Flux (Avg Daily)	5.38	gpd/sq feet
Net Flux (Max Monthly)	6.46	gpd/sq feet
Net Flux (Max Daily)	12.92	gpd/sq feet
Net Flux (Peak Hour)	17.76	gpd/sq feet
Hydraulic Peak Flux Rate (Peak Hour)	22.2	gpd/sq feet
Peak Flux Rate Capacity for healthy biological activity (Peak Monthly Flow)	13.9	gpd/sq feet
Basis of Design	Veolia	-
HRT = Hydraulic Retention Time		

HRT = Hydraulic Retention Time
MMF = Maximum Monthly Flow



The MBR Process Building will consist of the following architectural characteristics:

- 4-inch Brick veneer with CMU block bearing wall, with steel roof trusses, and stranding seam metal roof.
- FRP doors, windows, frames, etc., as required, with a new reinforced concrete slab.
- This building will be an enclosure for covering process equipment/blowers, chemical cleaning systems for the membranes, and methanol chemical storage/dosing equipment.

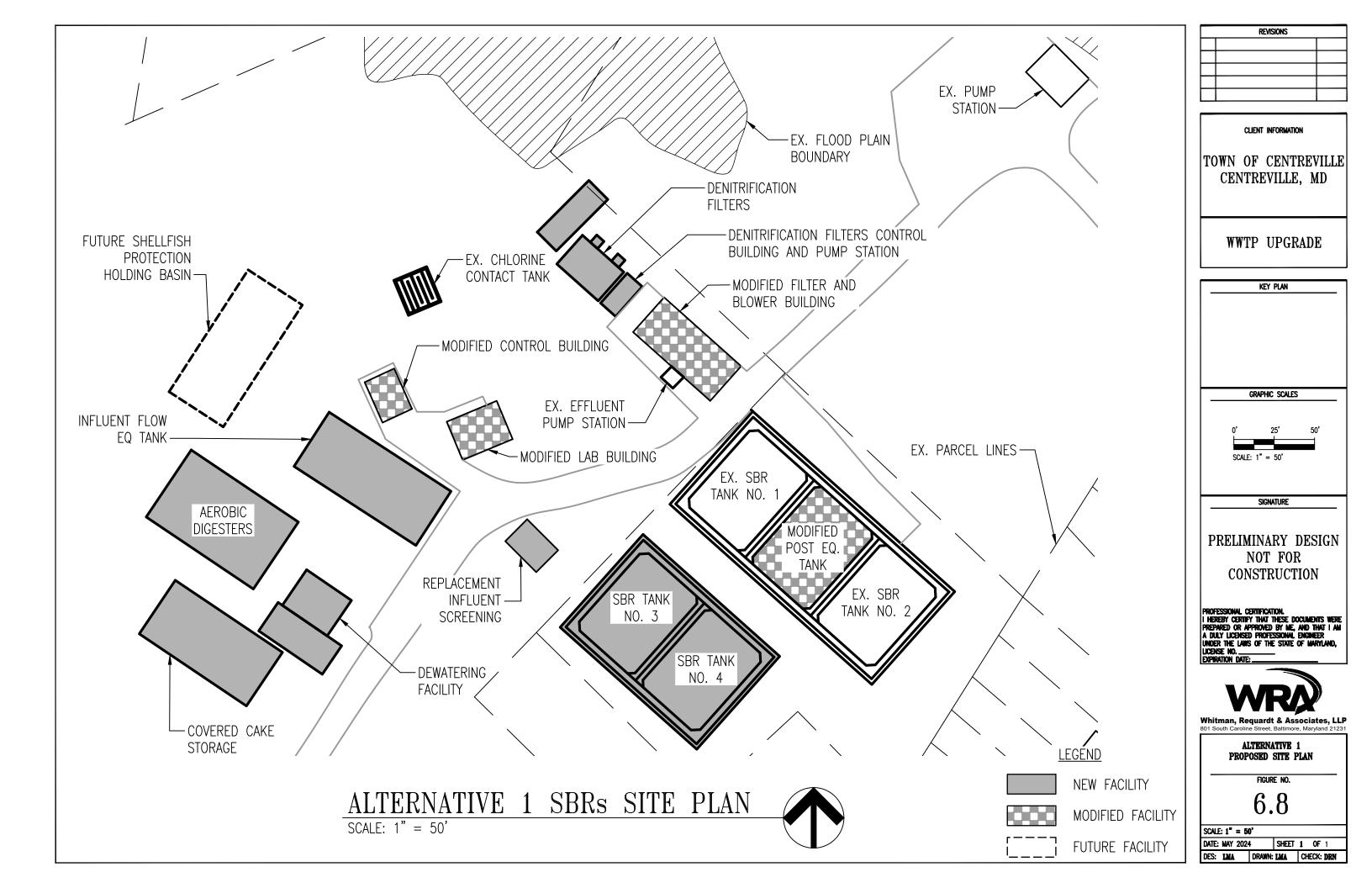
### 6.3 Site Plans and Schematics

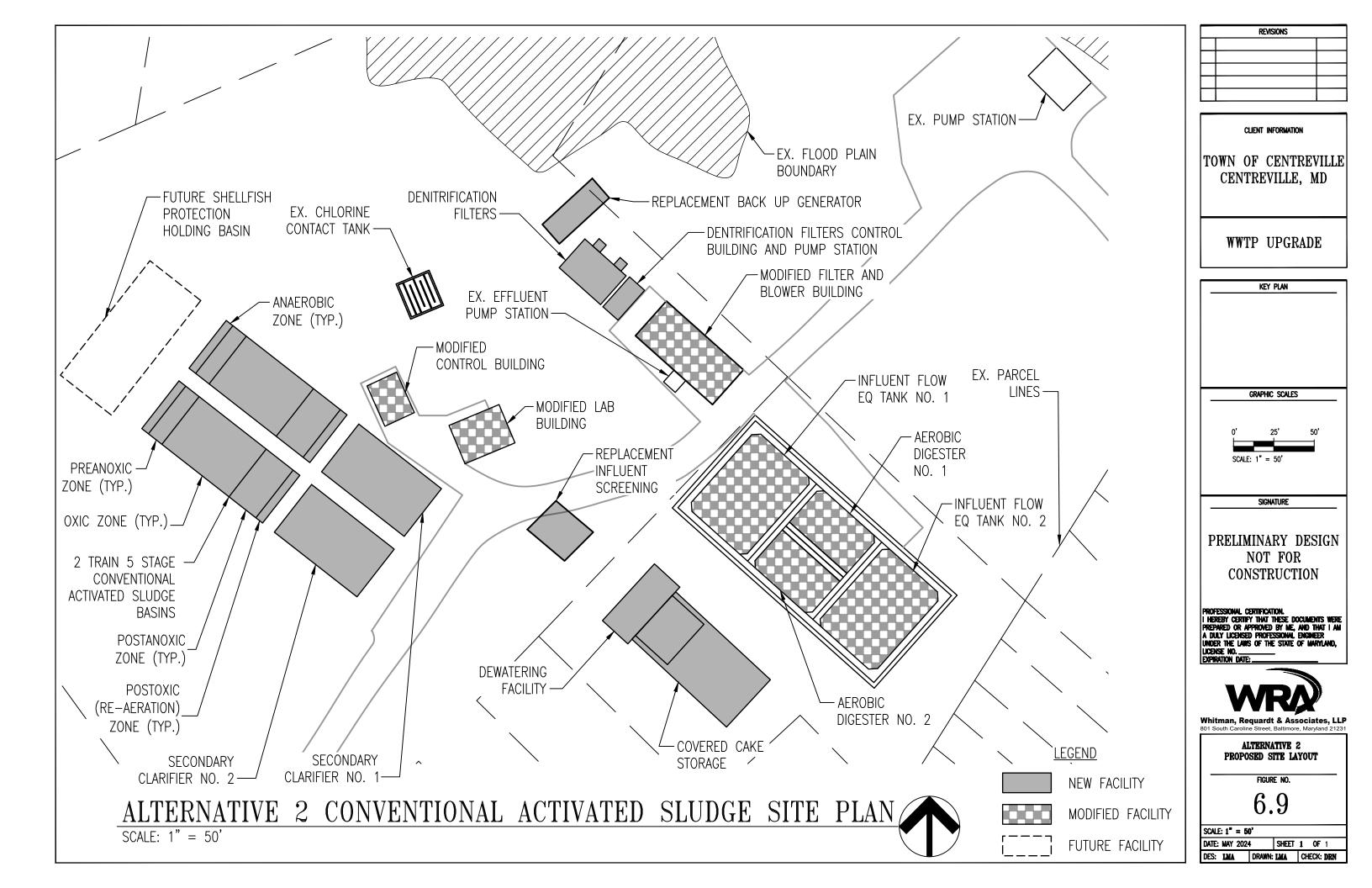
Alternative 1 – SBR will include the post EQ tank expansion, which includes the volume of the existing post EQ and sludge holding tanks. Two additional SBR tanks, adjacent to the current ones, will be constructed. Additionally, denitrification filters will be installed, and their respective Control Building will be built adjacent to the Filter and Blower Building. Finally, the sludge drying reed beds would be transformed to include the influent flow equalization tank, aerobic digesters, Dewatering Facility, and Covered Cake Storage Facility.

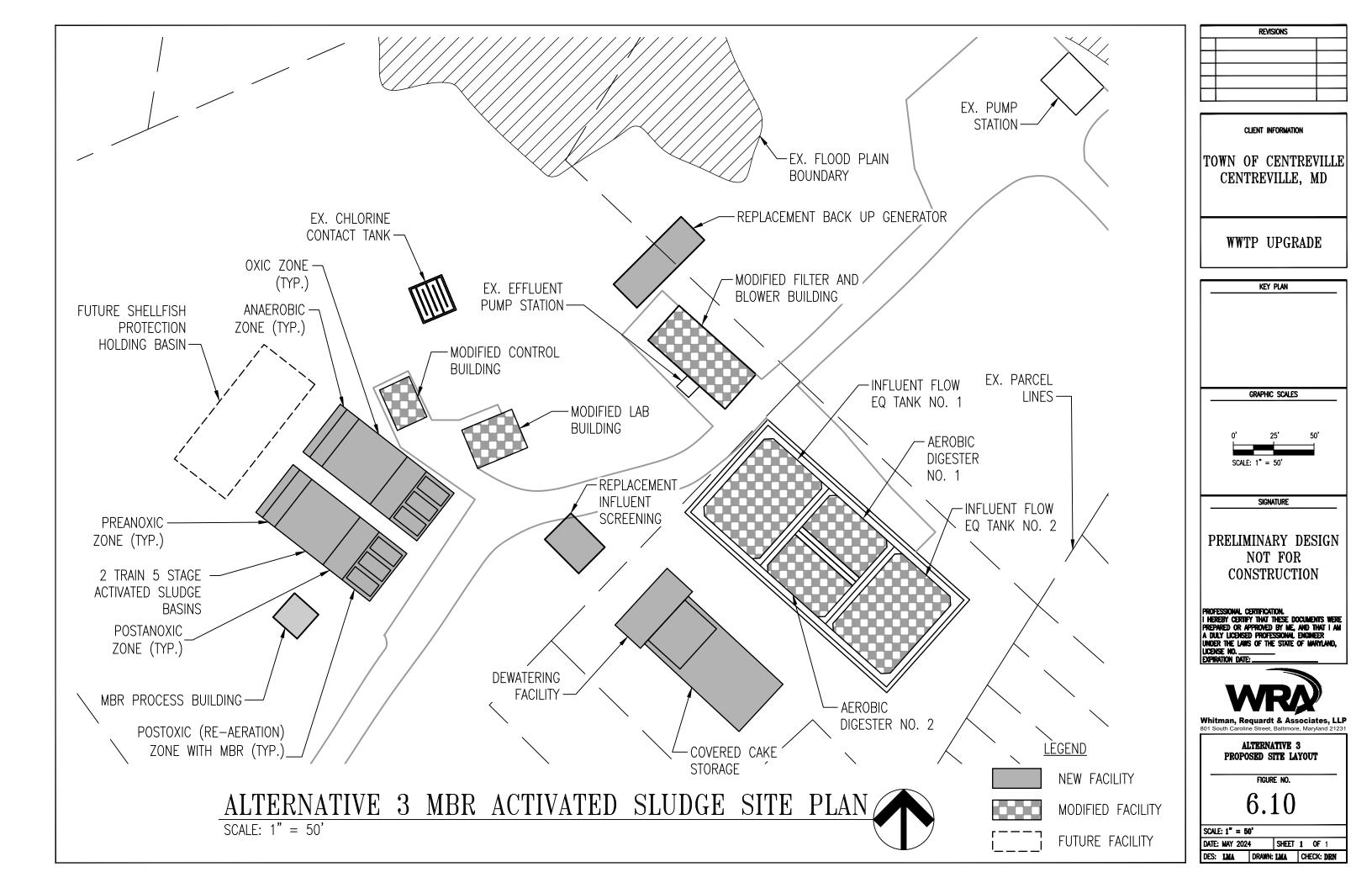
Alternative 2 – Conventional Activated Sludge and Alternative 3 – MBR Activated Sludge would see the existing SBR tanks converted into two (2) influent flow equalization tanks, while the middle tanks would be converted into two (2) aerobic digesters. The Dewatering Facility and Covered Cake Storage Facility will be constructed to the south of the influent screening, influent flow equalization tanks, and aerobic digesters. Across the road, where the existing sludge drying reed beds are located, a 2 train 5-stage activated sludge process would be constructed.

At the end of the train, Alternative 2 will have secondary clarifiers, while Alternative 3 will have the MBR tanks and MBR Process Building, housing the blower and methanol storage and feed equipment. Similar to Alternative 1, Alternative 2 will have denitrification filters installed, and their respective Control Building will be built adjacent to the Filter and Blower Building.

All three alternatives will include replacement of the influent screening, and backup generator, which will all be constructed in the same relative location of the existing facilities, respectively. The Control and Lab Buildings will each be modified, as discussed in **Sections 6.1.8, 6.1.9,** and **6.1.10**. Each site plan also includes reserved area for construction of a shellfish protection tank, if year-round stream discharge into the Corsica River is pursued in the future.









## 6.4 Environmental Impact

Environmental impacts associated with each design alternative are quantified in **Table 6.10** and summarized by alternative below. Alternative 3 - MBR Activated Sludge would have the least environmental impacts compared to both Alternative 1 - SBR and Alternative 2 - Conventional Activated Sludge.

Table 6.10: Environmental Impacts by Design Alternative for Centreville WWTP Site							
Environmental Resource	Alternative 1 (SQFT)	Alternative 2 (SQFT)	Alternative 3 (SQFT)				
Palustrine Emergent (PEM) Wetland	0	0	0				
Palustrine Forested (PFO) Wetland	459	486	0				
Perennial Stream	0	0	0				
Wetland 25-ft Buffer	2,199	2,985	177				
Forest Stands	1,733	5,598	178				
FEMA Floodplain	29	22	0				
Forest Interior Dwelling Bird (FIDS) Habitat	9,190	21,513	13,307				
Tier 2 Catchment	55,137	49,184	47,259				
Chesapeake Bay Critical Area	55,137	49,184	47,259				

### 6.4.1 Alternative 1 – Expand the Sequencing Batch Reactor

Alternative 1 – SBR would impact existing wetlands/wetland buffer, forest, the 100-year floodplain, all located in the northernmost portion of the LOD. There would be impacts to FIDS habitat across the northern, northeastern, and western portions of the LOD. There would also be impacts to the Gravel Run 1 Tier II (High Quality) catchment and CBCA throughout the entire LOD. Alternative 1 would not impact any streams.

### 6.4.2 Alternative 2 – Conventional Activated Sludge

Alternative 2 – Conventional Activated Sludge would impact existing wetlands/wetland buffer, forest located in the northernmost and westernmost portions of the LOD. This alternative would impact the 100-year floodplain located in the northernmost portion of the LOD. There would be impacts to the FIDS habitat throughout the northern and western portions of the LOD. There would also be impacts to the Gravel Run 1 Tier II (High Quality) catchment and CBCA throughout the entire LOD. Alternative 2 would not impact any streams.

### 6.4.3 Alternative 3 – Membrane Bioreactor

Alternative 3 – MBR Activated Sludge would impact existing wetland buffer in the northwestern portion of the LOD. There would be impacts to forests in the northernmost portion of the LOD and the FIDS habitat throughout the northern, northeastern, and western portions of the LOD. This alternative would also the Gravel Run 1 Tier II (High Quality) catchment and CBCA throughout the entire LOD. Alternative 3 would not impact streams or the 100-year floodplain.

### 6.5 Sustainability Considerations

The WWTP upgrade and expansion will be designed to reduce its impact on the environment and to be resilient to future changes in the climate as indicated in this section.



### 6.5.1 Water, Chemical and Energy Efficiency

The existing WWTP utilizes potable water for all its water needs. The upgrade will include an onsite non-potable water system to utilize treated effluent for the process related water needs.

The treated effluent water quality will be sufficient to meet off-site Class III and IV reclaimed water requirements. Potential future off site water reuse includes irrigation of the Queen Anne County recreational fields located adjacent to the Town.

In addition to the onsite water reuse, the potable water use onsite will be reduced through the replacement of the existing plumbing fixtures with low flow.

Energy efficiency will be considered for the selection of lighting and equipment for the project. Examples of improved energy efficiency include:

- The existing florescent tube and halogen lights will be replaced with LED lights. New lights will only be LED.
- All equipment will use high efficiency motors.
- The UV disinfection system will have the latest generation of UV intensity measurement and lamp controller.
- Pumps will have variable frequency drives (VFD) to operate at optimal speeds.
- New process blowers will be high efficiency turbo blowers.
- Dewatering equipment will consider slow speed, low energy demand type equipment.

The primary chemical consumptions include external carbon for denitrification and metal salt for phosphorus removal and are similar for all three alternatives. Alternative 3 has additional cost related to the use of acid and bleach for periodic membrane cleanings. Labor costs and electrical power costs are also slightly higher for Alternative 3 however differences are not significant. The major operating costs (maintenance, energy, chemical and labor) are included in **Table 7.5**.

#### 6.5.2 Green Infrastructure

The three alternatives will have similar opportunities for incorporating green infrastructure as deemed practical. As an example, the roof cover over the dewatered biosolids storage area will be designed to accommodate the future installation of solar PV cells. The solar PV cells will be connected to the utility electric grid to offset the electricity used by the WWTP. There may be other areas on the site that could accommodate additional solar PV cells.

#### 6.5.3 Climate Related Considerations

The upgrade and expansion of the WWTP is required to protect the receiving stream and the environment from wastewater that does not meet the discharge permit requirements. Without an expansion of the treatment capacity, the likelihood of future process upsets increases with the increase in influent flows stressing the capabilities of the existing system.

The new facilities will be constructed to protect them from a 100-year flood with 3 feet of additional protection provided. New structures will have a finished floor or top of wall of at least 3 ft above the 100-year flood elevation.

By selecting Alternative 3 – MBR Activated Sludge, the proposed facilities would have the smallest footprint of the alternatives considered. Therefore, the facilities would have a smaller impact to the site and can be located to reduce their impact on environmentally important features such as the wetlands and forested areas.



## 6.6 Cost Estimates

A conceptual cost estimate was developed for each of the three treatment alternatives that are being considered. The cost estimates were developed using preliminary equipment supplier quotations based on the design concepts described in this preliminary engineering report. The cost estimate does not include expansion of the effluent discharge, such as the cost of extending the outfall into the Corsica River or the cost to acquire and spray discharge to additional irrigation sites.

The cost estimates for each of the treatment alternatives were developed using the expertise and experience of the WRA engineers. The cost estimates presented represent WRA's best engineering judgement and assumes that competitive bids are received. However, the unpredictability of the current market should be taken into consideration when the project goes to bid. The estimates were prepared in accordance with AACE Class 4 Budgetary (planning-level) construction cost requirements. All costs are presented in 2023 dollars and will need to be indexed using the annual inflation rate. Contingency cost, an allowance that reflects the uncertainty associated with a construction cost opinion based on a "predesign" study of the indicated facilities, is included as a 30% markup in the estimate. Additionally, an escalation markup of 4% per year is also included in the estimate.

The conceptual cost estimates for each alternative are presented in **Table 6.11**. Refer to **Appendix A** for detailed breakdown.



Table 6.11: Conceptual Construction Cost Estimates for Treatment Alternatives			
	Alternative 1: SBR	Alternative 2: Conventional Activated Sludge	Alternative 3: MBR Activated Sludge
Base Facilities			•
Interior Demolition (Lab, Control, and Filter and Blower Buildings)	\$ 95,000	\$ 95,000	\$ 95,000
Influent Screening Expansion	\$ 825,000	\$ 825,000	\$ 825,000
Methanol Facility	\$ 618,000	\$ 618,000	\$ 618,000
Non-Potable Water System	\$ 54,000	\$ 54,000	\$ 54,000
Dewatering Facility	\$ 2,413,000	\$ 2,413,000	\$ 2,413,000
Covered Cake Storage Facility	\$ 835,000	\$ 835,000	\$ 835,000
Lab Building Refurbishment	\$ 139,000	\$ 139,000	\$ 139,000
Control Building Refurbishment	\$ 130,000	\$ 130,000	\$ 130,000
Filter and Blower Building Refurbishment	\$ 348,000	\$ 348,000	\$ 348,000
Base Subtotal Cost	\$ 5,457,000	\$ 5,457,000	\$ 5,457,000
Facilities for ENR Alternatives			
Influent Flow EQ Tank(s), Aerated, with Pumping	\$ 2,054,000	\$ 2,019,000	\$ 2,019,000
Existing Tank Modifications	\$ 214,000	\$ 643,000	\$ 643,000
Clarifier Tanks, Equipment, and RAS Pumps	-	\$ 4,749,000	-
Denitrification Filter Tanks, Equipment and Controls	\$ 3,112,000	\$ 3,112,000	-
Miscellaneous Process Piping and Equipment	\$ 157,000	\$ 235,000	\$ 784,000
Additional SBR Tanks, Equipment and Controls	\$ 3,564,000	+ _00,000	÷ · · · .,••••
Activated Sludge Equipment	-	\$ 1,012,000	_
MBR Process Equipment and Controls, including MBR Process Building	-	¢ 1,012,000	\$ 5,789,000
Post EQ Tank and Equipment	\$ 78,000	-	¢ 0;1 00;000
UV Disinfection System	\$ 642,000	\$ 642,000	\$ 642,000
Aerobic Digester Tank and Equipment	\$ 1,427,000	\$ 78,000	\$ 78,000
Alternative Subtotal Cost	\$ 11,248,000	\$ 12,490,000	\$ 9,955,000
Alternative Plus Base – Subtotal Construction Cost	\$ 16,705,000	\$ 17,947,000	\$ 15,412,000
Electrical	\$ 2,517,000	\$ 2,722,000	\$ 4,169,000
Site Civil, including Yard Piping and Demolition (15% Alternative + Base)	\$ 2,506,000	\$ 2,692,000	\$ 2,312,000
Site SCADA (5% Alternative + Base)	\$ 835,000	\$ 897,000	\$ 771,000
Subtotal	\$ 22,563,000	\$ 24,258,000	\$ 22,664,000
Guniolai	φ 22,303,000	φ <b>24,230,000</b>	φ 22,004,000
Contingency (30%)	\$ 6,769,000	\$ 7,278,000	\$ 6,799,000
Escalation to December 2026 (4%/year)	\$ 3,662,000	\$ 3,938,000	\$ 3,678,000
Grand Total Construction Cost	\$ 32,994,000	\$ 35,474,000	\$ 33,141,000



## 6.7 Design Criteria

Table 6.12: Influent Basis of Design						
Parameter	Units	Annual Average	Max Month	Max Day	Peak Inst.	Start Up Min Day
Flow	MGD	1.0	1.2	2.4	3.2	0.2
Wastewater Temperature	Degrees C	20	12			
Biochemical Oxygen	mg/l	130	156			
Demand	lbs/day	1,084	1,561			
Total Suspended	mg/l	145	174			
Solids	lbs/day	1,209	1,741			
Total Kjeldahl	mg/l	35	42			
Nitrogen	lbs/day	292	420			
Tatal Dhasabanus	mg/l	8	8			
Total Phosphorus	lbs/day	67	80			

The influent basis of design flows and loads for the upgrade and expansion are included in **Table 6.12**.

The flow peaking factors were developed based on the MDE Design Guidelines for Wastewater Facilities, with the exception of the monthly peaking factor. The design uses a peaking factor of 1.2, and the MDE design guidelines recommend a peaking factor of 1.6 for a 1 MGD facility. The upgrade and expansion of the Centreville WWTP is designed to provide full treatment during the maximum month flows and loads, during cold weather conditions, with one treatment train offline. Considering that Centreville will only have two MBR trains, sizing one train to treat the maximum monthly flow and loads at the MDE recommended monthly flow peaking factor of 1.6 would oversize the facilities for start-up conditions, and thereby requiring only one train to be operated at a time to be efficient. During final design a maximum monthly flow peaking factor of 1.3 will be considered and determined if it would represent a de minimis increase in construction costs and have a small impact on the operability of the facility during start-up. If so, the design basis will be adjusted.

The influent total suspended solids (TSS) and biological oxygen demand (BOD) were sampled and analyzed in 2017 and again in 2023 as presented in **Appendix C**. The TSS measurements were highly variable, with a standard deviation nearly as large as the average. In addition, the wastewater biological and chemical computer modeling software being used, BioWin by EnviroSim, requires the TSS concentration to be higher than the BOD concentration to allow for valid calculations. Therefore, the TSS concentrations were adjusted to be in line with the BOD values.

The design basis influent total phosphorus concentration selected utilizes the maximum daily composite sample from the 2017 and 2023 sampling and analysis. Again, the limited samples that were taken and analyzed resulted in a large distribution of values. The chemical dosing system will be designed to meet maximum month influent loading. Using a conservative value for phosphorus concentration will allow the system to reliably meet the



relatively low ENR effluent phosphorus concentration of 0.15 mg/L as proposed by MDE if year-round stream discharge is selected.

The design effluent quality basis of design for the project are summarized in **Table 6.13**. Each alternative must meet the effluent basis of design.

Table 6.13: Effluent Basis of Design							
Parameter	Units	Annual Monthly Average	Max Month				
Biochemical Oxygen Demand	mg/l	<10	<10				
Turbidity <sup>1</sup>	NTU	<2	<5 any time				
Total Suspended Solids	mg/l	<10	<10				
Total Nitrogen	mg/l	<3	<3				
Total Phosphorus <sup>2</sup>	mg/l	<0.15	<0.15				
E. Coli <sup>1</sup>	MPN / 100 mL E. Coli Monthly Median	1	23				

<sup>1</sup> Class IV Reclaimed Water Requirements, <sup>2</sup> Requirement with relocated outfall to Corsica River

### 6.8 Land Requirements

The WWTP upgrade and expansion will be constructed on developed land owned by the Town. The land disturbance at the WWTP will vary depending on which alternative is selected for the upgrade. Alternative 1 (SBR) would require the largest footprint at the WWTP to construct, while Alternative 3 (MBR) would require the smallest footprint. Refer to **Section 6.3** for proposed site plans for each of the three alternatives.

Future expansion of the spray irrigation system would require at least 300 acres of suitable land located near the Town. The identification, testing, and development of the field will be considered separately from the WWTP upgrade.

### 6.9 Potential Construction Issues

The construction of the three alternatives will have potential construction issues that need to be identified, the risks understood, and mitigation plans developed. Based on experience and knowledge of the site, an initial list of specific construction related issues and methods to mitigate the risk have been developed as summarized in **Table 6.14**.

In order to maintain plant operations during construction, both existing SBRs are required to remain online until the selected treatment process is constructed and put into service. For Alternative 2 – Conventional Activated Sludge and Alternative 3 – MBR Activated Sludge, this requires the 2 train 5-stage activated sludge basins to be constructed and put into service before either of the SBR tanks are converted into influent flow equalization tanks. The proposed site layouts for each of the three alternatives, described in **Section 6.3**, allow for maintenance of plant operations during construction.



Table 6.14: Summary of Potential Construction Issues						
Issue	Risk	Planned Mitigation Methods				
Encountering groundwater during excavations	Dewatering excavation expense	Plan for thorough soil borings and geotechnical investigations early during design				
Encountering unidentified underground piping and structures	Change in scope during construction	Review all available information. During design conduct subsurface investigation and test pitting where there are potential obstructions				
Product and equipment delivery longer than expected	Delay in construction schedule	Realistically estimate delivery times based on estimates from named manufacturers and experience with other projects and keep in contact with key manufacturers during construction				
Integration of manufacturer supplied control panels with the plant SCADA	Insufficient process data relayed to the SCADA from the manufacturer's control panels	Complete process and instrumentation diagrams, Input/Output lists and control descriptions will be included in the Contract Documents				
Level of automation that operations can maintain	Automation is too complex for operations to troubleshoot and maintain	Conduct workshops with operations to custom tailor the control system and the level of automation with the needs and skills of operations				
Treatment process testing	Assessing the treatment process under design conditions	Contract Documents will include a 30 day testing period of the complete treatment plant with a requirement to operate the plant with equipment and treatment trains offline to simulate design conditions. Testing conditions, sampling and requirements for the system passing the testing will be included.				
Turn over of treatment facilities	In complex upgrade projects some treatment facilities will be brought online before substantial completion	The definition of substantial completion for individual facilities and major equipment, and the responsibilities for the Owner and Contractor between facility substantial completion and final completion will be clearly defined in the specifications.				

The design phase will discover additional potential construction issues and where practical the Contract Documents will identify the area the Installing Contractor should be aware of.

Construction risks specific to Alternative 1 – SBRs include the deep structure construction of the SBRs in close proximity to the adjacent residences. Additionally Alternative 1 has the largest overall disturbed area, resulting in the greatest risk to encountering unknown obstructions. Therefore Alternative 1 has the largest risks during construction.

Construction risks associated with Alternative 2 – Conventional Activated Sludge include a large area of disturbance and the most number of new structures. One advantage of Alternative 2 to Alternative 1, and Alternative 2 avoids the deep construction adjacent to residences.

Construction risks associated with Alternative 3 – MBR are less than either of the two other options, as it avoids deep construction adjacent to residences, it also represents the fewest number of structures and has the smallest disturbance footprint.



# 7 Alternative Evaluation

## 7.1 Effluent Water Quality Comparison

Each of the three alternatives are capable of meeting the treatment and capacity goals for this project. To verify this, the expected secondary effluent water quality of each alternative was evaluated by modeling each of the three treatment alternatives using BioWin software. Each treatment alternative was modeled under average flow conditions and maximum monthly (simulating wet weather) flow conditions.

**Table 7.1** shows the different influent conditions that were used for modeling average conditions versus maximum monthly conditions for each alternative. These influent conditions are based on composite influent sampling data of the WWTP conducted in September/October 2017 and in March 2023, which is included in **Appendix C**. Note that influent sampling data was limited since it is not conducted on a regular basis.

Table 7.1: BioV	Table 7.1: BioWin® Model Influent Conditions						
Condition	Flow	Temperature	Biochemical Oxygen Demand (BOD)	Volatile Suspended Solids (VSS)	Total Suspended Solids (TSS)	Total Kjeldahl Nitrogen (TKN)	
	(MGD)	(Deg C)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	
Average	1.0	20	130	116	145	35	
Maximum Monthly	1.2	12	156	139	174	42	

Additional influent conditions were assumed in the model which did not change between average and maximum monthly model runs. These conditions include:

- Total Phosphorus = 8.0 mg/L
- Total Sulfur = 10 mg/L
- Nitrate = 0 mg/L
- pH = 7.3
- Alkalinity = 6.0 mmol/L

As stated in **Section 2.3.1**, the treatment quality goals for ENR include a TN concentration below 3.0 mg/L and a TP concentration below 0.3 mg/L. The reduction in TP at Centreville WWTP will depend on upgrades to the existing chemical dosing system and amount of PACI added to precipitate phosphorus. PACI chemical dosing was not modeled in BioWin. Therefore, this modeling study focused on comparing TN reduction in the biological treatment process for each of the three treatment alternatives.

**Table 7.2** lists the biological treatment quality parameters that were monitored in the secondary effluent in BioWin, as well as the target concentrations for each of the three alternatives in order to meet the effluent quality goals of this project. The parameters that were monitored include concentrations of MLSS, cBOD, TSS, ammonia, filtered TKN, and nitrate + nitrite.



Table 7.2: Target Secondary Effluent Water Quality Parameters						
Biological Treatment Parameter	Alternative 1 (SBR) and Alternative 2 (Conventional Activated Sludge)	Alternative 3 (MBR Activated Sludge)				
MLSS	<= 4,000 mg/L	<= 8,000 mg/L				
cBOD	<= 30 mg/L	<= 2 mg/L				
TSS	<= 20 mg/L	Non-detect				
Ammonia	< 1.0 mg/L	< 1.0 mg/L				
Filtered TKN	< 2.0 mg/L	< 2.0 mg/L				
Nitrate + Nitrite	< 8.0 mg/L	< 1.0 mg/L				

As shown in **Table 7.2**, Alternatives 1 and 2 have the same target effluent quality concentrations with respect to nitrate (/nitrite) and rely on the downstream denitrification filters to complete the nitrogen removal, although Alternative 2 can also incorporate methanol addition in the post-anoxic zone for enhanced nitrogen removal and use the tertiary filters for final solids removal only. For Alternative 3 all nitrogen removal is within the MBR process tankage as there is no additional downstream removal process, and is facilitated by methanol addition within the MBR secondary anoxic zone to reduce nitrate + nitrite concentrations below 1 mg/L.

All three alternatives require ammonia concentrations to be below 1 mg/L. MBRs can typically operate at higher MLSS concentrations compared to conventional activated sludge clarifiers, which is why Alternative 3 has a higher allowable MLSS concentration.

**Tables 7.3 and 7.4** show the secondary effluent water quality results from modeling at both average and maximum monthly conditions.

Table 7.3: Secondary Effluent Water Quality BioWin Modeling Results – Average Conditions							
Condition	Biological Treatment Parameter	Units	Alternative 1: SBR Expansion	Alternative 2: Conventional Activated Sludge	Alternative 3: MBR Activated Sludge <sup>(1)</sup>		
	Flow	MGD	1.0	1.0	1.0		
	MLSS	mg/L	3,600	2,500	5,100		
	cBOD	mg/L	6.0	2.4	1.0		
	TSS	mg/L	20	9	0		
Average	Ammonia	mg/L	0.30	0.12	0.06		
	Filtered TKN	mg/L	1.3	1.4	1.2		
	Nitrate	mg/L	2.9	4.0	0.08		
	Nitrite	mg/L	0.05	0.03	0.01		
	Total Nitrogen	mg/L	4.6	5.6	1.4		

<sup>(1)</sup> – Note that Alternative 3 (MBR Activated Sludge) modeling includes 75 gpd of methanol addition in the postanoxic zone.



Table 7.4: Secondary Effluent Water Quality BioWin Modeling Results – Maximum Monthly Conditions							
Condition	Biological Treatment Parameter	Units	Alternative 1: SBR Expansion	Alternative 2: Conventional Activated Sludge	Alternative 3: MBR Activated Sludge <sup>(1)</sup>		
	Flow	MGD	1.2	1.2	1.2		
	MLSS	mg/L	3,800	3,600	7,500		
	cBOD	mg/L	10	3.5	0.9		
Maximum	TSS	mg/L	20	14	0		
Monthly	Ammonia	mg/L	0.40	0.30	0.17		
Monuny	Filtered TKN	mg/L	1.5	1.7	1.5		
	Nitrate	mg/L	1.4	5.2	0.70		
	Nitrite	mg/L	0.8	0.1	0.04		
	Total Nitrogen	mg/L	4.1	7.3	2.4		

<sup>(1)</sup> – Note that Alternative 3 (MBR Activated Sludge) modeling includes 75 gpd of methanol addition in the postanoxic zone.

The secondary effluent quality of all three alternatives meets all of the target quality parameters listed in **Table 7.2**. The BioWin modeling results for filtered TKN concentrations are below 2 mg/L; however, the historical average TKN concentrations from 2014-2022, as listed in **Section 4.4**, are below 1 mg/L. It is assumed that BioWin is not properly accounting for the biodegradable portion of TKN, which is why filtered TKN concentrations are reporting unusually high in the models. In conclusion, BioWin modeling of each of the three treatment alternatives confirms that each alternative is capable of meeting the treatment and capacity goals for this project.

## 7.2 Life Cycle Cost Analysis

A life cycle cost analysis was performed on the three alternatives. A life cycle cost analysis combines the initial capital cost with the net present value of the operating costs across the expected life of the project into a present worth total. The life cycle cost analysis provides a more complete picture of the costs of the project than just the capital cost.

For the life cycle cost analysis, the electrical loads for the major equipment are multiplied by the percentage of time per year the equipment is expected to be running. Equipment with variable speed drives and variable loads are calculated using the expected annual average load.

Labor for each alternative was compared to the existing cost of labor for the current WWTP and extrapolated to consider the increased complexity of the upgrade as well as the increased size of the plant to treat the expanded flows.

The chemical costs indicated in **Table 7.5** under 'Annual Chemical Costs' are the estimated costs of methanol to drive the denitrification process and the addition of PACI to precipitate phosphorus. The design average influent nitrogen and phosphorus and the goals for effluent nitrogen and phosphorus concentrations are used in the calculations at an annual average influent flow of 1.0 MGD.

As noted in the **Table 7.5** footnote, the membrane cleaning chemical costs are included in the 'Annual Maintenance/Repairs Costs' for the Alternative 3 – MBR Activated Sludge. The 'Annual Maintenance/Repair Costs' also includes the annual contribution to replacement of the membranes every ten years.

For the life cycle cost analysis the project is assumed to have no salvage value at the end of the 20 years.



Table 7.5: Life Cycle Cost Analysis			
	Alternative 1 – SBR Expansion	Alternative 2 – Conventional Activated Sludge	Alternative 3 – MBR Activated Sludge
WWTP Capital Cost	\$32,994,000	\$35,474,000	\$33,141,000
Operating Cost			
Annual Maintenance/Repair Costs <sup>(1)</sup>	\$133,076	\$164,841	\$226,647
Annual Electric Cost	\$ 55,157	\$77,528	\$84,877
Annual Burdened Labor	\$384,800	\$395,200	\$499,200
Annual Chemical Costs	\$336,886	\$336,886	\$336,886
Operating Cost Subtotal	\$909,919	\$974,455	\$1,147,609
Real Discount Rate <sup>(2)</sup>	2%	2%	2%
Project Life, years	20	20	20
Operating Cost Present Value	\$14,880,000	\$15,930,000	\$18,770,000
Present Worth	\$47,874,000	\$51,404,000	\$51,911,000

(1) Maintenance estimated at 2% of equipment cost for Alternatives 1 and 2 and 2.5% for Alternative 3 to account for membrane replacement and cleaning chemicals

(2) December 2022 OMB Circular No. A-94

The life cycle cost analysis results in the present worth ranging from approximately \$48 million for Alternative 1 up to \$52 million for Alternative 3. The results are within 8% of each other. Considering the variability in estimating construction and operating costs, the three alternatives are similar in life cycle costs.

### 7.3 Non-Monetary Evaluation

Life cycle costs include items of each alternative that have a dollar value. The value of the project will also be influenced by factors that do not have a direct cost measure. The following criteria for the non-monetary evaluation have been defined based on feedback from the Town of Centreville

- Leverages operators experience
- Operational simplicity
- Ease of maintenance
- Public acceptance
- Compatibility with water reuse
- Ability to evolve with future technologies
- Ability to upgrade treatment process in the future
- Available site space for future improvements

The non-monetary evaluation criteria are categorized and weighted as described below.

#### Leverages Operators Experience

Operators currently at the Centreville WWTP have experience with operating the existing two-tank SBR system. This criterion evaluates the complexity in training operators for each of the three treatment alternatives. This criterion was assigned a weighting factor of 5%, due to significantly different levels of training that would be required for each of the three alternatives.

#### Operational Simplicity

Each of the three alternatives will have different day-to-day involvement for operators and will require different levels of attention to maintain operation. Because some of the alternatives are more complicated to operate, this criterion was assigned a weighting factor of 5%.



#### Ease of Maintenance

The equipment for each treatment alternative must be accessible for maintenance once it is placed into service. Equipment must also be reliable and have a low likelihood of experiencing upsets and discharging unacceptable effluent, so maintenance may be kept at a minimum. This criterion considers each alternative's reliability, how often the equipment for each alternative will require servicing, as well as the availability of replacement parts and how complex the equipment is to maintain. This criterion has been assigned a weighting factor of 10%.

#### Public Acceptance

This category considers the public view and acceptance of the three treatment alternatives. The upgrade and expansion of the Centreville WWTP has been presented to various public groups and stakeholders within the Centreville area, and the public perception and feedback received from these groups is considered in this criterion. Alternatives that received more positive feedback will be given a higher score. This criterion has been assigned a weighting factor of 10%.

#### Compatibility with Water Reuse

The Town of Centreville plans for future potable water reuse using the WWTP effluent. This criterion evaluates how each treatment alternative positions the Town to move towards potable water reuse in the future. This includes how much expansion to the treatment process will be required in the future to meet the effluent quality levels required for potable water reuse. This criterion has been assigned a weighting factor of 10%.

#### Ability to Evolve with Future Technologies

It is important to the Town that the Centreville WWTP is positioned to incorporate future treatment technologies and remain on the forefront for wastewater treatment. This category considered how flexible each treatment alternative is for incorporating future technologies. Because this criterion has a high importance to the Town, it has been assigned a weighting factor of 20%.

#### Ability to Upgrade Treatment Process in the Future

Federal and state regulatory agencies may establish more stringent effluent quality requirements in the future—therefore, this category considers how each treatment alternative can be upgraded in the future to meet higher effluent quality. This criterion has a high importance to the Town and therefore has been assigned a weighting factor of 20%.

#### Available Site Space for Future Improvements

Future expansion and development in the Town of Centreville will require additional expansion of the WWTP beyond the planned 1.0 MGD capacity. This category considers how much site space will be available after construction of each treatment alternative. It is desired to maintain as much site space as possible for future expansion efforts. Because of this, this criterion has been assigned a weighting factor of 20%.

Each of the evaluation criteria noted above have been weighted to reflect their relative importance to the construction and operation of the treatment process. The weighting factors were discussed with the Town staff and represent the consensus opinion. Each alternative was assigned a score based on a scale of 1 to 10, with 1 being the lowest or least desirable and 10 being the highest or most desirable for a given criterion. The total score for each category was then determined by multiplying the individual criteria scores by the assigned weight, and then summing up the weighted scores. **Table 7.6** presents the final criteria ranking tabulation.



Weight %		5		5		10		10		10		20		20		20		
		rages Operators Experience	Opera	tional Simplicity	ty Ease of Maintenance		Public Acceptance			patibility with ater Reuse		ty to Evolve with ire Technologies	Ability to Upgrade Treatment Process in the Future		Available Site Space for Future Improvements		Total Score	Ranl
	Score	Comments	Score	Comments	Score	Comments	Score	Comments	Score	Comments	Score	Comments	Score	Comments	Score	Comments		
Alternative 1: SBR Expansion	10	Operators are highly familiar with the existing SBR technology, which is currently used at the WWTP.	10	The SBR process has a relatively low complexity for operators.	10	The SBR process requires minimal maintenance of equipment.	8	The SBR process is viewed favorably by the public, but is seen as an older, conventional treatment process.	10	The SBR process will provide effluent quality suitable for future water reuse.	7	The SBR tanks are adaptable and can be modified to accommodate other technologies, such as aerobic granular sludge.	4	This alternative requires the most amount of site space. It will be the most challenging to further upgrade the future treatment capacity.	4	Requires relatively high amount of site disturbance.	680	3
Alternative 2: Conventional Activated Sludge	8	Operators will be able to use knowledge of the existing SBR process to learn the new conventional activated sludge process.	6	The activated sludge process is moderately complex for operators.	8	The activated sludge process requires relatively low maintenance equipment.	8	The activated sludge process is viewed favorably by the public, but is also an older conventional treatment process.	10	The activated sludge process will provide effluent quality suitable for future water reuse.	7	The activated sludge basins are adaptable and can be modified to accommodate future technologies.	6	This alternative requires a moderate amount of site space. There will be space to construct additional activated sludge basins for future upgrade.	6	Requires relatively low amount of site disturbance.	710	2
Alternative 3: MBR Activated Sludge	1	The MBR process is significantly different than the existing SBR process and will require considerable training.	1	The MBR process has highly complex equipment that requires a lot of operational attention.	1	The MBR process requires significant maintenance of complex equipment.	10	The MBR process received the most positive feedback from public groups. It is one of the newer, advanced technologies that public groups are most excited about.	10	The MBR process will provide effluent quality suitable for future water reuse.	9	The MBR is a newer treatment technology on the forefront of wastewater treatment. It removes the most solids of the three alternatives, providing marginally better effluent quality to be used in a future potable water	10	This alternative minimizes the amount of site space required. There will be space to construct additional MBR tanks for future upgrade.	10	Requires least amount of site disturbance.	800	1



Based on the results of the evaluation, of the three treatment alternatives evaluated, Alternative 3 – MBR Activated Sludge received the highest score based on the criteria listed in this section.

Before recommending Alternative 3 – MBR Activated Sludge, one non-monetary consideration that is specific to MBRs was also considered, and that is the design of MBR systems from different vendors varies widely. The MBR is purchased as an integrated system of equipment and controls, and there is no standardization among MBR vendors. The equipment required, physical layout, operational characteristics and control can be very different for each vendor's system.

To avoid expensive re-design during construction to accommodate the specifics of the provided MBR system, the Town will include a competitive pre-selection of MBR system and vendor during the final design of the project. By developing a request for proposals (RFP) for the MBR system between the 30% and 60% design submittals, the design can be tailored to the pre-selected system. This also shortens the time for development of shop drawings after the notice to proceed for construction, so key long lead items can be purchased in time to avoid the construction schedule critical path.

The design engineer and the Town will also consider pre-purchasing major equipment that could greatly impact the overall construction duration. One item included in each alternative scope that remain with an excessively long lead time is the back-up generator. Fortunately, the back up generator does not impact the installation and start up of any of the treatment facilities. So, if the generator could be installed before the scheduled substantial completion, it would not impact the overall construction duration. It is anticipated that the generator delivery could be 18 months, which would not impact the overall construction duration.

Having addressed these Alternative 3 – MBR Activated Sludge risks, and as a result of the life cycle cost analysis and the non-monetary evaluation, Alternative 3 – MBR Activated Sludge is the recommended treatment upgrade for the Centreville WWTP. This alternative will be further developed during detailed design.



# 8 Recommended Upgrades and Expansion

## 8.1 Preliminary Project Design

A summary of the project scope for the expansion and upgrade of the Centreville WWTP to an MBR activated sludge treatment process is detailed in this section. **Table 8.1** lists the preliminary project design and facility upgrades that are required for the recommended alternative.

Table 8.1: Preliminary Proje	ct Design – Alt 3 MBR Activated Sludge
Facility	Description
Influent Screening	Replace the existing mechanical screen with a bar rack rated at 4.0 MGD (peak hydraulic flow) and install new center feed fine screens down stream of bar rack.
Influent Flow Equalization Tank	Convert the existing SBR process tanks to two (2) 500,000-gallon working capacity each influent flow EQ tanks with surface aerator/mixers. Submersible pumps will pump flow from the EQ tank to the MBRs.
MBR Process	Install 2 train, 5-stage activated sludge process with membranes to separate solids from treated effluent. Fine bubble diffusers will be installed to incorporate air from proposed high efficiency blowers. Anoxic and swing zones will be agitated with submersible mixers. Permeate pumps will draw effluent through membranes. Low head propeller pumps for internal recycle and return activated sludge will be installed. Waste sludge pumps will pull mixed liquor from the reactors and discharge into the aerobic digesters. Chemical cleaning facilities will be provided to clean the membranes.
Chemical Dosing	Provide a double contained PACI tank located in the Filter and Blower Building with a minimum of 30 days of storage and dosing system. Provide methanol storage with a minimum of 30 days of storage and dosing facility.
UV Disinfection	Install two (2) in-line low pressure high output (LPHO) UV disinfection units to replace existing.
Effluent Disposal	<ul> <li>To be further evaluated:</li> <li>Additional spray irrigation disposal,</li> <li>Relocate outfall and expand stream discharge to year-round, and</li> <li>Planning for future beneficial water reuse.</li> </ul>
Non-potable Plant Water System	Install a non-potable water system that draws from the UV effluent to a buffer tank in the Filter and Blower Building. Install pumps to distribute non-potable water supply from the buffer tank throughout the WWTP.
Aerobic Digesters	Retrofit the existing post EQ and sludge holding tanks to two (2) aerobic digesters with ability to thicken solids and decant liquid back to treatment process.
Biosolids Dewatering System	Install new biosolids handling building for dewatering process. New covered sludge cake storage area for Class B biosolids.
Plant Control System and SCADA	Provide enhanced process controls at separate process areas with routine functions or complex control loops with centralized monitoring and control workstation for operator interface. Provide capabilities to provide hub for Town wide SCADA system of utilities.
Administration/Laboratory Space	Reconfigure the Administration/Laboratory Building to better utilize the space for the laboratory uses and provide dedicated space for locker rooms and offices.



## 8.2 Permit Requirements

Impacts to wetlands and other WOTUS would require Section 404 authorization from the U.S. Army Corps of Engineers (USACE) for the discharge of dredge or fill material. Impacts to waterways, 100-year floodplains, nontidal wetlands, 25-foot nontidal wetland buffers would require a Maryland Nontidal Wetlands and Waterways Permit. Additionally, a Section 401 Water Quality Certificate from MDE is required for any impacts to waterways or wetlands requiring a USACE Section 404 authorization. Projects with the potential to impact Tier II waters are subject to MDE's Tier II Antidegradation Review. Early coordination with MDE will be initiated during the permitting process to determine whether additional avoidance measures and best management practices (BMPs) are required. Impacts to forest, trees, and FIDS habitat within the CBCA would require coordination with the Chesapeake Bay Critical Area Commission (CAC) and/or the Queen Anne's County Critical Area Program.

**Table 8.2** summarizes the expected permits that will need to be obtained during the design phase of the project. Additional environmental permits will be identified during design.

Table 8.2: Permit Requirements	
Permitting Agency	Permit
Maryland Department of the Environment (MDE)	Sewerage Construction Permit
Maryland Department of the Environment (MDE)	Modification to NPDES Surface Water Discharge Permit
Maryland Department of the Environment (MDE)	NPDES Permit for Stormwater Discharge During Construction Activities
Queen Anne's County	Sediment and Erosion Control Plan Permit
Queen Anne's County	Stormwater Management Permit

### 8.3 Sustainability Considerations

The WWTP upgrade and expansion to an MBR treatment process will be designed to reduce its impact on the environment and to be resilient to future changes in the climate as indicated in this section.

### 8.3.1 Water and Energy Efficiency

As described in **Sections 6.1.7 and 6.5.1**, a non-potable water system will be installed at the Centreville WWTP to promote water efficiency by reducing the onsite potable water demand and reusing treated plant effluent. The treated effluent water quality will be sufficient to meet off-site Class III and IV reclaimed water requirements. New developments will be encouraged to connect into the reclaimed water for irrigation of common spaces.

Energy efficiency will also be at the forefront for the selection of lighting and equipment for the project. Examples of improved energy efficiency include:

- The existing florescent tube and halogen lights will be replaced with LED lights. New lights will only be LED.
- All equipment will use high efficiency motors.
- The UV disinfection system will have the latest generation of UV intensity measurement and lamp controller.
- Pumps will have variable frequency drives (VFD) to operate at optimal speeds.
- New process blowers will be high efficiency turbo blowers.



• Dewatering equipment will only consider slow speed, low energy demand type equipment.

### 8.3.2 Green Infrastructure

As described in **Section 6.5.2**, the selected treatment Alternative 3 will incorporate green infrastructure at a reasonable cost. The canopy over the dewatering cake storage area will be designed to accommodate the future installation of solar cells. The solar cells will be connected to the utility electric grid to offset the electricity used by the WWTP.

### 8.3.3 Climate Related Considerations

As described in **Section 6.5.3**, the upgrade and expansion of the WWTP is required to protect the receiving stream and the environment from wastewater that does not meet the discharge permit requirements. Without an expansion of the treatment capacity, the likelihood of future process upsets increases with the increase in influent flows stressing the capabilities of the existing system.

The new facilities will be constructed to protect them from a 100-year flood with 3 feet of additional protection provided. New structures will have a finished floor or top of wall of at least 3 ft above the 100-year flood elevation.

With the recommended treatment Alternative 3 – MBR Activated Sludge, the proposed facilities will have the smallest footprint of the alternatives considered and can be located to reduce the impact on environmental features such as wetlands and forested areas.

Additionally, expansion of the sludge treatment and handling facilities will result in Class B biosolids. This could potentially allow for land application of the dewatered biosolids, which is a more solution to minimizing landfill disposal.

### 8.4 Construction Cost Estimate

A budgetary cost estimate of construction for the recommended treatment alternative (Alternative 3 – MBR Activated Sludge) is included below. The cost estimate was developed using preliminary equipment supplier quotations based on the design criteria and unit costs for structures and ancillary construction. All three treatment alternatives were analyzed for construction and life cycle cost (see **Sections 6.6 and 7.2**), but this section will focus on the construction cost estimate of the recommended Alternative 3.

The estimates were prepared in accordance with AACE Class 4 Budgetary (planning-level) construction cost requirements. All costs are presented in 2023 dollars and will need to be indexed using the annual inflation rate. Contingency cost, an allowance that reflects the uncertainty associated with a construction cost opinion based on a planning level stage of the facilities, is included as a 30% markup in the estimate. Additionally, an escalation markup of 4% per year is also included in the estimate.

The estimated total construction cost for Alternative 3 – MBR Activated Sludge is summarized in **Table 8.3**. Additional cost breakdowns for Alternative 3 – MBR Activated Sludge are included in **Appendix A**.



Table 8.3: \$	Scope and Construction Cost Estimate – Alternative 3 (MBR Activated Slu	ıdge)
Item No.	Category	Cost
1	Interior Demolition (Lab, Control, and Filter and Blower Buildings)	\$95,000
2	Influent Screening Expansion	\$825,000
3	Converting Influent Flow Equalization Tanks, Aerated, with Pumping	\$2,019,000
4	Methanol Facility	\$618,000
5	UV Disinfection System	\$642,000
6	Non-Potable Water System	\$54,000
7	Dewatering Facility	\$2,413,000
8	Covered Cake Storage Facility	\$835,000
9	Lab, Control, and Filter and Blower Buildings Refurbishments	\$617,000
10	Existing Tank Modifications	\$643,000
11	Miscellaneous Process Piping and Equipment	\$784,000
12	MBR Process Building, MBR Equipment and Controls	\$5,789,000
13	Aerobic Digester Tank and Equipment	\$78,000
14	Electrical	\$4,169,000
15	Site Civil, including Yard Piping and Demolition (15% Items 1-12)	\$2,312,000
16	Site SCADA (5% Items 1-12)	\$771,000
	Subtotal	\$22,664,000
	Design Contingency (30% of Subtotal)	\$6,799,000
	Escalation to December 2026 (4%/year)	\$3,678,000
	Total	\$33,141,000
	Total (Low Range -20%)	\$26,513,000
	Total (High Range +50%)	\$49,712,000

## 8.5 Annual Operating Budget

### 8.5.1 Income

The Town projects income for the sewer system primarily from ongoing sewer service fees with some new connection fees expected. **Table 8.4** summarizes the currently projected annual income for the sewer system for the next five fiscal years.



Table 8.4: Sewer System In	come
Fiscal Year	Projected Annual Income
FY24	\$1,531,427
FY25	\$1,607,998
FY26	\$1,704,478
FY27	\$1,826,791
FY28	\$1,972,694

### 8.5.2 Annual O&M Costs

The primary operating and maintenance costs after Alternative 3 is implemented are summarized in **Table 7.5**, with Alternative 3 repeated in **Table 8.5** for ease of reference.

Table 8.5: Annual Sewer System O&M C	osts
Alternative 3 – MBR Activated Sludge	Annual Costs
Maintenance/Repair Costs <sup>(1)</sup>	\$226,647
Electric Cost	\$ 84,877
Burdened Labor	\$499,200
Chemical Costs	\$336,886
Total Operating and Maintenance Costs	\$1,147,610

<sup>(1)</sup> - Maintenance is estimated at 2.5% of equipment cost for Alternative 3 to account for membrane replacement and cleaning chemicals.

### 8.5.3 Debt Repayments

The Town's existing debt is being serviced from the annual budget. Additional debt will primarily be serviced through anticipated connection fees and additional sewer service fees from the planned and anticipated developments within the current town boundary and by the annexation of adjacent development.

### 8.5.4 Reserves

The Town maintains a healthy reserve fund. As the Town grows, the reserve fund will also be increased to keep pace with the increased operating and maintenance costs of the expanded treatment plant.

### 8.6 Project Implementation Schedule

A preliminary construction schedule has been developed for the scope of work. The task durations for each of the facility upgrades detailed in this report are included in **Figure 8.1**.

ID	Task Name	Duration	Start	Finish	Q4	2023 01	02	Q3 Q4	2024	Q2 0	)3 □ ∩4	2025	02	Q3 Q4	2026	Q2 Q3 Q	2027 4 Q1 Q2
1	Centreville WWTP ENR Upgrade and Expansion	1655 days	Mon 3/20/23	Mon 7/23/29			<u> </u>										
2	PER Development	255 days	Mon 3/20/2	3Fri 3/8/24	_	Ē			I								
3	Start PER Update	0 days	Mon 3/20/23	3 Mon 3/20/23	-	•	3/20										
4	Advertise for M/WBE	4 wks	Mon 3/27/23	3 Fri 4/21/23	_												
5	Town Assembles Requested Informatio	4 wks	Mon 3/27/23	3 Fri 4/21/23													
6	Develop ENR PER Amendment	2 wks	Mon 4/24/23	3 Fri 5/5/23													
7	Town Review PER Amendment	2 wks	Mon 5/8/23	Fri 5/19/23			<b>I</b>										
8	Sub Agreements	2 wks	Mon 5/22/23	3 Fri 6/2/23			l ii										
9	Develop Draft PER	4 wks	Mon 6/5/23	Fri 6/30/23													
10	Topo Survey	4 wks	Mon 6/5/23	Fri 6/30/23													
11	Develop Alternatives	16 wks	Mon 3/20/23	3 Fri 7/7/23		Ì		h									
12	Subconsultant Field W	orl 8 wks	Mon 5/22/23	3 Fri 7/14/23													
13	Develop PER	22 wks	Mon 7/17/23	3 Fri 12/15/23				*	5								
14	Draft PER to Town and MDE	0 days	Mon 12/18/23	Mon 12/18/23					12/1	8							
15	Review Draft PER	8 wks	Mon 12/18/2	2:Fri 2/9/24													
16	Incorporate Comment	s 2 wks	Mon 2/12/24	4 Fri 2/23/24					i i i								
17	Finalize PER	2 wks	Mon 2/26/24	4 Fri 3/8/24													
18	Develop Design Propo	sal 4 wks	Mon 12/18/2	2:Fri 1/12/24					<b>1</b>								
19	Town Reviews Design Proposal	2 wks	Mon 1/15/24	4 Fri 1/26/24					Ĩ								
20	ENR Upgrade Design	390 days	Mon 1/29/2	4 Fri 7/25/25					r				[	l			
21	Project Set Up	2 wks	Mon 1/29/24	4 Fri 2/9/24	_				i,								
22	Design Kick Off w Tow	n 0 days	Mon 2/12/24	4 Mon 2/12/24					<b>X</b>	2/12							
23	30% Design	8 wks	Mon 2/12/24	4 Fri 4/5/24	_					∎ŋ							
24	30% Design Internal Q	A 2 wks	Mon 4/8/24	Fri 4/19/24						й,							
25	30% Design to Town	0 days	Mon 4/22/24	4 Mon 4/22/24						<b>4/22</b>							
26	Town Reviews 30% De	sig2 wks	Mon 4/22/24	4 Fri 5/3/24						it.							
27	30% Design Comment Review Meeting	0 days	Mon 5/6/24	Mon 5/6/24						₹ 5/6							
28	60% Design	10 wks	Mon 5/6/24	Fri 7/12/24	_					<b>L</b>							
29	60% Design Internal Q	A 2 wks	Mon 7/15/24	4 Fri 7/26/24						, it							
30	60% Design to Town	0 days	Mon 7/29/24	4 Mon 7/29/24						•	7/29						
31	Town Reviews 60% De	5	Mon 7/29/24	4 Fri 8/9/24						ì							
	· · · · · · · · · · · · · · · · · · ·	<b>Y</b>									1						
	-	ask		Project	Summa	ry			Man	ual Task	1			Start-or	nly	E	Deadl
Projec	ct: Centreville WWTP ENR	Split		Inactive	Task				Dura	ition-only	J			Finish-c	only	Э	Progre
Date:	Mon 12/4/23	Vilestone	•	Inactive	Milesto	one	$\diamond$		Man	ual Summar	y Rollup 🗖			External	Tasks		Manu

Inactive Summary

Summary

Figure 8.1: Project Implementation Schedule

Manual Summary

 $\diamond$ 

External Milestone

Q3	Q4	2028 Q1	Q2	Q3	Q4	2029 Q1	Q2	Q3	Q4	
								-		
		+								
ine ess		*			_					
al Progre	SS	_			-					

D	Task Name	Duration	Start	Finish		2023 Q1	Q2 Q3 Q4	2024	Q2		2025	Q2 Q3		2026	02 Q3		2027 Q1 Q2
32	60% Design Comment Review Meeting	0 days	Mon 8/12/24	Mon 8/12/24	Q4	<u>Q1</u>	<u>  Q2   Q3   Q4</u>		<u> </u>	Q3 Q4 8/12	<u>Q1</u>	<u>Q2</u> <u>Q3</u>	<u> </u>	<u><u> </u></u>	<u> 22   Q3</u>	<u> </u>	01   02
33	Pre-Final Design	10 wks	Mon 8/12/24	Fri 10/18/24													
34	Pre-Final Design Internal QA	2 wks	Mon 10/21/24	Fri 11/1/24						<b>N</b>							
35	Pre-Final Design to Towr	n0 days	Mon 11/4/24	Mon 11/4/24						1	11/4						
36	Town Reviews Pre-Final Design	2 wks	Mon 11/4/24	Fri 11/15/24						1 I							
37	Pre-Final Design Comment Review	0 days		Mon 11/18/24						*	11/18						
38	Design Review Meeting with MDE	2 wks	Mon 11/18/24	Fri 11/29/24						it.							
39	MDE Review	8 wks	Mon 12/2/24	Fri 1/24/25													
40	Permitting	12 wks	Mon 1/27/25	Fri 4/18/25							<b>*</b>	B <sub>1</sub>					
41	Develop Bid Ready Documents	4 wks	Mon 4/21/25	Fri 5/16/25													
42	Bid Ready Doc Internal C	2 wks	Mon 5/19/25	Fri 5/30/25								1 A					
43	Bid Ready Docs to Town	0 days	Mon 6/2/25	Mon 6/2/25								<b>6</b> /2					
44	Town Finalizes Funding	8 wks	Mon 6/2/25	Fri 7/25/25													
45	ENR Upgrade and Expansion Bidding	130 days	Mon 7/28/25	Fri 1/23/26								r		1			
46	Town Prepares for Advertisement	4 wks	Mon 7/28/25	Fri 8/22/25								<b>i</b>					
47	Advertise	12 wks	Mon 8/25/25	Fri 11/14/25								Ì	Ц				
48	Open Bids	0 days	Mon 11/17/2	Mon 11/17/2									1	1/17			
49	Bid Review	2 wks	Mon 11/17/2	Fri 11/28/25									ц,				
50	Bid Recommendation to MDE	0 days	Mon 12/1/25	Mon 12/1/25									•	12/1			
51	MDE Bid Review	4 wks	Mon 12/1/25	Fri 12/26/25										7			
52	Construction NTP	4 wks	Mon 12/29/2	Fri 1/23/26									ì				
53	ENR Upgrade and Exp Construction	910 days		Mon 7/23/29										-			
54	Issue PO's	2 wks	Mon 1/26/26	Fri 2/6/26										й,			
55	Shop Drawing Submittals and Review	16 wks	Mon 2/9/26	Fri 5/29/26										<b>*</b>			
56	Major Equipment Delive	r 36 wks	Mon 6/1/26	Fri 2/5/27											+		<b>H</b>
57	Mobilize Site	0 days	Mon 2/8/27	Mon 2/8/27													2/8
58	Construction	104 wks	Mon 2/8/27	Fri 2/2/29													<b>*</b>
59	Substantial Completion	0 days	Mon 2/5/29	Mon 2/5/29													

	Task		Project Summary	1	Manual Task		Start-only	C	Deadli
Project: Centreville WWTP ENR	Split		Inactive Task		Duration-only		Finish-only	D .	Progre
Date: Mon 12/4/23	Milestone	•	Inactive Milestone	$\diamond$	Manual Summary Rollup		External Tasks		Manua
	Summary	I1	Inactive Summary	0	Manual Summary	1	External Milestone	$\diamond$	

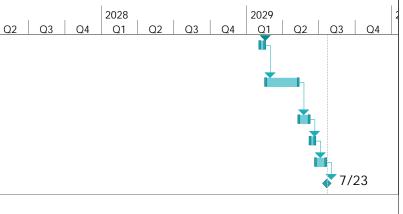
Figure 8.1: Project Implementation Schedule

			2028				2029	1	Q3 Q4		
Q2	Q3	Q4	Q1	Q2	Q3	Q4	Q1	Q2	Q3	Q4	
									1		
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								-			
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Deadlii	ne		₽								
Progre	SS					•					
Manua	I Progre	SS				•					

ID	Task Name	Duration	Start	Finish		2023				2024				2025				2026				2027	
					Q4	Q1	Q2	Q3	Q4	Q1	Q2												
60	Operations Process Training	2 wks	Mon 2/5/29	Fri 2/16/29																			
61	Commissioning and Start Up	12 wks	Mon 2/19/29	Fri 5/11/29																			
62	Process Testing	4 wks	Mon 5/14/29	Fri 6/8/29																			
63	Develop Punchlist	2 wks	Mon 6/11/29	Fri 6/22/29																			
64	Project Closeout	4 wks	Mon 6/25/29	Fri 7/20/29																			
65	Final Completion	0 days	Mon 7/23/29	Mon 7/23/29																			

	Task		Project Summary		Manual Task		Start-only	C	Deadline	+
Project: Centreville WWTP ENR	Split		Inactive Task		Duration-only		Finish-only	3	Progress	
Date: Mon 12/4/23	Milestone	<b>♦</b>	Inactive Milestone	$\diamond$	Manual Summary Rollup		External Tasks		Manual Progress	
	Summary	I1	Inactive Summary	[	Manual Summary	1	External Milestone	$\diamond$		

Figure 8.1: Project Implementation Schedule





# 9 Project Asset Management

### 9.1 Inventory of Critical Assets

After the implementation of the Centreville WWTP ENR Upgrade and Expansion project the following will be the Town's Critical Assets at the WWTP:

- 1. Incoming Power Distribution
- 2. Back Up Power Generator
- 3. Aeration Blowers Existing
- 4. Aeration Blowers New
- 5. Administration/Lab Building
- 6. Disinfection (Filter and Blower) Building
- 7. Control Building
- 8. Influent Screening Facility
  - a. Mechanically Cleaned Screens
  - b. Washer/Compactor
  - c. Concrete Channels
- 9. Process Tanks
  - a. Flow EQ Tanks
  - b. Aerobic Digesters
  - c. Diffusers
- 10. MBR Trains
  - a. Mixers
  - b. Diffusers
  - c. Internal Recycle Pumps
- 11. Ultraviolet Light Disinfection
- 12. Effluent Pump Station
- 13. Dewatering Facility
  - a. Dewatering Press
  - b. Polymer Storage and Dosing
  - c. Sludge Conveyor
- 14. Covered Cake Storage Area

### 9.2 Condition of Critical Assets

All critical assets will be new with the exception of the following:

- A. Incoming Power Distribution
- B. Aeration Blowers Existing
- C. Administration/Lab Building
- D. Disinfection (Filter and Blower) Building
- E. Process Tanks
- F. Effluent Pump Station

The condition of the critical assets is described below:

A. Incoming power distribution

The incoming power distribution includes the utility owned transformer and cables to the overhead power system. The incoming switchboard owned by the Town is in in good condition with many years of remaining expected life.



B. Aeration Blowers – Existing

The three existing aeration blowers are 20 years old and are operating as designed and are in good condition. The blowers have many years of remaining expected life.

The existing aeration blowers will be used for processes that are ancillary to the treatment process, specifically to provide aeration of the two influent flow equalization tanks and the two aerated digesters. The blowers will have a standby unit.

C. Administration/Lab Building

The Administration/Lab Building will be refurbished with the project and will have many years of remaining expected life.

D. Disinfection (Filter and Blower) Building

The Disinfection (Filter and Blower) Building is in good condition with many years of remaining expected life.

E. Process Tanks

The concrete process tanks are in good condition with more than 30 years of remaining expected life. The mechanical equipment will be replaced with the project. Handrails, lighting, and other appurtenances will be refurbished or replaced during the upgrade.

### 9.3 Critical Asset Maintenance and Replacement Plan

With the installation of the majority of the equipment and tanks being newly installed with the upgrade and expansion project, there is the typically a cut to maintenance budgets. In conjunction with the lower maintenance budget, the Town must institute a replacement fund which is funded annually with the monies the Town would have spent maintaining 20+ year old equipment. Therefore, with funding similar to current, the Town will be prepared for the eventual replacement of equipment as needed with the saved funds.

## 9.4 Critical Asset Energy and Water Efficiency Plan

There are two parts of critical asset energy and water efficiency: operational efficiency and future upgrades.

Operational efficiency refers to how the treatment process is actually operated compared with the optimal theoretical energy and water efficiency. For example, aeration is the single largest cost for the activated sludge treatment process and automating the speed of the blowers to provide just enough air to meet the process requirements, will save considerable energy compared to manually operating the blowers.

The ENR upgrade will include simple and proven process instrumentation and automation to assist the operations to operate the treatment process with operational efficiency. Examples include in tank continuous read dissolved oxygen and ammonia instruments to monitor the treatment process and adjust aeration needs automatically using Ammonia Based Aeration Control (ABAC). Chemical dosing will also have flow pacing implemented to automatically adjust the phosphorus precipitant to adjust dosing based on continuous flow measurement inputs. The methanol dosing will be controlled based on nitrate readings entering and leaving the second anoxic zones.

Water efficiency will be primarily through the replacement of potable water use with non-potable water everywhere practical.



Future upgrades consider the improvements in energy efficiency over time. For example, at some point in the future, it is likely the ultraviolet (UV) light disinfection system installed with the ENR upgrade and expansion which is highly efficient by today's standards, will be eclipsed by future technologies of UV disinfection, or another completely different technology. The Town's DPW needs to keep up to date with the latest equipment available by attending wastewater conferences, or by bringing a consulting engineer into an on call contract. The on call engineer can be tasked with reviewing the energy efficiency of the treatment processes and make recommendations for improvements.



Appendix A

# Cost Estimate Line Items

#### Centreville ENR Upgrade and 1 MGD Expansion Town of Centreville Preliminary Construction Cost Estimate

	Alt 1	- 4 SBR	Alt 2	- Act Sludge	Alt 3 - MBR		
Alternative 3 Facilities							
Existing Tank Modifications	\$	214,000	\$	643,000	\$	643,000	
Clarifier Tanks, Equipment and RAS PS	\$	-	\$	4,749,000	\$	-	
Denitrification Filter Tanks, Equip and Controls	\$	3,112,000	\$	3,112,000	\$	-	
Misc Process Piping and Equipment	\$	157,000	\$	235,000	\$	784,000	
Additional SBR Tanks, Equip, and Controls	\$	3,564,000	\$	-	\$	-	
Activated Sludge Equipment	\$	-	\$	1,012,000	\$	-	
MBR Process Equipment and Controls	\$	-	\$	-	\$	5,789,000	
Post EQ Tank and Equipment	\$	78,000	\$	-	\$	-	
Aerobic Digester Tank and Equipment	\$	1,427,000	\$	78,000	\$	78,000	
Ultraviolet Disinfection System	\$	642,000	\$	642,000	\$	642,000	
Pre-Flow EQ Tank, Aerated, w Pumping	\$	2,054,000	\$	2,019,000	\$	2,019,000	
Alternative Subtotal Cost	\$	11,248,000	\$	12,490,000	\$	9,955,000	
Base Facilities							
Interior Demolition (Lab, Control, and Filter and Blower Buildings)	\$	95,000	\$	95,000	\$	95,000	
Influent Screening Expansion	\$	825,000	\$	825,000	\$	825,000	
Methanol Facility	\$	618,000	\$	618,000	\$	618,000	
Non-Potable Water System	\$	54,000	\$	54,000	\$	54,000	
Dewatering Facility	\$	2,413,000	\$	2,413,000	\$	2,413,000	
Covered Cake Storage	\$	835,000	\$	835,000	\$	835,000	
Lab Building Refurb	\$	139,000	\$	139,000	\$	139,000	
Control Building Refurb	\$	130,000	\$	130,000	\$	130,000	
Filter & Blower Building Refurb	\$	348,000	\$	348,000	\$	348,000	
Base Subtotal Cost	\$	5,457,000	\$	5,457,000	\$	5,457,000	
Alternative Plus Base - Subtotal Construction Cost	\$	16,705,000	\$	17,947,000	\$	15,412,000	
Electrical	\$	2,517,000	\$	2,722,000	\$	4,169,000	
Site Civil, inc Yard Piping and Demo (15%)	\$	2,505,750	\$	2,692,050	\$	2,311,800	
Site SCADA (5%)	\$	835,250	\$	897,350	\$	770,600	
Subtotal	\$	22,563,000	\$	24,258,400	\$	22,663,400	
Contingency (30%)	\$	6,769,000	\$	7,278,000	\$	6,799,000	
WWTP ENR Total Const Cost (December 2023 Dollars)	\$	29,332,000	\$	31,536,400	\$	29,462,400	
Escalated to December 2026 (4%/year)	\$	32,994,000	\$	35,474,000	\$	33,141,000	
	\$	3,662,000	\$	3,937,600	\$	3,678,600	

MARK-UP SU	JMMARY						-	
PROJECT NAME:	Centreville WWTP EN	NR Upgrade and Expansion PER Centreville, Maryland		CLIENT: Town of Centreville		ESTIMATED BY: WRA	W	
PROJECT LOCATION:				DESIGN SUBMISSION:	PER	WORK ORDER NUMBER: 14375-000		
MARKUP		DESCRIPTION		1		MATERIAL	LABOR	EQUIPMENT
SUBCONTRAC	TOR MARKUPS					MARKUP %	MARKUP %	MARKUP %
		Factors from Means Locatio	n Factor Tables	Cit	y Centreville			
		and City Cost Index Tables t		Materia	al 100.00%			
LOC	CATION FACTORS	market conditions at project	location	Labo	r 100.00%	0.00%	0.00%	0.00%
				Equipmer	t 100.00%			
WORK REST	RICTION FACTOR/PHASING	This factor adjusts for project requirements, limited site ac			on work hours, security		0.00%	
SALES TAX O	N MATERIAL & EQUIPMENT	Sales tax may be added to r MD	naterials costs, e	quipment costs, and su	6.00%		6.00%	
L	ABOR BURDEN						29.17%	
Worke	ers Comp. Insurance	State specific; Means 2022				9.00%		
F	Fixed Overhead	Federal and State Unemploy	yment, FICA, Risl	k Insurance & Liability; I		18.50%		
INSTALLING CONTRACTOR OVERHEAD		Home office overhead for In	stalling Contracto	or. This markup is typica	10.00%	10.00%	10.00%	
INSTALLIN	G CONTRACTOR PROFIT	Profit for Installing Contracto	r. This markup is	typically in the range of	10.00%	10.00%	10.00%	
TOTAL MARKU	IP - SUBCONTRACTOR					1.283	1.563	1.283
PRIME CONTRA	ACTOR MARKUPS							
GENERA	L REQUIREMENTS & CQC	Job office overhead costs in etc. Line items (Div 01) or pe 3% CQC.			5.00%	5.00%	5.00%	
PR		Home office overhead for Pr small projects self-performer			8.25%	8.25%	8.25%	
I	PRIME PROFIT	Profit for Prime Contractor. performed by the Prime, this		pically in the range of 5	6.00%	6.00%	6.00%	
	BOND	The bond is used to pay for ranges from 0.5% - 2%, dep			1.50%	1.50%	1.50%	
Cost growth (escalation) from the ESCALATION Source of escalation index = (Me *Note: Escalation is calculated in				IAVFAC, etc.)	ed mid-point of construction.	0.00%	0.00%	0.00%
DESI	GN CONTINGENCY	Required to account for cost	of unknowns bas	sed on level of design d	0.00%	0.00%	0.00%	
MISC. PRO	JECT-SPECIFIC MARKUP	(Enter description here. This	will not be used	for most projects.)				
TOTAL MARKU	IP - PRIME CONTRACTOR	R				1.223	1.223	1.223
	IP - COMBINED					1.568	1.911	1.568

	COST: GENERAL REQUIREMENT	•								
PROJECT NAME:	Centreville WWTP ENR Upgrade and	d Expansi	on PER	CLIENT: Town of (	Centreville	ESTIMATED BY: W	RA		VR	
PROJECT LOCATION:	Centreville,			DESIGN SUBMISSIO	N:	WORK ORDER NUM	BER:	V		
	Maryland			PI	ER	1437	5-000	-		
SOURCE	ITEM DESCRIPTION	QUANTITY	UNIT OF		UNIT COSTS			TOTAL COSTS		TOTAL
SOURCE		QUANTITY	MEASURE	MATERIAL	LABOR	EQUIPMENT	MATERIAL	LABOR	EQUIPMENT	TOTAL
	DEMOLITION									
	3 existing buildings: miscellaneous interior demolition of interior partitions, casework, etc.	1	LS	\$-	\$ 50,000.00	\$-	\$-	\$ 50,000	\$-	\$ 50,000
	NEW WORK									
	Dewatering Building	1960	Sq Ft	\$ 150.00	\$ 150.00	\$ 10.00	\$ 294,000	\$ 294,000	\$ 19,600	\$ 607,600
Alfa Laval Quote	2m Belt Filter Press	1	LS	\$ 385,500.00	\$ 115,650.00		\$ 385,500	\$ 115,650	\$ -	\$ 501,150
	Sludge Conveyors	1	LS	\$ 100,000.00	\$ 100,000.00	\$-	\$ 100,000	\$ 100,000	\$ -	\$ 200,000
	Polymer System	1	LS	\$ 100,000.00	\$ 15,000.00	\$-	\$ 100,000	\$ 15,000	\$-	\$ 115,000
	Non-Potable Water System	1	LS	\$ 25,000.00	\$ 7,500.00	\$-	\$ 25,000	\$ 7,500	\$ -	\$ 32,500
	Influent Screening Concrete	100	CY	\$ 400.00	\$ 400.00	\$ -	\$ 40,000	\$ 40,000	\$ -	\$ 80,000
Huber Quote	Influent Screens	2	ea	\$ 160,000.00	\$ 48,000.00	\$ -	\$ 320,000	\$ 96,000	\$-	\$ 416,000
	Methanol Facility	1	LS	\$ 150,000.00	\$ 200,000.00	\$-	\$ 150,000	\$ 200,000	\$-	\$ 350,000
	Covered Cake Storage	3200	Sq Ft	\$ 75.00	\$ 75.00	\$ -	\$ 240,000	\$ 240,000	\$-	\$ 480,000
	Lab Building Refurbishment	800	Sq Ft	\$ 50.00	\$ 50.00	\$ -	\$ 40,000	\$ 40,000	\$ -	\$ 80,000
	Control Building Refurbishment	750	Sq Ft	\$ 50.00			\$ 37,500			\$ 75,000
	Filter & Blower Building Refurbishment	2000	Sq Ft	\$ 50.00	\$ 50.00	\$ -	\$ 100,000	\$ 100,000	\$ -	\$ 200,000
	SUBTOTAL DIRECT COSTS						\$ 1,832,000	\$ 1,335,650	\$ 19,600	\$ 3,187,250
	SUBCONTRACTOR MARKUP						\$ 517,723			
	SUBTOTAL			1			\$ 2,349,723			
	PRIME CONTRACTOR MARKUP						\$ 523,741	, ,		
	<b>BASE BID DIVISION 1 - TOTAL COSTS</b>						\$ 2,873,464	\$ 2,552,772	\$ 30,742	\$ 5,456,97

DETAILED C	OST: ALTERNATIVE 1 - SBRs														14		
PROJECT NAME:	Centreville WWTP ENR Upgrade and	Expansio	n PER	CLIE	NT: Town of Ce	entr	eville	ESTI	MATED BY:	RA					R		
PROJECT LOCATION:	Centreville,			DES	GN SUBMISSION:			WOR		BER:			- V	V			<b>/</b> /
	Maryland				PEF	२			1437		00		-	_			
001/205	ITEM DESCRIPTION		UNIT OF	T		U	NIT COSTS					т	OTAL COSTS				TOTAL
SOURCE		QUANTITY	MEASURE		MATERIAL		LABOR	E	EQUIPMENT		MATERIAL		LABOR	E	QUIPMENT		TOTAL
	DEMOLITION																
	DEMOLITION			\$		\$		\$		\$		\$	-	\$	-	\$	
				\$		\$		\$	-	φ \$	-	\$	-	φ \$		\$	
				\$		\$		\$	-	\$	-	\$	-	\$		\$	
	NEW WORK			Ŷ		Ŷ		Ŧ		Ŧ		Ŷ		Ŷ		Ŷ	
Evoqua Budget Quote	Closed Vessel Low Pressure UV System	1	ls	\$	357,448,00	\$	107,234.40	\$	-	\$	357,448	\$	107,234	\$	-	\$	464,682
					,	,	. ,			\$	-	\$	-	\$	-	\$	-
Leopold Budget Quote	Denitrification Filters Equipment	1	ls	\$	1,500,000.00				-	\$	1,500,000	\$	450,000		-	\$	1,950,000
	Denitrifiction Concrete	300	CY	\$	400.00	\$	400.00	\$	-	\$	120,000		120,000		-	\$	240,000
				-						\$	-	\$	-	\$	-	\$	-
AquaSBR Budget Quote	AquaSBR System	1 1500	ls CY	\$			246,492.00		-	\$	821,640		246,492		-	\$ \$	1,068,132
	2 x SBR Tank Concrete	1500	CY	\$ \$	400.00	\$ \$		\$ \$	-	\$ \$	600,000	\$ \$	600,000	\$ \$	-	\$ \$	1,200,000
	Existing Tank Modifications	1	LS	\$	100,000.00	_		\$	-	φ \$	100,000		30,000	\$		\$	130,000
		•	20	\$	-	\$		\$	-	\$	-	\$	-	\$	-	\$	-
	Misc Process Piping	1	LS	\$	100,000.00			\$	-	\$	100,000		-	\$	-	\$	100,000
				\$	-	\$		\$	-	\$	-	\$	-	\$	-	\$	-
	Post EQ Modifications	1	LS	\$	50,000.00	\$	-	\$	-	\$	50,000	\$	-	\$	-	\$	50,000
				\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
	Pre-Eq Tank Concrete	750	CY	\$	400.00	\$	400.00	\$	-	\$	300,000	\$	300,000	\$	-	\$	600,000
	Pre-Eq Pumps, Blowers and Diffusers	1	LS	\$	400,000.00	\$	200,000.00	\$	-	\$	400,000	\$	200,000	\$	-	\$	600,000
				\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
	Aerobic Digester Concrete	900	CY	\$	400.00			\$	-	\$	360,000		360,000		-	\$	720,000
	Aerobic Digester Blowers and Equipment	1	LS	\$	50,000.00		,		-	\$	50,000		50,000	\$	-	\$	100,000
				\$	-	\$		\$	-	\$	-	\$	-	\$	-	\$	-
				\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
	SUBTOTAL DIRECT COSTS									\$	4,759,088		2,463,726		-	\$	7,222,814
	SUBCONTRACTOR MARKUP									\$	587,808		1,386,823		-	\$	1,974,631
	SUBTOTAL									\$	5,346,896		3,850,549		-	\$	9,197,445
	PRIME CONTRACTOR MARKUP									\$	1,191,796	_	858,268	_	-	\$	2,050,063
	BASE BID DIVISION 1 - TOTAL COSTS									\$	6,538,692	\$	4,708,817	\$	-	\$	11,247,508

PROJECT NAME:	Centreville WWTP ENR Upgrade a	and Expa	nsion	CLIENT: Town of (	Centreville	ESTIMATED BY:	A, DEI			VF		
PROJECT LOCATION:	Centreville,			DESIGN SUBMISSIO		WORK ORDER NUM			V			
ROJECT LOCATION.	Maryland			-	ER		5-000		-			
SOURCE	ITEM DESCRIPTION	QUANTITY	UNIT OF		UNIT COSTS				TOTAL COSTS			TOTAL
		Q0/11111	MEASURE	MATERIAL	LABOR	EQUIPMENT	MAT	TERIAL	LABOR	EQUIPME	T	
	DEMOLITION											
	Demolition	1	LS	\$-	\$ 100,000.00	\$ -	\$	_	\$ 100,000	¢	- \$	100,0
		1	LJ	φ -	\$ 100,000.00	φ -	φ	-	\$ 100,000	φ	- φ	100,0
	NEW WORK											
	Filter Building	4		¢ 000 000 00	¢ 00.000.00	¢	¢	200.000	¢ 00.000	¢		000.0
	600A Motor Control Center (MCC-A) Branch Circuit Wiring from MCC-A	<u>1</u> 1	EA LS	\$ 200,000.00	\$ 20,000.00 \$ 150,000.00			200,000 100,000			- \$ - \$	
	New Feeder for 600 MCC-A	1	LS	\$ 100,000.00			ծ \$	15,000	. ,			,
	Existing Panel DP modifications including new	<u> </u>	Lo	\$ 15,000.00	\$ 3,500.00	ъ -	¢	15,000	\$ 3,500	Þ	- \$	18,5
	breakers and branch circuits	1	LS	\$ 30,000.00	\$ 15,000.00	\$-	\$	30,000	\$ 15,000	\$	- \$	45,0
	Lighting and Branch Wiring	2000	SF	\$ 7.00	\$ 5.00	\$-	\$	14,000	\$ 10,000	\$	- \$	24,0
	Lab Duilding											
	Lab Building 208V Panelboard	2	EA	\$ 10,000.00	\$ 2,000.00	¢	\$	20,000	\$ 4,000	¢	- \$	24,0
	Lighting and Branch Wiring	∠ 750	SF	\$ 10,000.00	. ,		ծ \$	4,500	. ,		- 5 - \$	,
	Receptacles including branch wiring	750	SF	\$ 0.00			ծ \$	4,500	. ,		- 5 - \$	,
	Receptacies including branch withing	750	J	φ 2.00	φ 3.00	φ -	φ	1,500	φ 2,230	φ	- a	5,73
	Dewatering Building											
	480V Panelboard	1	EA	\$ 15,000.00	\$ 3,000.00	\$-	\$	15,000	\$ 3.000	\$	- \$	18.0
	208V Panelboard	2	EA	\$ 10,000.00	. ,		\$	20,000			- \$	- / -
	Lighting and Branch Wiring	1900	SF	\$ 7.00	. ,		\$	13,300	. ,		- \$	,
	Receptacles including branch wiring	1900	SF	\$ 3.00	\$ 4.00	\$ -	\$	5,700	\$ 7,600	\$	- \$	13,3
	Branch circuits for mechanical loads	1900	SF	\$ 6.00	\$ 8.00	\$ -	\$	11,400	\$ 15,200	\$	- \$	26,6
	Dry type transformer 45kVA	2	EA	\$ 2,500.00	\$ 1,250.00	\$-	\$	5,000	\$ 2,500	\$	- \$	7,5
	Outside											
	600A Motor Control Center (MCC-B) including											
	VFDs	1	EA	\$ 150,000.00	\$ 40,000.00	\$-	\$	150,000	\$ 40,000	\$	- \$	5 190,0
	Branch Circuit Wiring from MCC-B including	1	LS	\$ 100,000.00	\$ 125,000.00	\$ -	\$	100,000	\$ 125,000	\$	- \$	225,0
	underground ducts				. ,			,	· ,	· ·		· · · · ·
	Feeder for Dewatering Building	1	LS	\$ 10,000.00	. ,		\$	10,000	. ,		- \$	,.
	Feeder for Control Building	1	LS LS	\$ 5,000.00 \$ 40,000.00			\$ \$	5,000 40,000			- \$	
	Site Lighting and Branch Wiring	I	Lõ	\$ 40,000.00	\$ 40,000.00	5 -	¢	40,000	\$ 40,000	\$	- ⊅	00,0
	Testing and Commisioning	1	LS	\$-	\$ 40,000.00	\$-	\$	-	\$ 40,000	\$	- \$	40,0
	Grounding and Bonding	1	LS	\$ 25,000.00	\$ 50,000.00	\$-	\$	25,000	\$ 50,000	\$	- \$	5 75,0
							¢	705 100	<b>A</b> 070 550			4 457 4
	SUBTOTAL DIRECT COSTS						· ·	785,400	,		- \$	, ,
	SUBCONTRACTOR MARKUP							221,954		· ·	- \$	,
	SUBTOTAL						\$ 1,	007,354	\$ 1,051,167	\$	- \$	5 2,058,5
	PRIME CONTRACTOR MARKUP						\$	224,534	\$ 234,300	\$	- \$	5 458,8
F	BASE BID DIVISION 16 - TOTAL COSTS						\$ 1.	231,888	\$ 1,285,466	\$	- \$	2,517,3

PROJECT NAME:	Centreville WWTP ENR Upgrade an	d Expansi	on PER	CLIE	NT: Town of C	entr		ESTI	IMATED BY:	RA					R	А	
PROJECT LOCATION:	Centreville,			DES	IGN SUBMISSION:			WOR	RK ORDER NUM	BER:			V	V			<b>/</b>
	Maryland			-	PE				1437		00		-	-			
SOURCE	ITEM DESCRIPTION	QUANTITY	UNIT OF			U	INIT COSTS					Т	OTAL COSTS			Ι	TOTAL
SOONCE		QUANTIT	MEASURE		MATERIAL		LABOR	E	EQUIPMENT		MATERIAL		LABOR	EC	QUIPMENT		IUIAL
	DEMOLITION																
	DEMOLITION			\$	-	\$		\$	_	\$	_	\$	- 1	\$	-	\$	
				\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	
				\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
	NEW WORK							-									
Brentwood Budget Quote	60' Dia. Sludge Rapid Removal Clarifiers	2	ea	\$	494,750.00	\$	247,375.00	\$	-	\$	989,500	\$	494,750	\$	-	\$	1,484,250
	Clarifier Concrete	1300	CY	\$	400.00	\$	400.00	\$	-	\$	520,000	\$	520,000	\$	-	\$	1,040,000
	RAS Pump Station	1	LS	\$	500,000.00	\$	-	\$	-	\$	500,000	\$	-	\$	-	\$	500,000
Evoqua Budget Quote	Closed Vessel Low Pressure UV System	1	ls	\$	357,448.00	\$	107,234.40	\$	-	\$	357,448	\$	107,234	\$	-	\$	464,682
Leopold Budget Quote	Denitrification Filters	1	ls	\$	1,500,000.00	\$	450 000 00	\$	_	\$	1,500,000	\$	450,000	\$	-	\$	1,950,000
	Denitrifcation Concrete	300	CY	\$	400.00		400.00		-	\$	120,000		120,000		-	\$	240,00
	Internal Recycle Pumps	8	ea	\$	25,000.00	\$	7,500.00			\$	200,000	\$	60,000	\$	-	\$	260,00
	Fine Bubble Diffusers and Blowers	1	LA	\$	250,000.00	\$	100,000.00	\$	-	\$	250,000	\$	100,000	\$	-	\$	350,00
	Reactor Tank Modifications	1	LS	\$	40,000.00	\$	12,000.00	\$	-	\$	40,000	\$	12,000	\$	-	\$	52,000
	Reactor Tank Concrete	400	CY	\$	400.00	\$	400.00	\$	-	\$	160,000	\$	160,000	\$	-	\$	320,000
	Misc Process Piping and Equipment	1	LS	\$	150,000.00	\$	-	\$	-	\$	150,000	\$	-	\$	-	\$	150,000
	Aerobic Digester Tank Modifications	1	LS	\$	50,000.00	\$	-	\$	-	\$	50,000	\$	-	\$	-	\$	50,000
	Pre-Eq Pumps, Blowers and Diffusers	2	LS	\$	400,000.00	\$	200,000.00	\$	-	\$	800,000	\$	400,000	\$	-	\$	1,200,00
	SUBTOTAL DIRECT COSTS			-						\$	5,636,948	\$	2,423,984	\$		\$	8,060,93
	SUBCONTRACTOR MARKUP			-		-				\$	788,454		1,364,452		-	\$	2,152,90
	SUBTOTAL									\$	6,425,402		3,788,437		-	\$	10,213,83
	PRIME CONTRACTOR MARKUP									\$	1,432,189	\$	844,423		-	\$	2,276,61
	<b>BASE BID DIVISION 3 - TOTAL COSTS</b>									\$	7,857,591	\$	4,632,860	\$	-	\$	12,490,45

PROJECT NAME:	Centreville WWTP ENR Upgrade and	d Expansi	on PER	CLIENT: Town of C	Centreville	ESTIMATED BY:	A. DE	1			V			
PROJECT LOCATION:	Centreville,			DESIGN SUBMISSIC		WORK ORDER NUM	,	·			VI	R		/
ROJECT LOCATION:	Maryland			-	ER	1437		00			•			*
SOURCE	ITEM DESCRIPTION	QUANTITY	UNIT OF		UNIT COSTS	I				TAL COSTS			_	TOTAL
			MEASURE	MATERIAL	LABOR	EQUIPMENT		MATERIAL		LABOR	EQUIPN	/ENT		
	DEMOLITION													
	Demolition	1	LS	\$-	\$ 100,000.00	\$ -	\$	-	\$	100,000	\$	-	\$	100,00
	Demonitori		10	ψ -	φ 100,000.00	ψ -	Ψ		Ψ	100,000	Ψ	-	Ψ	100,00
	NEW WORK				1	1								
	Filter Building		<b></b>	<b>A</b> 000 000 00	<b>A</b> 00 000 00			000 000	-	00.007	<b>^</b>		•	
	600A Motor Control Center (MCC-A)	1	EA		\$ 20,000.00		\$	200,000		20,000		-	\$	220,0
	Branch Circuit Wiring from MCC-A New Feeder for 600 MCC-A	1	LS LS		\$ 150,000.00	\$- \$-	\$ \$	100,000 15,000		150,000		-	\$ \$	250,0
	Existing Panel DP modifications including new	1	LS	\$ 15,000.00	\$ 3,500.00	<b>Ъ</b> -	\$			3,500	\$	-	\$	18,5
	breakers and branch circuits	1	LS	\$ 30,000.00	\$ 15,000.00	\$-	\$	30,000	\$	15,000	\$	-	\$	45,0
	Lighting and Branch Wiring	2000	SF	\$ 7.00	\$ 5.00	\$-	\$	14,000	\$	10,000	\$	-	\$	24,0
	Lab Duilding													
	Lab Building 208V Panelboard	2	EA	\$ 10,000.00	\$ 2,000.00	\$ -	\$	20,000	¢	4,000	¢	-	\$	24.0
	Lighting and Branch Wiring	750	SF	\$ 10,000.00	, ,	· ·	۰ ۶	4,500		3,000		-	ֆ \$	7,5
	Receptacles including branch wiring	750	SF	\$ 2.00		· ·	\$	1,500		2,250		-	\$	3,7
	Dewatering Building													
	480V Panelboard	1	EA	\$ 15,000.00	\$ 3,000.00	\$ -	\$	15,000	¢	3,000	\$	-	\$	18,0
	208V Panelboard	2	EA	\$ 10,000.00			\$	20,000		4,000		-	\$	24,0
	Lighting and Branch Wiring	1900	SF	\$ 7.00			\$	13,300		9,500		-	\$	22,8
	Receptacles including branch wiring	1900	SF	\$ 3.00			\$	5,700		7,600		-	\$	13,3
	Branch circuits for mechanical loads	1900	SF		\$ 8.00		\$	11,400		15,200		-	\$	26,6
	Dry type transformer 45kVA	2	EA	\$ 2,500.00			\$	5,000		2,500		-	\$	7,5
	Outside													
	600A Motor Control Center (MCC-B) including VFDs	1	EA	\$ 200,000.00	\$ 40,000.00	\$-	\$	200,000	\$	40,000	\$	-	\$	240,0
	Branch Circuit Wiring from MCC-B including underground ducts	1	LS	\$ 150,000.00	\$ 150,000.00	\$-	\$	150,000	\$	150,000	\$	-	\$	300,0
	Feeder for Dewatering Building	1	LS	\$ 10.000.00	\$ 20,000.00	\$-	\$	10,000	\$	20,000	\$	-	\$	30,0
	Feeder for Control Building	1	LS	\$ 5,000.00			\$	5,000		8,000		-	\$	13,0
	Site Lighting and Branch Wiring	1	LS		\$ 40,000.00		\$	40,000		40,000		-	\$	80,0
	Testing and Commisioning	1	LS	\$-	\$ 40,000.00	\$-	\$		\$	40,000	\$	-	\$	40,0
	Grounding and Bonding	1	LS		\$ 50,000.00		\$	25,000		50,000		-	\$	75,0
	SUBTOTAL DIRECT COSTS						\$	885,400	\$	697,550	\$	_	\$	1,582,9
	SUBCONTRACTOR MARKUP						φ \$	250,214		392,691		-	\$	642,9
	SUBCONTRACTOR MARKOP						φ \$	1,135,614		1,090,241		-	\$	2,225,8
							ֆ \$		_				\$ \$	2,225,0
	PRIME CONTRACTOR MARKUP BASE BID DIVISION 16 - TOTAL COSTS						\$ \$	253,123 1,388,737		243,009 <b>1,333,250</b>		-	\$ \$	496,1 <b>2,721,9</b>

DETAILED	COST: ALTERNATIVE 3 - MBR ACTIVA	TED SLU	DGE														
PROJECT NAME:	Centreville WWTP ENR Upgrade and E	xpansion	PER	CLIE	INT: Town of C	entr	eville	ESTI	IMATED BY: W	RA					R	A	
PROJECT LOCATION:	Centreville,			DES	GIGN SUBMISSION:			WOF	RK ORDER NUM	BER:			V	A.			<b>/</b>
	Maryland				PE	R			1437	5-0	00		-				-
SOURCE	ITEM DESCRIPTION	QUANTITY	UNIT OF MEASURE		MATERIAL	U	NIT COSTS LABOR	E	EQUIPMENT		MATERIAL	T	DTAL COSTS	E	QUIPMENT		TOTAL
	DEMOLITION			\$	-	\$	_	\$	-	¢		\$	-	\$	-	\$	
				\$ \$	-	ֆ \$		э \$	-	\$ \$		\$	-	ծ \$	-	э \$	-
				\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
	NEW WORK									·							
Evoqua Budget Quote	Closed Vessel Low Pressure UV System (1/2 for MBR)	1	ls	\$	357,448.00	\$	107,234.40	\$	-	\$	357,448	\$	107,234	\$	-	\$	464,682
Quet	MBR Equipment	1	ls	¢	4,000,000.00	¢		\$		\$	3,000,000	¢	1,000,000	¢		\$	4,000,000
Quot	New MBR Process Building	400	SF	\$	4,000,000.00		- 150.00		-	φ \$	60,000		60,000		-	\$	120,000
																· ·	
	Reactor Tank Modifications	1	LS	\$	40,000.00		12,000.00		-	\$	40,000		12,000		-	\$	52,000
	Reactor Tank Concrete	400	CY	\$	400.00	\$	400.00	\$	-	\$	160,000	\$	160,000	\$	-	\$	320,000
	Misc Process Piping	1	LS	\$	500,000.00	\$	-	\$	-	\$	500,000	\$	-	\$	-	\$	500,000
	Pre-Eq Pumps, Blowers and Diffusers	2	LS	\$	400,000.00	\$	200,000.00	\$	-	\$	800,000	\$	400,000	\$	-	\$	1,200,000
	Aerobic Digester Tank Modifications	1	LS	\$	50,000.00	\$	-	\$	-	\$	50,000	\$	-	\$	-	\$	50,000
	SUBTOTAL DIRECT COSTS									\$	4,967,448	\$	1,739,234	\$	-	\$	6,706,682
	SUBCONTRACTOR MARKUP									\$	454,986	\$	979,009	\$	-	\$	1,433,995
	SUBTOTAL									\$	5,422,434		2,718,243		-	\$	8,140,677
	PRIME CONTRACTOR MARKUP									\$	1,208,633		605,882		-	\$	1,814,515
	BASE BID DIVISION 4 - TOTAL COSTS									\$	6,631,067	\$	3,324,126	\$	-	\$	9,955,192

DETAILED	COST: ALTERNATIVE 3 (MBR AC	TIVATE	O SLUDO	E) - ELEC	FRICAL						
PROJECT NAME:	Centreville WWTP ENR Upgrade and	d Expansi	on PER	CLIENT: Town of (	Centreville	ESTIMATED BY: WR/	A, DEI		<b>NF</b>		
PROJECT LOCATION:	Centreville,			DESIGN SUBMISSIC	N:	WORK ORDER NUM	/BER-				<b>~</b>
	Maryland			-	ER		'5-000	-		_	
			UNIT OF	1		1407	1	70741 00070		-	
SOURCE	ITEM DESCRIPTION	QUANTITY	UNIT OF MEASURE	MATERIAL	UNIT COSTS LABOR	EQUIPMENT	MATERIAL	TOTAL COSTS LABOR	EQUIPMENT		TOTAL
				WATERIAL	LABOR	EQUIFMENT	WATERIAL	LABOR	EQUIFINENT		
	DEMOLITION										
		4	10	¢	¢ 100 000 00	\$ -	¢	¢ 100.000	L c	L C	100.000
	Demolition	1	LS	\$ -	\$ 100,000.00	\$ -	\$-	\$ 100,000	\$ -	\$	100,000
	NEW WORK										
	NEW WORK										
	Filter Building										
	600A Motor Control Center (MCC-A)	1	EA	\$ 200.000.00	\$ 20,000.00	\$-	\$ 200,000	\$ 20,000	\$ -	\$	220,000
	Branch Circuit Wiring from MCC-A	1	LS	\$ 100,000.00	\$ 150,000.00		\$ 100,000			\$	250,000
	New Feeder for 600 MCC-A	1	LS	\$ 15,000.00			\$ 15,000			\$	18,500
	Existing Panel DP modifications including new	1	LS			s -	\$ 30,000	\$ 15,000	\$ -	\$	45,000
	breakers and branch circuits			\$ 30,000.00	\$ 15,000.00	1		· · ·			
	Lighting and Branch Wiring	2000	SF	\$ 7.00	\$ 5.00	· ·	\$ 14,000	\$ 10,000	\$ -	\$	24,000
						\$ -					
	Lab Building					\$ -			-	-	
	208V Panelboard	2	EA		\$ 2,000.00	\$ -	\$ 20,000			\$	24,000
	Lighting and Branch Wiring	750	SF	\$ 6.00	\$ 4.00	\$ -	\$ 4,500			\$	7,500
	Receptacles including branch wiring	750	SF	\$ 2.00	\$ 3.00	\$ -	\$ 1,500	\$ 2,250	\$ -	\$	3,750
	MBR Process Building									_	
	480V Panelboard	1	EA	\$ 15,000.00	\$ 3,000.00	\$-	\$ 15,000	\$ 3,000	\$ -	\$	18,000
	208V Panelboard	2	EA	\$ 10,000.00	\$ 2,000.00		\$ 20,000			\$	24,000
	Lighting and Branch Wiring	400	SF	\$ 7.00	\$ 5.00		\$ 2,800			\$	4,800
	Receptacles including branch wiring	400	SF	\$ 3.00	\$ 4.00		\$ 1,200			\$	2,800
	Branch circuits for mechanical loads	400	SF	\$ 6.00	\$ 8.00	\$ -	\$ 2,400			\$	5,600
	Dry type transformer 45kVA	2	EA	\$ 2,500.00	\$ 1,250.00	\$-	\$ 5,000	\$ 2,500	\$ -	\$	7,500
	Dewatering Building										
	480V Panelboard	1	EA				\$ 15,000			\$	18,000
	208V Panelboard	2	EA	\$ 10,000.00	\$ 2,000.00		\$ 20,000			\$	24,000
	Lighting and Branch Wiring	1900	SF				\$ 13,300			\$	22,800
	Receptacles including branch wiring Branch circuits for mechanical loads	1900	SF	\$ 3.00 \$ 6.00	\$ 4.00	\$ -	\$ 5,700			\$	13,300
	Dry type transformer 45kVA	1900 2	SF	\$ 6.00 \$ 2,500.00	\$ 8.00 \$ 1,250.00	\$ - \$ -	\$ 11,400 \$ 5,000			\$	26,600 7,500
		2	LA	\$ 2,300.00	φ 1,230.00	φ -	φ 5,000	φ 2,500	φ -	φ	7,500
	Outside									_	
	600A Motor Control Center (MCC-B) including										
	VFDs	1	EA	\$ 300,000.00	\$ 40,000.00	\$ -	\$ 300,000	\$ 40,000	\$-	\$	340,000
	Branch Circuit Wiring from MCC-B including	4	10	¢ 250 000 00	¢ 000 000 00	¢	\$ 250,000	¢ 000.000	\$ -	¢	450.000
	underground ducts	1	LS	\$ 250,000.00	\$ 200,000.00	\$ -	\$ 250,000	\$ 200,000	\$-	\$	450,000
	Feeder for MBR Process Building	1	LS	\$ 10,000.00	\$ 20,000.00	\$-	\$ 10,000	\$ 20,000	\$ -	\$	30,000
	Feeder for Dewatering Building	1	LS	\$ 10,000.00	\$ 20,000.00	\$ -	\$ 10,000			\$	30,000
	Feeder for Control Building	1	LS	\$ 5,000.00			\$ 5,000			\$	13,000
	Site Lighting and Branch Wiring	1	LS				\$ 40,000			\$	80,000
	Diesel Generator 750kW including ATS	1	EA	\$ 500,000.00	\$ 50,000.00	\$ -	\$ 500,000	\$ 50,000	\$ -	\$	550,000
	Testing and Commisioning	1	LS	\$-	¢ 40.000.00	¢	¢	¢ 40.000	¢	\$	40.000
	Grounding and Bonding	1	LS	\$ 25,000.00	\$ 40,000.00 \$ 50,000.00		\$ - \$ 25.000	\$ 40,000 \$ 50,000		\$	40,000 75,000
	orounding and bonding			φ 20,000.00	\$ 50,000.00	<i>y</i> -	φ 23,000	φ 50,000	φ -	φ	73,000
	SUBTOTAL DIRECT COSTS		1				\$ 1,641,800	\$ 833,850	\$ -	\$	2,475,650
	SUBCONTRACTOR MARKUP						\$ 463,973			\$	933,394
	SUBTOTAL						\$ 2,105,773			\$	3,409,044
	PRIME CONTRACTOR MARKUP						\$ 469,366			\$	759,858
E	BASE BID DIVISION 16 - TOTAL COSTS						\$ 2,575,139	\$ 1,593,764	\$-	\$	4,168,903



Appendix B

# Existing NPDES Stream and Spray Discharge Permits



# DISCHARGE PERMIT



Ben Grumbles, Secretary Horacio Tablada, Deputy Secretary

# DISCHARGE PERMIT

NPDES Discharge Permit Number:	MD0020834	State Discharge Permit Number:	20-DP-0116
Effective Date:	12/01/2021	Expiration Date:	11/30/2026
Modification Date:	(Not applicable)	Reapplication Due Date:	11/30/2025

Pursuant to the provisions of Title 9 of the Environment Article, <u>Annotated Code of Maryland</u>, and regulations promulgated thereunder, and the provisions of the Clean Water Act, 33 U.S.C. Section 1251 <u>et seq.</u>, and implementing regulations 40 CFR Parts 122, 123, 124 and 125, the Department of the Environment hereby establishes conditions and requirements pertinent to the wastewater treatment plant and collection system and authorizes:

Town of Centreville 101 Lawyers Row Centreville, Maryland 21617

TO DISCHARGE FROM: Centreville Wastewater Treatment Plant

LOCATED AT: 116 Johnstown Lane Centreville, Queen Anne's County Maryland 21617

THROUGH OUTFALL: 001A (WWTP Effluent)

TO: Gravel Run, designated as Use – I waters, which is protected for water contact recreation and nontidal warmwater aquatic life; in accordance with the following special and general conditions and a map incorporated herein and made a part hereof.

- A. "Ambient temperature" of the effluent receiving stream means the water temperature that is not impacted by a point source discharge, and it shall be measured in areas of the stream representative of typical or average conditions of the stream segment in question.
- B. "Bypass" means the intentional diversion of pollutants from any portion of a treatment or collection facility.
- C. "BOD<sub>5</sub> (Biochemical Oxygen Demand)" means the amount of oxygen consumed in a standard BOD<sub>5</sub> test without the use of a nitrification inhibitor at 20 degree centigrade on an unfiltered sample.
- D. "Clean Water Act" means the Federal Water Pollution Control Act, as amended, 33 U.S.C. Section 1251 <u>et seq</u>.
- E. "CFR" means the Code of Federal Regulations.
- F. "COMAR" means the Code of Maryland Regulations.
- G. "Department" means the Maryland Department of the Environment (MDE).
- H. Discharge Limits
  - 1. "Daily *maximum* (or *minimum*)" limitation means the *highest* (or *lowest*) allowable the daily averages in a calendar month. The daily discharge expressed as concentration (in mg/l) shall be calculated by dividing total of measurement readings by number of sample collected during a calendar day or any 24-hour period that reasonably represents the calendar day for purposes of sampling. The daily discharge expressed as loading rate (in pounds/day) is calculated by using this formula {daily average concentration (mg/l) x the same day total flow (in million gallons) x 8.34}.
  - 2. "Weekly average (maximum or minimum)" limitation means the highest or lowest allowable average of "daily discharges" over a calendar week, calculated as the sum of all "daily discharges" measured during a calendar week divided by the number of "daily discharges" measured during that week. Each of the following 7-day periods is defined as a calendar week: Week 1 is Days 1 7 of the month; Week 2 is Days 8 14; Week 3 is Days 15 21; and Week 4 is Days 22 28. For weekly average maximum, if the "daily discharge" on days 29, 30 or 31 exceeds the "weekly average" discharge limitation, MDE may elect to evaluate the last 7 days of the month as Week 4 instead of Days 22 28. For weekly average minimum, if the "daily discharge" on days 29, 30 or 31 is lower than the "weekly average" discharge limitation, MDE may elect to evaluate the last 7 days of the month as Week 4 instead of Days 22 28.

- 3. "Monthly average *maximum* (or *minimum*)" limitation means the *highest* (or *lowest*) allowable monthly average concentration or waste load of a parameter over a calendar month. The monthly average is calculated as the sum of all daily discharges for a parameter sampled and/or measured in that calendar month divided by the number of days on which monitoring was performed.
- 4. "Minimum or maximum" limit means the lowest or highest allowable value measured during a calendar day or any 24-hour period that reasonably represents the calendar day for purposes of sampling.
- 5. "Monthly loading rate (in pounds/month)" means the total load of a parameter calculated for that calendar month. It is calculated using this formula {(monthly average concentration in mg/l) x (Total monthly flow in Million Gallons) x 8.34}.
- 6. "Year-to-date cumulative load (pounds)" value means cumulative load of a pollutant in the effluent through each reporting month in a calendar year. It is calculated as a sum of the individual total monthly loads from January through the reporting month in a calendar year.
- "Annual Maximum Loading Rate (in pounds/year)" limit means the maximum load allowed for a pollutant in the effluent to be discharged in a calendar year. The Year-to-date cumulative load (as defined above in Definition I.H.6) shall be used to determine the compliance status of this requirement.
- 8. "Monthly log mean (Monthly geometric mean)" limit means the highest allowable value calculated as the logarithmic <u>or</u> geometric mean of all samples taken in the calendar month. The geometric mean is the antilogarithm of the mean of the logarithms.
- I. Discharge Monitoring
  - 1. "Composite sample" means a combination of individual samples obtained at hourly or smaller intervals over a time period. Either the volume of each individual sample is proportional to discharge flow rates or the sampling interval (for constant volume samples) is proportional to the flow rates over the time period used to produce the composite.
  - 2. "Grab sample" means an individual sample collected over a period of time not exceeding 15 minutes.
  - 3. "Estimated flow" value means a calculated volume or discharge rate which is based on a technical evaluation of the sources contributing to the discharge including, but not limited to, pump capabilities, water meters, and batch discharge volumes.

- 4. "Measured flow" value means any method of liquid volume measurement, the accuracy of which has been previously demonstrated in engineering practice, or for which a relationship to absolute volume has been obtained.
- 5. "Recorded flow" means any method of providing a permanent, continuous record of flow including, but not limited to, circular and strip charts.
- 6. "Monthly average flow" means the total flow for a calendar month divided by the number of days in the same month.
- J. "i-s (immersion stabilization)" means a calibrated device immersed in the effluent or stream, as applicable, until the temperature reading is stabilized.
- K. "NetDMR" means a nationally-available electronic reporting tool, initially designed by states and later adapted for national use by EPA, which can be used by NPDES-regulated facilities to submit discharge monitoring reports (DMRs) electronically to EPA through a secure Internet application over the National Environmental Information Exchange Network (NEIEN). EPA can then share this information with authorized states, tribes, and territories.
- L. "NPDES (National Pollutant Discharge Elimination System)" means the national system for issuing permits as designated by the Clean Water Act.
- M. "Nondetectable Level" for total residual chlorine means a residual concentration of less than 0.10 mg/l as determined using either the DPD titrimetric or chlorimetric method or an alternative method approved by the Department.
- N. "Outfall" means the location where the effluent is discharged into the receiving waters.
- O. "Overflow" means any loss of wastewater or discharge from a sanitary sewer system, combined sewer system or wastewater treatment plant bypass (as defined in I.B) which results in the direct or potential discharge of raw, partially treated wastewater into the waters of the State.
- P. "Permittee" means an individual or organization holding the discharge permit issued by the Department.
- Q. "POTW" means a publicly owned treatment works.
- R. "Sampling Point" means the effluent sampling location in the outfall line(s) downstream from the last addition point or as otherwise specified.
- S. "Sanitary Sewer Overflow (SSO)" means a discharge of untreated or partially treated sewage from a separate sewer system before the sanitary wastewater reaches the headworks of a wastewater treatment facility, pursuant to COMAR 26.08.10.01.

#### Permit No. 20-DP-0116 (NPDES MD0020834)

- T. "Secondary Treatment" means the treatment of sewage to produce effluent equal to or better than the following quality, except as provided for 40 CFR §133.103, or paragraphs (d), (e) or (f) of the same section:
  - 1. Five-day biochemical oxygen demand (BOD<sub>5</sub>):
    - a. 30 milligrams/liter average for a 30-day period;
    - b. 45 milligrams/liter average for a 7-day period;
    - c. The 30-day average percent removal shall not be less than 85 percent.
  - 2. Total Suspended Solids (TSS):
    - a. 30 milligrams/liter average for a 30-day period;
    - b. 45 milligrams/liter average for a 7-day period;
    - c. The 30-day average percent removal shall not be less than 85 percent.
  - 3. Bacterial Control: As required to meet water quality standards.
- U. "Significant Industrial User (SIU)" is defined as any industrial user (IU) that:
  - 1. is subject to national categorical standards; and
  - 2. any other IU that:
    - a. discharges an average of 25,000 gallons per day or more of process wastewater (excluding sanitary, non-contact cooling and boiler blowdown wastewater); or
    - b. contributes a process wastestream that makes up 5% or more of the average dry weather hydraulic or organic capacity of the POTW; or
    - c. is designated as such by the POTW on the basis that the IU has a reasonable potential for adversely affecting the POTW's operation or for violating any pretreatment standard or requirement; or
    - d. is found by the POTW, the Department, or the Environmental Protection Agency (EPA) to have significant impact either individually or in combination with other contributing industries to the POTW, on the quality of the sludge, the POTW's effluent quality, or air emissions generated by the system.

#### Permit No. 20-DP-0116 (NPDES MD0020834)

- V. "TKN (Total Kjeldahl Nitrogen)" means organic nitrogen plus ammonia nitrogen.
- W. "TSS (Total Suspended Solids)" means the residue retained on the filter by an analysis done in accordance with Standard Methods or other approved methods.
- X. "Upset" means the exceptional incident in which there is unintentional and temporary noncompliance with technology-based permit effluent limitations because of factors beyond the reasonable control of the permittee. An upset does not include noncompliance to the extent caused by operational error, improperly designed treatment facilities, inadequate treatment facilities, lack of preventive maintenance, or careless or improper operation

### **II. SPECIAL CONDITIONS**

### A. Effluent Limitations, Outfall $001A^{(1)(2)(3)(4)(21)}$

These limitations shall be applicable from December 1 through March 31 only. No stream discharge is permitted from April 1 through November 30 from Outfall 001A. The rest of the year, the wastewater will be disposed of by spray irrigation to the ground waters of the State, as regulated by Groundwater Discharge Permit No. 14-DP-3323. The quality of the effluent discharged by the facility at a discharge point location (Outfall 001A) shall be limited at all times as shown below:

		1	Maximum Efflu	ent Limits, excep	t as noted	
Effluent Characteristics	Monthly Average Loading Rate, Pounds/day	Weekly Average Loading Rate, Pounds/day	Daily Average Loading Rate, Pounds/day	Monthly Average Concentration, mg/l	Weekly Average Concentration, mg/l	Daily Average Concentration, 
BOD <sub>5</sub> (12/1 to 3/31)	130	190	N/A	28	42	-
						N/A
BOD <sub>5</sub> , Percent Removal <sup>(8)</sup>			85 % minimu	n monthly average	e	
TSS (12/1-3/31)	130	190	N/A	28	42	N/A
TSS, Percent Removal <sup>(8)</sup>			85 % minimu	n monthly average	•	

**Maximum Effluent Limits Total Monthly Annual Maximum Monthly Average** Loading Rate, Loading Rate, Concentration, **Effluent Characteristics Pounds/Month** Pounds/Season mg/l Total Phosphorus-P<sup>(4) (5)(6)</sup> 140 457 1.0 (12/1 - 3/31)Total Nitrogen-N<sup>(4(5(6)</sup> 750 3,004 5.5 (12/1 - 3/31)

	Effluent Limits	
Effluent Characteristics	Maximum	Minimum
E. coli	116 MPN/ 100 ml monthly geometric mean value	N/A
Total Residual Chlorine	(See footnote – 7)	N/A
рН	8.5	6.5
Dissolved Oxygen (All Year)	N/A	5.0 mg/l at anytime

### **II. SPECIAL CONDITIONS**

A. Effluent Limitations, Continued

An annual average flow of <u>0.542</u> million gallons per day (mgd) was used in waste allocation calculations (expressed as waste loading rate limit), and this unit shall be used when reporting on the Discharge Monitoring Report (DMR) as required by General Condition III.A.2. Notification is to be provided to the Department at least 180 days before the annual average flow is expected to exceed this flow level. If a permit modification is required, the Department will initiate the public participation NPDES process. Because this facility is authorized to discharge only 4 months per year, the permitted flow is equivalent to a minor facility. *Footnotes for effluent limitations:* 

- <sup>(1)</sup> When this permit is renewed, the new limitations may not be equal to the above limitations. There shall be no discharge of floating solids or visible foam other than trace amounts.
- <sup>(2)</sup> The permit may also be reopened in accordance with the requirements of MDE's Watershed Permitting Plan under which all discharge permits in a watershed are issued the same year.
- (3) The specific designated use of Corsica River of the Lower Chester River Mesohaline segment is Use II Support of Estuarine and Marine Aquatic Life and Shellfish Harvesting. The Maryland Department of the Environment (MDE) has identified the waters of the Corsica River of the Lower Chester River Mesohaline segment on the State's Integrated Report as impaired by the following pollutants (listing year and Integrated Report Assessment Unit Identification in parentheses): total suspended solids (1996; MD-CHSMH), nutrients (1996; MD-CHSMH), fecal coliform (1996; MD-CHSMH-Corsica\_River), and polychlorinated biphenyls (PCBs) in fish tissue (2002; MD-CHSMH-02130507) and impacts to biological communities (2004; MD-CHSMH). Nutrients, fecal coliform, and PCB TMDLs for the restricted shellfish harvesting portion of the Corsica River were approved by the US EPA in 2000, 2005, and 2011 respectively. This permit is in conformance with these TMDLs and the "Chesapeake Bay TMDL for Nitrogen, Phosphorus, and Sediment" established on December 29, 2010. When TMDLs for other remaining parameters are completed, limits may be imposed, after the public participation process, to incorporate any TMDL requirements.
- (4) The loading caps for the Centreville WWTP for the seasonal stream discharge from December 1 through March 31, equal to 3,004 pounds for TN and 457 pounds for TP. The permittee shall also comply with the monthly loading cap limits of 750 lbs/month and 140 lbs/months for TN and TP respectively. The first exceedance of the permit limit shall be counted and reported as daily exceedances beginning from the first exceedance, determined to the nearest day, through March 31. In addition, after any such exceedance, the permittee shall demonstrate to the Department's satisfaction that the facility is optimizing its nutrient removal capability, and neither the arrival of the next season (December 1 thru March 31) nor the issuance of a permit renewal during a period of noncompliance shall obviate continuance of any noncompliance status related to treatment optimization requirements.
- (5) The current plant operates the Biological Nutrient Removal (BNR) process on a year round basis and the Town is also authorized under groundwater permit GW 14-DP-3323 to operate the spray irrigation system 365 days a year. Consequently, the level of nutrient control that is achieved by the combined surface water and ground water systems is equivalent to Enhanced Nutrient Removal (ENR) level treatment and an ENR upgrade at this plant is not required.
- <sup>(6)</sup> The permittee may request that the permit be reopened and modified to include nutrient trading consistent with the most current "Maryland Policy for Nutrient cap Management and Trading in Maryland's Chesapeake Bay Watershed" in effect at that time.
- <sup>(7)</sup> Total residual chlorine limitation of the nondetectable level shall be applicable, when chlorine or any chlorinecontaining compound is used in any treatment process(es), including but not limited to disinfection, that could become a potential constituent of the effluent discharged from the Centreville WWTP. The wastewater shall be dechlorinated to reduce effluent total residual chlorine concentration to the nondetectable level (See definition I.M).
- <sup>(8)</sup> In accordance with 40CFR \$133.102, the 30-day average percent removal for BOD<sub>5</sub> and TSS shall not be less than 85 (eighty-five) percent as the minimum level of effluent quality attainable by the secondary treatment. Refer to the footnotes 22 and 23 for further details for calculations and reporting requirements toward compliance to the BOD<sub>5</sub> and TSS percent removal effluent limitations (See Definition I.T).

#### Permit No. 20-DP-0116 (NPDES MD0020834)

### **II. SPECIAL CONDITIONS**

### B. (1) (a) Minimum Monitoring Requirements<sup>(21)</sup>:

The effluent characteristics listed below in Table B shall be monitored at the sampling point (Definition I.R). If the sampling point is other than the outfall 001A, the permittee shall ensure that the effluent samples taken at the above stated sampling point are representative of the effluent quality discharged at the Outfall 001A.

Effluent Characteristics	Monitoring Period	<u>Measurement</u> Frequency	<u>Sample Type</u>
BOD <sub>5</sub> <sup>(9)(19)</sup>	All Year	Two per week	24-hour composite
Total Suspended Solids <sup>(9)(19)</sup>	All Year	Two per week	24-hour composite
BOD <sub>5</sub> , Percent Removed <sup>(9)((22)(23)</sup>	All Year	One per month	Calculated
TSS, Percent Removed <sup>(9)(22)(23)</sup>	All Year	One per month	Calculated
TKN <sup>(9)(10)(11)(12)(19)</sup>	All Year	Two per week	24-hour composite
Total Ammonia Nitrogen as N (9)(10)(11)(12)(19)	All Year	Two per week	24-hour composite
Total Phosphorus as P <sup>(9)(11)(13)(19)</sup>	All Year	Two per week	24-hour composite
Total Nitrogen as N $^{(9)(12)(13)(19)}$	All Year	Two per week	Calculated
(Nitrite + Nitrate) as N <sup>(9)(10)(11)(12)(19)</sup>	All Year	Two per week	24-hour composite
Organic Nitrogen as N <sup>(9)(10)(12)(19)</sup>	All Year	Two per week	Calculated
Orthophosphate as P <sup>(9)(10)(11)(19)</sup>	All Year	Two per week	24-hour composite
E. coli <sup>(9)(19)</sup>	All Year	Two per week	Grab
Total Residual Chlorine <sup>(9)(14)(15)</sup>	All Year	Two per day	Grab
Dissolved Oxygen <sup>(9)(15)</sup>	All Year	Two per day	Grab
pH <sup>(9)(15)</sup>	All Year	Two per day	Grab
Flow <sup>(9)(16)(17)(20)</sup>	All Year	Continuous	Recorded
Total Flow <sup>(9)(18)(20)</sup>	All Year	Monthly	Calculated

### **II. SPECIAL CONDITIONS**

#### B. (1) (b) Raw Wastewater Influent at Sampling Point 101A:

The quality of the wastewater influent entering the Centreville WWTP shall be monitored at Influent Chamber All the times as shown below:

<u>Wastewater Influent</u> <u>Characteristics</u>	<u>Monitoring Period</u>	Measurement Frequency	<u>Sample Type</u>
BOD <sub>5</sub> <sup>(9)(22)(23)</sup>	All Year	Two per month	Grab
Total Suspended Solids <sup>(9)(22)(2</sup>	All Year	Two per month	Grab

#### B. Minimum Monitoring Requirements, continued:

Footnotes for the monitoring requirements (B)(1)(a) and (B(1)(b)):

- (14) The Minimum monitoring requirements of Two per day-grab samplings for total residual chlorine shall be applicable, when chlorine or any chlorine compound is used in any treatment process(es), including but not limited to disinfection, that could become a potential constituent of the effluent discharged from the Centreville WWTP. The minimum level (quantification level) for total residual chlorine is 0.10 mg/l. The permittee may report all results below the minimum level as <0.10 mg/l. All results reported below the minimum level shall be considered in compliance.</p>
- <sup>(15)</sup> Samples for these parameters (total residual chlorine, pH and dissolved oxygen) shall be taken at intervals evenly distributed throughout the staffed period each day to comply with the General Condition III.A.1 for the representative sampling requirements.

<sup>(9)</sup> "STORET" (short for STOrage and RETrieval) is a widely-used repository for water quality data reporting and monitoring. The STORET codes for the effluent characteristics described as limitations and/or monitoring requirements are: BOD<sub>5</sub> (00310), BOD<sub>5</sub> percent removed (81010), Total Suspended Solids (00530), Total Suspended Solids percent removed (81011), TKN (00625), Total Ammonia Nitrogen as N (00610), Total Phosphorus as P (00665), Total Nitrogen as N (00600), (Nitrite + Nitrate) as N (00630), Organic Nitrogen as N (00605), Orthophosphate as P (04175), Fecal Coliform (74055), E. Coli (51040), Total Residual Chlorine (50060), Dissolved Oxygen (00300), pH (00400), Flow (50050), and Total monthly flow (82220)(10)This parameter (without effluent limitations) must be monitored, and it shall be reported on the Monthly Operating Report (MOR) as individual results and on the Discharge Monitoring Report as monthly average concentrations. (11) The monitoring of total phosphorus, total ammonia nitrogen, TKN, (nitrite + nitrate)-N and orthophosphate shall be two per week 24 hour composite samplings.

<sup>&</sup>lt;sup>(12)</sup> Total nitrogen as N (in mg/l) is a calculated parameter as the sum of individual results for total ammonia nitrogen as N, organic nitrogen as N and (nitrite + nitrate) as N. Total Kjeldahl Nitrogen (TKN) is defined as the total concentration of organic nitrogen and ammonia as N. All nitrogen species must be sampled at the same day. The monitoring result for organic nitrogen may be calculated through the subtraction of the total Ammonia as N monitoring result from the result of TKN sample taken at the same day.

<sup>&</sup>lt;sup>(13)</sup> The permittee shall also calculate and report on the DMR the TN and TP total monthly loads (Definition I.H.5) plus seasonal cumulative December 1 thru March 31loads (Definition I.H.6) for the outfall- 001A.

### **II. SPECIAL CONDITIONS**

- <sup>(16)</sup> Flows shall be reported in million gallons per day (mgd) to at least the nearest 1,000 gallons per day. (Example: A flow of 524,699 gallons per day shall be reported as 0.525 mgd.). For each calendar month, flows shall be reported on the MOR as daily individual results and on the DMR as monthly average (mgd) and daily maximum (mgd).
- <sup>(17)</sup> Continuous electronic flow measurement and recording which can produce a permanent record are acceptable to the Department.
- <sup>(18)</sup> Total monthly flow is a calculated parameter equal to sum of the daily flow results in a calendar month. It shall be reported on the monthly DMR as Total monthly flow in million gallons (MG) to at least the nearest 1,000 gallons. (Example: A flow of 15,524,699 gallons shall be reported as 1.53 MG).
- <sup>(19)</sup> The permittee shall distribute the timing for effluent sampling with minimum of 48-hour apart for two per week monitoring frequencies. The 48 hours interval for two per week sampling shall be defined as the period between the starting times of the two consecutive effluent sample collections for the same effluent parameter.
- <sup>(20)</sup> Effluent flow to outfall 001A and to the spray irrigation system shall be measured and reported year round on the monthly DMR reports.
- <sup>(21)</sup> See General Condition III.A.2.a.ii.
- $^{(22)}$  The BOD<sub>5</sub> and TSS in the raw wastewater influent and effluent shall be sampled on the same day. The measurements shall be utilized to calculate the BOD<sub>5</sub> and TSS percent removed using the formula listed below in the footnote 23, and the results shall be used to complying with the Percent removal limits of BOD<sub>5</sub> and TSS (Special Condition II.A).

Upon the effective date of the discharge permit, if the DMR records from the last 12 months indicate the average removal efficiencies for these pollutants at the facility are significantly higher than the required 85%, the permittee may petition for performance – based monitoring frequency reduction for BOD<sub>5</sub> and TSS in the raw wastewater influent.

<sup>(23)</sup> At the end of each calendar month, the monthly percent (%) of the parameter (BOD<sub>5</sub> and TSS) removed shall be calculated using the following formula:

Monthly Percent (%) of Parameter Removed =  $((A-B)/A) \times 100$ 

Where:

- A = Monthly Average Concentration of Parameter in Influent in mg/l
- B = Monthly Average Concentration of Parameter in Effluent in mg/l



# STATE GROUNDWATER DISCHARGE PERMIT



Larry Hogan, Covernor Boyd K. Rutherford, Lt. Governor

Ben Grumbles, Secretary Horacio Tablada, Deputy Secretary

#### STATE GROUNDWATER DISCHARGE PERMIT

STATE DISCHARGE PERMIT NUMBER	20-DP-3323
NPDES ID NUMBER	MD3323R05
EFFECTIVE DATE	01/01/2022
EXPIRATION DATE	12/31/2026

Pursuant to the provisions of Title 9 of the Environment Article, <u>Annotated Code of Maryland</u>, and regulations promulgated thereunder, the Department of the Environment, hereinafter referred to as "the Department", hereby authorizes

#### The Town of Centreville 101 Lawyer's Row Centreville, Maryland 21617

hereinafter referred to as "Permittee", to discharge treated wastewater by spray irrigation as described herein, from:

Centerville Wastewater Irrigation Facility 751 Hope Road Centreville, Maryland 21617

to groundwaters of the State in accordance with the following special and general conditions, including the attached maps made a part hereof.

www.mde.maryland.gov

### I. SPECIAL CONDITIONS

#### A. <u>Waste and Wastewater Limitations</u>

- 1. This Permittee is authorized to discharge treated wastewater via spray irrigation to ground waters of the State at the site shown on Maps A & B up to a maximum annual average flow of 0.542 million gallons per day. The authorized discharge period is March 1 through December 15.
- 2. Prior to discharge at the spray irrigation site, all wastewaters shall be treated to produce an effluent which does not exceed the following maximum limitations and is in accordance with the approved nutrient management plan required under Condition I.B.7.

		Ef	fluent Limita	tions	Monitoring R	equirements
Parameter		Load	ling	Concentration		
Code (STORET)	Effluent Parameter	Monthly Average	Yearly Maximum	Monthly Average	Monitoring Frequency	Sample Type <sup>(4)(5)</sup>
50050	Flow	0.542 mgd <sup>(1)</sup>	N/A	N/A	Continuous	Recorded
00310	BOD <sub>5</sub>	N/A	N/A	30 mg/l	Twice/Week	8hr-Comp
00530	Suspended Solids	N/A	N/A	30 mg/l	Twice/Week	8hr-Comp
00400	pH	N/A	N/A	6.5 - 8.5 <sup>(2)</sup>	Daily	Grab
00600	Total Nitrogen (N) <sup>(3)</sup>	N/A	N/A	8 mg/l	Twice/Week	8hr-Comp
00625	TKN	N/A	N/A	Report Value	Twice/Week	8hr-Comp
00630	Nitrate + Nitrite (N+N)	N/A	N/A	Report Value	Twice/Week	8hr-Comp
74055	Fecal Coliform <sup>(6)</sup>	N/A	N/A	200 MPN/100ml	Twice/Week	Grab

(1) This is a yearly average sewage flow. Flow shall be measured via flow measurement device installed at spray irrigation control building and evaluated on Calendar year basis.

- (2) These are minimum (6.5) and maximum (8.5) values of pH.
- (3) A permit modification is required for any future expansion of this facility. Such a modification shall include a yearly nitrogen load limitation to groundwater of no more than 13,199 lbs/year at the spray irrigation system. The 13,199 lbs/year nitrogen loading requirement was determined based on 0.542 mgd average daily flow and 8 mg/l effluent nitrogen concentration. Total nitrogen is defined as the sum of Nitrate plus Nitrite (N+N) and Total Kjeldahl Nitrogen (TKN). The concentration of each constituent shall also be reported. This nitrogen loading cap is not an assigned allocation for discharge to the Bay via groundwater because other natural processes reduce the amount of nitrogen reaching the Bay from this system. The factsheet of this permit includes calculations for estimating the amount of nitrogen delivered to the nearby surface water from this system.
- (4) Composite samples shall be obtained from the effluent line leaving the wastewater treatment plant.
- (5) Grab samples shall be obtained from the effluent line just prior to entering the storage lagoon.
- (6) The fecal coliform shall be determined as a geometric mean of the monthly data.

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### I. <u>SPECIAL CONDITIONS</u>

- 3. Groundwater samples taken from ten (10) groundwater monitoring wells per requirements of Section I.C.2. shall be monitored by the permittee according to the following limitations:
  - a. The discharge of the wastewater authorized in this permit shall not cause groundwater quality to exceed the limitations listed below, as measured in the designated down gradient monitoring wells (MWs 2, 3, 4, 5, 6, 7, 9 and 10 shown on Map B). The Table below includes limitations based on the drinking water standards for NO<sub>2</sub> (Nitrite 00615), Total Dissolved Solids (70295), Chloride (00940), and Fecal Coliform (74055):

Parameter Code (STORET)	Parameter	Groundwater Quality Yearly Average Limitations <sup>(2) (3)</sup>	Measurement Frequency	Sample Type
00620	NO <sub>3</sub> (Nitrate)	Footnote (1)	Once every 3 months	Grab
00615	NO <sub>2</sub> (Nitrite)	l mg/l	Once every 3 months	Grab
00625	TKN	Footnote (1)	Once every 3 months	Grab
00600	Total Nitrogen (TKN+NO <sub>2</sub> +NO <sub>3</sub> )	Footnote (1)	Once every 3 months	Grab
00400	pH	Footnote (1)	Once every 3 Months	Grab
00650	PO <sub>4</sub> (Total Phosphate)	Footnote (1)	Once every 3 months	Grab
70295	Total Dissolved Solids	500 mg/l	Once every 3 months	Grab
00940	Chloride	250 mg/l	Once every 3 months	Grab
74055	Fecal Coliform	Non-Detect	Once every 3 months	Grab

<sup>(1)</sup> Monitoring required without limitation.

(2) For any reported exceedance, if the average groundwater quality in either background upgradient well (MW1 and MW8) exceeds the groundwater discharge standards, the Department may evaluate whether a violation exists on a case-by-case basis.

<sup>(3)</sup> The groundwater quality limitations are not applicable to the upgradient wells (MW1 and MW8) as shown on Map B.

b. For other parameters not included in (a) above, the discharge of the treated wastewater, which is authorized in this permit, shall not cause an exceedance of the groundwater quality standards adopted by the Department of the Environment in COMAR 26.04.01, and 26.08.02.09.C. For any exceedance, if the average groundwater quality in the background upgradient wells exceeds the groundwater discharge standards, the Department may evaluate whether a violation exists on a case-by-case basis.

### I. SPECIAL CONDITIONS

### B. Land Application Requirements and Limitations

- 1. The Permittee shall apply treated wastewaters via spray irrigation on areas (223.7 acres suitable area, 173.44 acres installed with center pivots and spray guns) shown on the attached Map B.
- 2. The Permittee is prohibited from discharges of any wastewater to surface water except as authorized under a separate surface water discharge permit.
- 3. At no time shall spray irrigation be conducted on areas with bare unvegetated soils except on wheel tracks and during seeding periods. Excessive irrigation resulting in surface run-off beyond the property line or ponding causing vegetation die off is prohibited. Spray irrigation of treated wastewater that results in or is likely to result in surface runoff to surface water is prohibited.
- 4. Irrigation of treated wastewater shall not take place during periods of precipitation, freezing conditions, and saturated soil. Irrigation of treated wastewater that results in aerosols or droplets being carried off site is prohibited. The permittee shall provide a storage facility designed to hold treated wastewater during periods when surface discharge and irrigation cannot take place. The storage facility shall be sealed or constructed to prevent the direct seepage of stored waters into ground waters beneath the site. A minimum of a two-foot freeboard at the storage facility shall be maintained at all times. An easily observable staff gauge for measuring the water level in the lagoon shall be maintained. The permittee shall notify the Department when the water level in the lagoon reaches the two and half-foot freeboard level. Water elevation indicating two and half-foot freeboard shall be marked on the staff gauge.
- 5. The annual average hydraulic loading rates shall be limited to 2"/week in spray fields 1 (23.1 acres), 3 (33.5 acres), 5 (23.7 acres) and 6 (8.1 acres); 0.6"/wk in spray field 4 (30.7 acres); 0.5"/wk in spray field 2 (35.4 acres) and 0.3"/wk in spray fields 7 (25.3 acres), 8 (2.2 acres) and 9 (41.7 acres). The locations of spray fields are shown in Map B. The actual annual average hydraulic loading rate of each spray field must be computed and included in the Annual Spray Irrigation Report required per General Condition II.A.3.
- 6. Irrigation shall be terminated in any spray field with depth to groundwater table of less than two feet from the ground surface within the wetted spray field.
- 7. The Permittee shall annually update and submit to the department by January 15 for approval a nutrient management plan for the spray irrigation system. The plan shall include procedures to minimize nitrogen discharge to the groundwater system. The plan shall be prepared in accordance with COMAR 15.20.08.05 with applicable effluent characteristics. The permittee has ruled out the potential for applying chicken manure waste as fertilizer at this site and therefore such application is prohibited. Any changes made to the NMP must be approved by the Department.

#### I. <u>SPECIAL CONDITIONS</u>

- 8. A reserve spray irrigation area with a design capacity equivalent to 25% of permitted flow should be identified and set aside in case future adjustments in application rates are necessary. The required 25% reserve area is in addition to and separate from the required buffer zone in Condition 9 below.
- 9. The Permittee shall provide adequate means to prevent spray droplets from entering adjacent properties, either by direct application or wind carry-over. These means shall include a buffer zone that is:
  - a. Two hundred feet (200) from the wetted perimeter of the spray irrigation site to property lines in open areas or one hundred feet (100) in areas with tree buffer.
  - b. Five hundred feet (500) from the wetted perimeter of the spray irrigation site to houses or other occupied structures in open areas or two hundred fifty feet (250) in area with tree buffer.

Other alternate means may also be approved by the Maryland Department of the Environment as suitable to control the movement of spray onto adjacent land (i.e., wind break of tightly placed trees; etc.). Upon review and approval by the Department, the buffer zone distance specified in items 9.a and 9.b above may be reduced to meet the buffer distance stipulated in §9-303.1 of the Environmental Article if the effluent quality meets the reclaimed

water quality of  $BOD_5 < 10 \text{ mg/l}$ , total suspended solids < 10 mg/l and fecal coliform < 3 MPN/100 ml.

- 10. Daily logs of the response of each disposal area to the application of treated effluent shall be kept by the plant operator. Subjects to be included in the log are:
  - a. Area(s) or section(s) under irrigation.
  - b. Application rates (hourly and weekly). Each spray field that is in use shall have a flow meter to accurately determine the irrigation rate.
  - c. Instances of ponding or runoff.
  - d. Weather conditions.
  - e. Water level in the lagoon.
  - f. Weekly measurement of groundwater table depth.

The log shall be kept at the spray irrigation site and be available for inspection by the Department personnel upon request.

#### I. <u>SPECIAL CONDITIONS</u>

11. The permittee shall implement a set of standard operating procedures (SOPs) for the supervision of any temporary certified operator to ensure permit requirements are being implemented. The SOPs shall include daily review of the operating logs by the supervising operator. The SOPs shall be kept onsite and be available for inspection by the Department personnel upon request.

### C. Monitoring Requirements of the Land Application System

- 1. The wastewater treatment plant and the spray irrigation system shall be operated by a Maryland State Certified Operator in accordance with the provisions of COMAR 26.06.01 and consistent with the approved operation and maintenance manual. In order to ensure that the Operator is proficient in the operation of the spray irrigation system, the operator shall take required training courses, when available, at a frequency approved by the MD Board of Waterworks and Waste Systems Operator. This training shall be specific to the operation of the wastewater system in addition to any other training requirements of the operator's class.
- The Permittee is responsible for the proper installation, operation and maintenance of ten (10) groundwater monitoring wells to be used for obtaining grab or pumped samples of the groundwater. Locations of the wells are shown on the attached Map B.
- 3. The Permittee shall take and analyze one water sample every three months from each monitoring well.
  - a. Water samples may be obtained by either pumping or bailing the monitoring wells. Prior to taking the sample, a volume of water equal to 300% of the wetted volume of the casing and screen shall be removed.
  - b. The water sample shall be analyzed for the parameters shown in I.A.3.a.

The Permittee shall install and maintain piezometers for monitoring the groundwater levels in spray fields where shallow groundwater tables (<2') are expected.

4. The Permittee shall maintain three (3) surface water monitoring stations along tributaries of the Three Bridge Branch adjacent to the irrigation site for monitoring stream water quality. Sampling frequency and parameters for surface water quality analyses shall be the same as specified in Section I.C.3. The locations of these sampling stations are shown on the attached Map B.



Appendix C

# Influent Sampling Data and 9-Year Effluent Operating Data



Data 1: Operating Effluent Weekly Spreadsheets

										Stream Effl	Jent										Spray Effluent			
			Flow	R	DD	т	SS	TKN	Ammonia		+ NO3	Total Ni	trogen-N	т	P	E. Coli	Flow	BOD	TSS	TKN	NO2 + NO3		TP	E Coli
Year	Month	Week	MGD	mg/L	lbs/day	mg/L	lbs/day	mg/L	Ammonia mg/L	mg/L	lbs/day	mg/L	lbs/day	mg/L	lbs/day	Geomean MPN/100 ml		BOD mg/L	TSS mg/L	TKN mg/L	NO2 + NO3 mg/L	Ammonia mg/L		E. Coli MPN/100 ml
		1 2	0.320 0.351	5.50 2.50	16.15 6.75	4.50 4.50	13.43 12.12	0.79	0.27	0.90	2.73 4.64	1.69 2.52	5.07 6.76	0.09		1.00 1.00					, , , , , , , , , , , , , , , , , , ,	, v	× ·	
	1-Jan	3	0.312	2.00	5.30 4.47	4.00	10.59	1.21	0.54	1.45	3.88	2.65 3.36	6.99 10.47	0.08	0.20	1.00	0.322	2.31	4.05	0.93	2.24	0.23	0.344	7.4
		1	0.355	6.00	21.97	4.00	14.65	1.04	0.33	2.57	9.38	3.61	13.20	0.07	0.25	1.00	0.368	3.88	4.50	1.09	1.99	0.25	0.473	1.8
	1-Feb	2	0.358	4.50 2.69	11.68 7.73	4.00 4.75	10.39 13.92	1.56 0.98	0.42	2.52 2.68	6.53 7.79	4.07 3.65	10.57 10.63	0.10	0.27	1.00 1.76	0.522 0.570	3.75 2.40	4.38 4.30	0.90	2.23 2.18	0.23	1.143 1.049	11.5 6.0
		4	0.360	6.00 8.50	17.01 25.67	4.00 4.00	11.64 11.76	0.54 4.79	1.10 3.51	2.66 1.91	7.86 5.52	3.19 6.69	9.43 20.23	0.09 2.33	0.25	3.26	0.568	2.42 2.44	4.25 4.33	0.68	2.19 2.48	0.27	0.905	5.9 1.8
	1-Mar	2	0.336	5.50 4.50	15.74 13.19	5.00 4.00	14.22 12.11	1.17 0.80	0.37	2.14 2.32	6.10 7.08	3.31 3.12	9.42 9.52	0.17	0.49	3.26 2.00	0.267	2.67 2.15	4.00 4.00	0.80	2.66 2.21	0.21 0.20	0.736	1.9 1.8
		4	0.336	4.50	12.03	4.50	12.03	0.95	0.20	2.74	7.41	3.69	9.97	0.31	0.83	2.05								
	1-Apr	2																						
		4															0.378	4.48	5.40 6.08	1.20	1.48 1.40	0.27	0.342	1.8 5.0
	1-May	2															0.289	4.13 4.51	3.74	1.02	1.14	0.26	1.861	6.7
		3 4															0.619 0.481	4.37 3.27	0.94 2.56	0.42	1.60 1.66	0.12	1.572 1.328	1.8 1.9
	1-Jun	1 2															0.245 0.298	6.26 1.47	1.30 2.75	1.04 0.50	1.61 2.08	0.14 0.13	1.689 0.973	4.3 1.8
2014	1-5011	3 4															0.173	1.89	1.44	0.70	2.00	0.12	0.531	1.8
2014		1																						
	1-Jul	3															0.0717	3.46	3.22	0.39	1.67	0.16	0.596	3.6
		1															0.306	3.36	1.44	0.37	1.64	0.16	1.582	1.8
	1-Aug	2															0.395	3.83 3.04	1.22 0.56	0.34	1.26 1.52	0.32	2.244 3.143	2.0 4.5
		4															0.597 0.506	3.97 5.22	1.10 1.88	0.97	1.56 1.67	0.15	3.151 2.574	6.4 1.9
	1-Sep	2															0.323 0.274	4.23 1.71	2.56 1.30	1.03 1.05	1.70 1.59	0.15 0.15	1.815 1.684	1.8 1.8
		4	-														0.170	2.41	0.88	0.98	1.64	0.18	1.408	1.8
	1-Oct	2	-														-							
		4	-														0.060	2.59	1.81	1.01	1.32 1.07	0.29	0.899	1.8 4.5
	1-Nov	2															0.564	3.23	0.75	0.59	1.70	0.11	2.056	11.0
		3 4		<u> </u>													0.245	3.34	1.19	0.53	1.17	0.12	2.538	14.6
	1-Dec	1 2	0.315	4.19	3.46 10.94	0.50 2.50	1.35 6.53	1.00 0.88	0.10 0.15	2.05 1.99	5.54 5.24	3.05 2.87	8.24 7.56	0.18 0.14	0.50	1.00 1.00	L			L-				
	I-Dec	3 4	0.303 0.319	2.31 9.00	5.69 32.58	1.00 4.00	2.48 14.48	0.90 0.75	0.13 0.20	1.89 2.43	4.64 8.80	2.79 3.18	6.86 11.52	0.13 0.10	0.31 0.36	1.00								
		1 2	0.315	2.06	5.09	0.50	1.23	1.00	0.12 0.10	1.90	4.65	2.90	7.10	0.10	0.27	1.80								
	1-Jan	3	0.373	6.32	16.72 6.58	2.25	5.93	0.80	0.15	1.95	5.21	2.75	7.36	0.09	0.25	1.41								
		4	0.344	3.08	9.30	0.50	1.52	0.72	0.11	1.58	4.82	2.29	7.03	0.23	0.71	1.00								
	1-Feb	3	0.321	3.20 2.20	8.72 5.56	1.25 0.50	3.49 1.27	0.63 0.45	0.12	1.19 1.90	3.26 4.83	1.82 2.35	4.99 5.96	0.14 0.16	0.39	1.76 7.10								
		4	0.322 0.412	2.10 3.67	5.74 12.00	6.50 0.75	17.72 2.50	0.60	0.14 0.16	1.24 1.84	3.38 6.04	1.84 2.44	5.00 8.02	0.20	0.54 0.49	1.00 8.92								
	1-Mar	2	0.415 0.411	5.81 3.08	19.61 11.37	0.75	2.54 3.77	0.79	0.21 0.10	2.06	6.95 7.23	2.85	9.60 11.40	0.31 0.31	1.04	6.82 38.78								
		4	0.366	4.64	14.22	1.00	3.10	0.67	0.22	2.08	6.50	2.75	8.59	0.26	0.81	7.96								
	1-Apr	2																						
		4																						
	1-May	2																						
		3 4																						
	1-Jun	1 2																						
2015	1 3411	3 4																						
2015		1 2																						
	1-Jul	3																						
		1	-		<b></b>																			
	1-Aug	2 3																						
		4																						
	1-Sep	2		<u> </u>																				
		4																						
	1-Oct	2	$\pm$	L	E-									L-			L			L-				
		4																						
	1-Nov	2			-																			
		4	0.431	1.59	5.62	0.50	1.82	0.40	0.12	2.30	8.36	2.69	9.80	0.14	0.49	4.02	-							
	1-Dec	2	0.397	3.20	10.49	0.75	2.44	0.40	0.10	1.97	6.67	2.37	8.01	0.15	0.49	32.23								
		3	0.413	2.22	8.58 10.18	0.50	1.74	0.40	0.10	2.60 2.31	9.03 9.85	2.99	10.41	0.19	0.66	13.10 12.46								
	1-Jan	1 2	0.406	3.42	7.77 8.28	1.75 2.25	6.11 7.52	0.44 0.40	0.10	2.88 3.61	10.01 9.07	3.31 4.00	11.53 10.17	0.14 0.47	0.49	11.65 58.30								
	. 3011	3 4	0.439	2.00 2.81	6.60 8.83	0.50	1.62 3.31	0.48	0.17	2.56 2.85	8.12 9.44	3.05 3.21	9.66 10.61	0.22 0.17	0.70	144.50 50.02								]
	15.1	1 2	0.481 0.469	4.29 1.31	16.80 5.26	0.75	2.94 2.01	0.35	0.31 0.16	1.68 1.99	6.59 8.00	2.03 2.84	7.95 11.40	0.26	1.00 0.52	8.12 1.00								
	1-Feb	3 4	0.473	1.74	7.30	2.50	10.35	0.33	0.24 0.12	2.46	10.18	2.79	11.58	0.13	0.54	1.00								
		4 1 2	0.470	1.00	3.77 8.10	2.00	7.54	0.38	0.12 0.23 0.19	1.97	7.43	2.45	8.85	0.13	0.49	4.88								
	1-Mar	3	0.449	1.10	4.06 10.44	0.50	0.10 1.86 4.82	0.37	0.13	2.26	8.30	2.63	9.71	0.19	0.70	4.38	<u> </u>							
		4	0.414	2.79	10.44	1.25	4.82	0.39	0.11	2.23	8.40	2.61	9.85	0.33	1.21	1.00								
	1-Apr	2																						
		4																			├			
	1-May	2																						
		4																						
	1-Jun	2	-														-							
2016		4																						
	1-Jul	2																						
		3		I	I	I	I										<u> </u>							

										Stream Eff	luent										Spray Effluer	ıt		
			Flow		DD		SS	TKN	Ammonia		+ NO3		itrogen-N		P	E. Coli Geomean	Flow	BOD	TSS	TKN	NO2 + NO3	Ammonia	TP	E. Coli
Year	Month	Week 4 1	MGD	mg/L	lbs/day	mg/L	lbs/day	mg/L	mg/L	mg/L	lbs/day	mg/L	lbs/day	mg/L	lbs/day	MPN/100 ml	MGD	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	MPN/100 ml
	1-Aug	2 3																						
		4 1 2																						
_	1-Sep	3 4 1																						[
	1-Oct	2																						
-		4 1 2																						
	1-Nov	3																						
	1-Dec	1 2 3																						
		4	0.419	2.37	9.01	0.50	1.86	0.68	0.11	1.01	3.71	1.69	6.17	0.18	0.64	1.00								
	1-Jan	2 3 4	0.385 0.372 0.405	4.64 3.76 1.73	14.33 12.06 6.07	4.50 0.50 0.50	13.88 1.57 1.75	1.23 1.59 0.59	0.18 0.43 0.13	1.53 1.13 1.33	5.06 3.62 4.64	2.75 2.72 1.92	8.97 8.63 6.71	0.25 0.38 0.19	0.82 1.20 0.65	1.00 1.00 1.00								
	1-Feb	1 2 3	0.364 0.370 0.349	3.15 2.85 1.56	9.18 8.91 4.72	1.00 1.75 0.75	2.92 5.65 2.27	1.05 0.48 0.49	0.13 0.12 0.11	1.63 1.81 1.86	4.78 5.63 5.65	2.68 2.29 2.35	7.90 7.11 7.17	0.43 0.39 0.49	1.25 1.20 1.48	1.00 1.00 1.00								
-		4	0.376	2.34	7.29	0.75	2.34	0.69	0.10	1.32	4.10	2.01	6.25 5.74	0.49	2.09	1.00								
	1-Mar	2 3 4	0.404 0.392 0.382	2.05 2.71 3.27	5.46 8.83 11.12	1.75 2.50 1.25	4.57 8.52 4.06	0.96 0.78 0.99	0.16 0.19 0.21	1.46 1.30 1.15	4.19 4.29 3.92	2.42 2.08 2.14	6.90 6.89 7.28	0.85 0.44 0.38	2.47 1.44 1.26	1.00 1.00 1.00								
F	1-Apr	1 2	3.302	3.61	12	1.20	1.00	3.77	0.21	1.10	J.7Z	2.14	1.20	0.00	1.20	1.00								
F		3 4 1	<u> </u>		$\vdash$	$\vdash$								$\vdash$										
	1-May	2	-																					
		4 1 2																						
2017	1-Jun	3 4 1																						
	1-Jul	2																						
-		4 1 2																						
	1-Aug	3 4																						
	1-Sep	1 2 3																						
-	1-Sep 1-Oct	4																						
		2 3 4																						
	1-Nov	1 2 3																						
-		4	0.375	1.77	5.04	2.50	7.16	0.78	0.14	1.83	5.31	2.61	7.59	0.60	1.72	1.00								
	1-Dec	2 3 4	0.371 0.365 0.371	1.80 3.70	5.95 10.83	3.00	9.59 5.85	0.93	0.55 0.21	1.80 1.27	5.86 3.71	2.72 2.70	8.88 7.90	0.52	1.67 0.85	4.45 27.10								
	1-Jan	1 2	0.362 0.427	5.66 6.190	16.510 21.249	2.50 2.250	7.385 8.134	0.86	0.205	1.61 0.936	4.804 3.155	2.47 2.491	7.370 8.506	0.64	1.977 0.614	44.75 55.050								
-		3 4 1	0.366 0.377 0.404	1.820 3.513 4.305	5.536 10.579 13.586	2.500 6.667 3.000	7.631 20.477 9.533	1.410 1.373 1.455	0.496 0.235 0.295	0.747 1.861 2.119	2.299 5.739 6.790	2.157 3.234 3.574	6.615 9.928 11.446	0.220 0.170 0.165	0.671 0.517 0.527	28.400 10.767 1.000								
	1-Feb	2 3 4	0.545 0.474 0.440	3.915 3.080 3.255	15.783 12.656 12.254	5.000 4.000	19.124 16.434 10.146	2.610 1.110	1.477 0.389 0.327	1.927 0.841 1.580	7.221 3.455	4.537 3.070	17.445 12.611 8.641	0.625 0.255 0.450	2.523 1.047 1.659	10.100 3.100 1.000								
-	1-Mar	4 1 2	0.416	3.265 3.555	12.008 13.851	1.250 3.000	4.285 11.972	0.710	0.343 0.306	1.816	5.816 6.733 7.355	2.350 2.526 2.351	9.403 9.501	0.785	2.863 2.658	7.100								
-	1-Ividi	3 4 1	0.451		11.642 10.538		6.595 3.891	1.065 1.310	0.270	1.748 1.904	6.252 7.550	2.813 3.214	9.987 12.930	0.440	1.569 4.178	1.000 55.733	0.000	4.66	0.75	1.52	1.66	0.29	0.86	1.90
	1-Apr	2									-			-			0.000	1.990 7.230	0.500	1.415 3.055	1.470 0.982	0.436	1.450 1.260	2.000 3.250
ŀ		4 1 2	-		<u> </u>	<u> </u>								<u> </u>			0.700	4.650 3.505 1.000	0.750	1.100 1.420 1.460	1.462 1.745 1.329	0.343 0.28 0.183	0.760 1.54 2.615	1.800 1.90 4.500
ļ	1-May	3															0.000	4.145 3.243 1.989	1.000	0.845 0.848 1.267	1.633 1.723 1.730	0.100 0.132 0.119	1.515 1.448 1.165	2.600 4.300 1.000
	1-Jun	1 2 3			E	E								E			0.390	1.465 2.545	1.500 0.500	0.730	2.133 1.720	0.120 0.100	1.785 0.635	1.500 23.000
2018		4 1 2	1		$\vdash$	$\vdash$								<u> </u>			0.235 0.407 0.655	1.000 4.360 3.960		1.135 1.955 1.265	1.930 1.211 2.168	0.100 0.100 0.105	1.465 1.380 1.575	4.850 6.500 1.500
	1-Jul	3									-			-			0.624	1.460 2.480	1.250 1.167	1.280 0.720	1.466 1.442	0.103 0.102	2.280 2.877	16.500 4.633
	1-Aug	1 2 3	-		<u> </u>	<u> </u>								<u> </u>			0.483	1.610 1.480 2.445	0.100	0.725 1.290 0.750	1.295 1.142 1.006	0.100 0.108 0.108	1.585 2.050 2.050	4.850 6.700 1.000
ŀ		4															0.829	3.030 0.100	2.500 0.500	1.303 0.930	1.007 1.193	0.100 0.104	2.243 3.030	1.000 1.500
	1-Sep	2 3 4	-			-							<u> </u>					4.850 4.360 2.520		0.730 0.720 0.720	1.584 1.310 1.788	0.140 0.108 0.100	2.650 2.340 2.200	3.100 4.200 1.500
Ī	1-Oct	1 2 3															0.631 0.141	1.425 3.625	0.500	1.235 0.760	1.573 1.575	0.106	2.050 2.450	2.050 1.000
-		4															0.000	2.278 1.525	0.750	0.925 0.948 1.120	1.615 1.768 1.759	0.107 0.117 0.110	2.015 1.570 0.550	3.650 2.325 2.000
	1-Nov	2 3 4															0.498	1.375 2.900 2.355		1.150 0.895 0.720	1.980 1.973 1.956	3.130 2.868 2.676	0.125 0.100 0.114	1.050 0.220 0.520
ŀ	1-Dec	1 2	0.558	1.470	10.146	0.500	14.045	0.860	0.199	2.071	9.636 13.003	2.931 3.681	13.648 16.558	0.645	2.981	1.000	3.210							
		3 4 1	0.758 0.730 0.735	2.160 4.383 2.400	12.609 #DIV/0! #DIV/0!	1.750 2.833 0.500	10.087 #DIV/0! #DIV/0!	0.720 0.837 0.720	0.166 0.205 0.259	1.878 1.627 1.722	11.413 #DIV/0! #DIV/0!	2.598 2.464 2.442	15.776 #DIV/0! #DIV/0!	0.260 0.233 0.205	1.584 #DIV/0! #DIV/0!	4.250 1.000 6.050	-	-						
	1-Jan	2	0.644	1.745 1.740	#DIV/0! #DIV/0!	0.500 3.250	#DIV/0! #DIV/0!	0.720 0.720	0.385	2.018 1.950	#DIV/0! #DIV/0!	2.738 2.670	#DIV/0! #DIV/0!	0.465	#DIV/0! #DIV/0!	1.000 1.600								
		4	0.640	1.477 1.955	#DIV/0! #DIV/0!	0.500	#DIV/0! #DIV/0!	0.720	0.287	2.352 2.719	#DIV/0! #DIV/0!	3.072 3.439	#DIV/0! #DIV/0!	0.450 0.610	#DIV/0! #DIV/0!	2.067							<u> </u>	

											Stream Effl	uent										Spray Effluen	t		
				Flow	BC	DD	T	SS	TKN	Ammonia		+ NO3	Total Ni	trogen-N	Т	Р	E. Coli	Flow	BOD	TSS	TKN	NO2 + NO3	Ammonia	TP	E. Coli
Year	Month	Week 3		MGD	mg/L 1,880	lbs/day #DIV/0!	mg/L 0.500	lbs/day #DIV/0!	mg/L 0.790	mg/L 0.129	mg/L 2.375	lbs/day #DIV/0!	mg/L 3.165	lbs/day #DIV/0!	mg/L 0.560	lbs/day #DIV/0!	Geomean MPN/100 ml 1.000	MGD	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	MPN/100 ml
		4	(	0.645		#DIV/0! #DIV/0!	1.333	#DIV/0! #DIV/0!	0.720	0.232	2.901	#DIV/0! #DIV/0!	3.621 3.062	#DIV/0! #DIV/0!	0.757	#DIV/0! #DIV/0!	1.000								
	1-Mar	2	(	0.667	2.675	#DIV/0! #DIV/0!	0.500	#DIV/0! #DIV/0!	0.820	0.173	1.591	#DIV/0! #DIV/0!	2.411	#DIV/0! #DIV/0!	0.640	#DIV/0! #DIV/0!	1.500								
		4		0.624		#DIV/0!	1.000	#DIV/0!	0.855	0.394	1.549	#DIV/0!	2.404	#DIV/0!	0.240	#DIV/0!	1.000	0.525	1.0/0	0.750	0.700	1 207	2.007	0.001	0.225
	1-Apr	2																0.525	1.620	0.500	0.720	1.287 1.407 1.890	2.007 2.157 2.690	0.251	0.225
		3																0.390	1.677	0.750	0.800	1.972	3.222	0.260	0.680
	1-May	2																0.111 0.053	1.245	0.500	0.910	2.106 1.814	3.016 2.534	0.285	1.865 1.830
	,	3																0.502	1.090	0.500	0.720	2.392 1.745	3.112 2.732	0.232 0.194	1.965 1.387
	1-Jun	2																0.038	1.135	0.500	0.930	2.488 1.996	3.418 3.896	0.297 0.191	1.615 1.075
2019	1 341	3	_															0.614 0.523	1.155	0.500	0.835	1.271 1.403	2.106	0.121 0.113	1.465 2.035
2019	1.64	1																0.330	1.000	0.500	0.850	1.156 1.186	2.006	0.100	2.215 2.480
	1-Jul	3																0.663	1.010	0.750	0.790	1.227 1.144	2.017 1.864	0.391 0.130	2.900 2.563
		1																0.681	1.295	0.500	0.720	1.289	2.009 3.425	0.120	1.950
	1-Aug	3																0.641	1.515	2.250	1.000	1.500	2.500	0.100	1.295
		1																0.411 0.412 0.471	1.030	0.500	1.000	1.558	2.558	0.100	1.860
	1-Sep	2																0.537	1.085	0.500	1.000	1.500	2.500	0.111	1.965
		4																0.304 0.221	1.230	0.750	1.000	1.659 1.437	2.659 2.536	0.121 0.195	1.905 1.537
	1-Oct	2																0.090	1.000	0.500	1.000 1.000	1.799 1.455	2.799 2.498	0.111 0.157	2.250 1.750
		4				L_												0.032 0.265	1.103 1.355	0.625	1.000 1.000	1.868 1.317	2.868 2.317	0.337	1.950 1.620
	1-Nov	2				-												0.111 0.509	1.280 1.085	0.500	1.000 1.000	1.411 1.447	2.411 2.447	0.142 0.186	1.535 1.280
		4		0.442	1.840	#DIV/0!	0.750	#DIV/0!	0.500	0.145	0.873	#DIV/0!	1.373	#DIV/0!	1.535	#DIV/0!	1.500	0.194		0.750	0.790	0.922	1.712	0.295	0.970
	1-Dec	2	(	0.455		#DIV/0! #DIV/0!	0.750	#DIV/0! #DIV/0!	0.573	0.285	1.524	#DIV/0! #DIV/0!	2.096	#DIV/0! #DIV/0!	1.985	#DIV/0! #DIV/0!	102.350								
		4	(	0.412	1.177	#DIV/0! #DIV/0!	0.833	#DIV/0! #DIV/0!	0.500	0.161	0.816	#DIV/0! #DIV/0!	1.347	#DIV/0! #DIV/0!	0.377	#DIV/0! #DIV/0!	72.000								
	1-Jan	2	(	0.450	1.295	#DIV/0!	0.500	#DIV/0!	0.500	0.269	1.164	#DIV/0!	1.664	#DIV/0!	0.465	#DIV/0!	101.250								
		3	(	0.413 0.441	1.000	#DIV/0! #DIV/0!	0.750	#DIV/0! #DIV/0!	0.568	0.221	1.029	#DIV/0! #DIV/0!	1.596 1.883	#DIV/0! #DIV/0!	0.950	#DIV/0! #DIV/0!	4.200								
	1-Feb	2	(	0.457 0.474		#DIV/0! #DIV/0!	0.500	#DIV/0! #DIV/0!	0.500 0.570	0.211 0.557	1.254 1.303	#DIV/0! #DIV/0!	1.754 1.873	#DIV/0! #DIV/0!	0.965	#DIV/0! #DIV/0!	12.100 101.250								
	1460	3		0.466	1.945 3.265	#DIV/0! #DIV/0!	0.500	#DIV/0! #DIV/0!	0.675 0.730	0.577 0.379	1.279	#DIV/0! #DIV/0!	1.954 1.858	#DIV/0! #DIV/0!	0.865	#DIV/0! #DIV/0!	200.500 200.500								
	1-Mar	1		0.448 0.445	2.010	#DIV/0! #DIV/0!	0.500	#DIV/0! #DIV/0!	0.690	0.538	1.094	#DIV/0! #DIV/0!	1.784 1.549	#DIV/0! #DIV/0!	0.830	#DIV/0! #DIV/0!	200.500								
	I-IVIAI	3		0.444 0.470	1.980	#DIV/0! #DIV/0!	1.750	#DIV/0! #DIV/0!	0.903	0.501 0.555	0.932	#DIV/0! #DIV/0!	1.835 1.798	#DIV/0! #DIV/0!	0.950	#DIV/0! #DIV/0!	1.000 200.500								
		1																0.376	1.640	1.750	0.848	5.136 1.562	5.983 1.779	0.443 0.309	1.635 1.450
	1-Apr	3																0.056		0.750	0.535	1.445	1.980	0.291 0.277	2.160
		4 1 2																0.090	1.640	1.500	0.500	1.545	2.045	0.218	1.915
	1-May	3																0.514	1.685 1.400	1.000	0.588	0.478	1.707	3.420 0.215	1.830 0.885
		4																0.082 0.419		0.500	0.500	1.238 1.210	1.738 1.710	0.260 0.248	2.015 1.770
	1-Jun	2																0.456		0.500	4.770 2.080	1.093 0.863	5.863 2.943	0.310 0.229	2.535 1.765
2020		4																0.676	1.400	0.667	0.958	0.991 1.024	1.950 1.524	0.166 0.126	2.620 2.500
	1-Jul	2																0.391	1.090 1.270	1.500	0.500	1.266 1.356	1.766	0.175 0.333	2.380 2.400
		4																0.330	1.323	0.667	0.798	1.496 3.542	2.033	0.471 0.391	1.870
	1-Aug	2																0.477		0.750	1.018	1.317	2.335	1.303	2.515
		4																0.609	1.665	0.500	0.823	1.793	2.615 3.004	0.169	2.420
	1-Sep	2																0.311	1.970	0.500	0.905	1.904	2.809	0.149	1.685
	-	3																0.579		1.125	0.500	1.502	2.002	0.189	1.685
	1-Oct	1																0.188		0.500	0.500	1.167	1.667	0.254 0.276	1.180 0.865
		3																	2.460 2.015	0.500	0.695	1.436	2.131 1.679	0.238	1.485 1.635
	1-Nov	1 2																0.726 0.349	1.345		0.500	1.273 1.233	1.773 1.733	0.462 0.377	1.935 1.480
	7110V	3 4			_													0.805	3.310 2.255		0.500 2.853	1.264 1.435	1.765 3.788	0.362 0.519	1.585 1.135
	1-Dec	1	(	0.701 0.658		#DIV/0! #DIV/0!	1.250 0.500	#DIV/0! #DIV/0!	2.293 2.855	0.427 0.521	1.190 1.283	#DIV/0! #DIV/0!	3.482 4.138	#DIV/0! #DIV/0!	0.785 0.815	#DIV/0! #DIV/0!	20.150 76.100								
	1-Dec	3		0.753 0.731	1.800	#DIV/0! #DIV/0!	0.500	#DIV/0! #DIV/0!	2.770 4.867	0.441 0.300	1.703 1.668	#DIV/0! #DIV/0!	4.473 6.534	#DIV/0! #DIV/0!	1.335 0.857	#DIV/0! #DIV/0!	164.600 381.300								
		1 2		0.778 0.717	1.515 1.375	#DIV/0! #DIV/0!	1.500 3.000	#DIV/0! #DIV/0!	<1.0 0.510	0.275 0.397	1.762 1.957	#DIV/0! #DIV/0!	2.262 2.712	#DIV/0! #DIV/0!	1.165 1.000	#DIV/0! #DIV/0!	1.000 <1.0								
	1-Jan	3	(	0.699 0.634	1.815	#DIV/0! #DIV/0!	3.500	#DIV/0! #DIV/0!	<1.0 1.030	0.430	1.819	#DIV/0! #DIV/0!	2.819 3.149	#DIV/0! #DIV/0!	0.900	#DIV/0! #DIV/0!	1.000								
		1 2	(	0.644 0.634	1.370	#DIV/0! #DIV/0!	<0.5	#DIV/0! #DIV/0!	1.020	0.192 0.187 0.225	2.005	#DIV/0! #DIV/0!	3.025	#DIV/0! #DIV/0!	0.900	#DIV/0! #DIV/0!	<1.0 <1.0 1082.150								
	1-Feb	3	(	0.676	1.605	#DIV/0! #DIV/0! #DIV/0!	2.000	#DIV/0!	1.050	0.156	3.268	#DIV/0!	4.293	#DIV/0!	0.735		1213.500								
		1	(	0.743	<1.0	#DIV/0!	20.500	#DIV/0! #DIV/0!	<1.0	0.296	2.475	#DIV/0! #DIV/0!	3.475 3.148	#DIV/0! #DIV/0!	0.360	#DIV/0!	<1.0								
	1-Mar	2	(	0.693 0.651		#DIV/0! #DIV/0!	<0.5	#DIV/0! #DIV/0!	2.155 <1.0	1.438	1.715	#DIV/0! #DIV/0!	3.870	#DIV/0! #DIV/0!	0.400	#DIV/0! #DIV/0!	<1.0 1210.300								
		4	(	0.651	1.580	#DIV/0!	1.500	#DIV/0!	1.910	0.273	1.886	#DIV/0!	3.190	#DIV/0!	0.157	#DIV/0!	7.750	0.628	2.607	<0.5	1.350	1.593	0.265	0.340	4.650
	1-Apr	2																0.569		1.000 <0.5	1.300 0.970	2.194 2.125	0.780	0.295 0.170	3.100 11.550
		4																0.540		2.500 1.500	2.420 1.950	2.065 2.015	1.010 0.523	0.860 2.550	2419.600 <1.0
	1-May	2			_	L												0.477 0.468	2.350 3.230	<0.5 1.500	1.620 2.775	2.460 2.615	0.655 0.457	2.100 2.100	<1.0 1.000
		4	-															0.468 0.457	2.970 2.900	<0.5 <0.5	2.405 1.445	3.085 2.835	0.572 0.217	2.900 1.900	<1.0 1.000
	1-Jun	2																0.453	2.920	<0.5 <0.5	1.145	2.545	0.156	2.170	<1.0
		4																0.438	3.175	3.000	1.338	2.853	0.224	2.643	<1.0
	1-Jul	2																0.443	2.380	<0.5 <0.5 <0.5	1.210	2.215 3.085	0.262	1.080	<1.0
		3 4																0.398	2.930	<0.5	<1.0	3.495	0.215	2.120	1.000
	1-Aug	2																0.408		<0.5	1.255	1.390 0.855	0.374	0.700	<1.0 <1.0
		3																	5.350 2.583		9.900 1.923	1.845 2.093	8.790 0.154	0.405	143.700 3.767
		1		_			I	I							I			0.412	1.660	<0.5	1.320	2.000	0.165	1.450	3.100

							Flow BOD TSS TKN Ammonia NO2 + NO3 Total Nitrogen-N TP Geor														Spray Effluen	ıt		
			Flow			T	SS	TKN	Ammonia	NO2	+ NO3	Total Ni	trogen-N	T	P	E. Coli Geomean	Flow	BOD	TSS	TKN	NO2 + NO3	Ammonia	TP	E. Coli
Year	Month	Week	MGD	mg/L	lbs/day	mg/L	lbs/day	mg/L	mg/L	mg/L	lbs/day	mg/L	lbs/day	mg/L	lbs/day	MPN/100 ml	MGD	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	MPN/100 ml
	1-Sep	2				_				-		-				_	0.421	1.850	1.000	1.120	2.265	0.160	1.365	3.100
	1 bep	3															0.412	1.585	<0.5	<1.0	2.145	0.170	1.410	<1.0
		4															0.441	1.533	< 0.5	<1.0	1.997	0.166	1.737	1.000
		2															0.400	1.460	<0.5	<1.0 1.070	1.840 1.435	0.218	2.100	<1.0 >2419.6
	1-Oct	3															0.397	1.280	< 0.5	1.450	1.435	0.219	1.485	<1.0
		4															0.431	1.320	1.000	1.370	1.395	0.348	2.185	1210.300
		1															0.413	1.455	< 0.5	2.070	0.950	0.381	1.765	<1.0
	1-Nov	2															0.406	1.300	<0.5	2.270	1.560	0.366	2.600	<1.0
	1-1404	3															0.393	1.300	<0.5	2.985	0.815	0.416	1.465	5.200
		4	0.472	1.435	#DIV/0!	1.000	#DIV/0!	2.155	0.808	0.785	#DIV/0!	2.940	#DIV/0!	1.000	#DIV/0!	<1.0	0.428	1.365	1.000	1.840	0.760	0.474	0.977	<1.0
		2	0.472	1.435	#DIV/0! #DIV/0!	<0.5	#DIV/0! #DIV/0!	2.155	0.808	0.785	#DIV/0! #DIV/0!	2.940	#DIV/0! #DIV/0!	0.935	#DIV/0! #DIV/0!	<1.0								
	1-Dec	3	0.443		#DIV/0!	<0.5	#DIV/0!	1.470	0.792	0.965	#DIV/0!	2.400	#DIV/0!	0.700	#DIV/0!	<1.0								
		4	0.439	2.340	#DIV/0!	1.000	#DIV/0!	2.527	1.078	0.817	#DIV/0!	3.343	#DIV/0!	0.767	#DIV/0!	1.000								
		1	0.468	1.555	#DIV/0!	< 0.5	#DIV/0!	1.825	0.551	1.470	#DIV/0!	3.295	#DIV/0!	0.510	#DIV/0!	<1.0								
	1-Jan	2	0.458	1.29	#DIV/0!	< 0.5	#DIV/0!	3.42	0.5485	1.05	#DIV/0!	4.47	#DIV/0!	0.6	#DIV/0!	<1.0								
	1-3011	3	0.484	1.595	#DIV/0!	< 0.5	#DIV/0!	1.975	0.6595	0.92	#DIV/0!	2.895	#DIV/0!	0.7	#DIV/0!	<1.0								-
<u>г Т</u>		4	0.425		#DIV/0!	1	#DIV/0!	1.81	0.854	1.007	#DIV/0!	2.817	#DIV/0!	0.78	#DIV/0!	<1.0	L					L	]	
$\vdash$		2	0.479		#DIV/0!	1.000	#DIV/0!	1.450	0.630	0.845	#DIV/0! #DIV/0!	2.295	#DIV/0!	0.795	#DIV/0!	5.400 <1.0								
┝───┤	1-Feb	2	0.434 0.430	1.850 1.480	#DIV/0! #DIV/0!	< 0.5	#DIV/0! #DIV/0!	1.430	0.604 0.726	0.445	#DIV/0! #DIV/0!	1.875 2.170	#DIV/0! #DIV/0!	0.615	#DIV/0! #DIV/0!	<1.0								
┝──┤		4	0.430	4.093	#DIV/0!	2.333	#DIV/0!	1.460	1.043	0.633	#DIV/0!	2.170	#DIV/0!	1.043	#DIV/0!	<1.0								
		4	0.447	5.380	#DIV/0!	2.000	#DIV/0!	1.830	1.043	1.255	#DIV/0!	3.085	#DIV/0!	0.850	#DIV/0!	<1.0	1							
		2	0.449	3.690	#DIV/0!	2.000	#DIV/0!	1.620	0.870	1.380	#DIV/0!	3.000	#DIV/0!	1.100	#DIV/0!	1.000								
	1-Mar	3	0.455	3.635	#DIV/0!	< 0.5	#DIV/0!	1.490	0.730	1.295	#DIV/0!	2.785	#DIV/0!	1.050	#DIV/0!	2.000								-
		4	0.421	3.193	#DIV/0!	4.500	#DIV/0!	1.460	0.653	1.270	#DIV/0!	2.730	#DIV/0!	0.920	#DIV/0!	1210.300								
		1															0.446	2.730	3.000	1.445	1.285	0.880	0.930	1.000
	1-Apr	2															0.421	1.555	2.000	1.020	1.450	0.485	1.350	1210.300
		3															0.419	1.330	<0.5	<1.0	0.930 0.510	0.301 0.145	0.950	<1.0 2.000
		4															0.399	1.945	<0.5	1.225	0.915	0.145	3.000	307.600
		2															0.402	3.090	<0.5	1.225	0.475	0.527	2.170	<1.0
	1-May	3															0.412	3.570	< 0.5	1.450	0.130	0.693	2.200	346.000
		4															0.430	2.587	3.000	1.920	0.443	0.687	2.490	4.100
		1															0.391	2.580	4.000	1.975	0.520	0.556	2.515	1.000
	1-Jun	2															0.398	2.755	< 0.5	1.415	0.565	0.703	2.630	1.000
		3															0.385	3.180 2.207	<0.5 1.000	1.130	0.785	0.423 0.209	2.730 2.620	1210.300 1.000
		4															0.391	1.380	< 0.5	1.203	1.130	0.209	3.330	1.000
		2															0.417	1.410	5.000	1.160	1.390	0.287	3.040	<1.0
	1-Jul	3															0.420	1.925	2.000	1.040	1.555	0.336	2.815	<1.0
		4															0.392	1.875	1.000	<1.0	1.480	0.264	2.665	<1.0
		1															0.391	1.445	<0.5	<1.0	2.515	0.218	3.015	1222.750
$\vdash$	1-Aug	2															0.384	1.480	< 0.5	1.140	1.670	0.156	4.150	<1.0
┝───┤	-	3 4															0.351	1.445	<0.5	1.220	6.375 3.383	0.124 0.164	3.620	<1.0 1210.300
<b>├</b> ──┤		1	-														0.353	2.065	4.500	<1.0	4.555	<0.1	1.835	<1.0
	1.0	2															0.369	2.005	2.000	<1.0	4.610	<0.1	2.850	<1.0
	1-Sep	3															0.352	2.910	1.000	1.670	2.435	<0.1	3.000	3.000
		4				_	_			-		-	-			_	0.352	1.745	1.000	<1.0	2.265	0.122	2.250	353.800
$\vdash$		1															0.413	1.615	1.000	<1.0	2.450	0.127	0.783	42.800
$\vdash$	1-Oct	2															0.381 0.384	1.945	1.500	1.030	2.135	<0.1	2.100 2.750	<1.0
$\vdash$		3															0.384	1.550	< 0.5	<1.0 <1.0	2.450	<0.1 <0.1	2.150	1.000
<b>├</b> ──┤		4	-														0.404	1.000	<0.5	<1.0	2.400	<u.1< td=""><td>2.470</td><td>&lt;1.0</td></u.1<>	2.470	<1.0
	1-Nov	2															0.371	1.510	< 0.5	<1.0	1.580	<0.1	0.333	<1.0
	I-NOV	3															0.390	1.490	2.000	<1.0	1.420	<0.1	0.833	<1.0
		4				_	_			-		-	-			_	0.378	1.640	2.000	<1.0	1.565	0.106	1.235	<1.0
$\vdash$		1	0.476	1.130	#DIV/0!	1.000	#DIV/0!	1.030	0.149	2.435	#DIV/0!	2.950	#DIV/0!	2.420	#DIV/0!	<1.0								
$\vdash$	1-Dec	2	0.440	1.505	#DIV/0! #DIV/0!	< 0.5	#DIV/0! #DIV/0!	1.040	0.173	2.030 2.125	#DIV/0! #DIV/0!	2.550 2.125	#DIV/0! #DIV/0!	2.785	#DIV/0! #DIV/0!	<1.0						<u> </u>		
$\vdash$		3	0.539		#DIV/0! #DIV/0!	< 0.5	#DIV/0! #DIV/0!	<1.0	0.223	2.125	#DIV/0! #DIV/0!	3.247	#DIV/0! #DIV/0!	2.163	#DIV/0! #DIV/0!	1613.400								
			0.000	1.173		1.000	. DIV/0!	1.130	0.170	2.005	1011/0	3.247	2010/0	2.103	* DI ¥7 U!	1013.400								
															1					1				
		2014-2022																						
		Max	0.799	9.000	#DIV/0!	20.500	#DIV/0!	4.867	3.510	3.608	#DIV/0!	6.690	#DIV/0!	2.785	#DIV/0!	1613.400	1.094	7.230	6.075	9.900	6.375	8.790	4.150	2419.600
$\vdash$		Average	0.469	2.635	#DIV/0!	1.913	#DIV/0!	1.061	0.361	1.730	#DIV/0!	2.772	#DIV/0!	0.558	#DIV/0!	77.370	0.383	2.238	1.448	1.171	1.702	1.167	1.243	63.835
		Min	0.303	1.000	#DIV/0!	0.500	#DIV/0!	0.333	0.100	0.445	#DIV/0!	1.316	#DIV/0!	0.069	#DIV/0!	1.000	0.000	0.100	U. 100	0.336	0.130	0.100	0.100	0.220



Data 2: Operating Effluent Annual Spreadsheet

	r					Stream Effluent																		
								Stream Efflu	ient										Spray	Effluent				Total Flow
Calendar	Flow	В	DD	T	SS	TKN	Ammonia	NO2	+ NO3	Total Ni	trogen-N	1	ΓP	Geomean E. Coli	Flow	BOD	TSS	TKN	NO2 + NO3	Total Nitrogen-N	Ammonia	TP	Geomean E. Coli	
Year	MGD	mg/L	lbs/day	mg/L	lbs/day	mg/L	mg/L	mg/L	lbs/day	mg/L	lbs/day	mg/L	lbs/day	MPN/100 ml	MGD	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	MPN/100 ml	MGD
2014	0.34	4.19	12.17	3.53	10.08	1.20	0.50	2.09	5.97	3.29	9.45	0.25	0.76	1.28	0.25	2.72	4.22	0.82	2.28	3.10	0.22	0.79	3.54	0.58
2015	0.38	3.15	9.92	1.31	3.91	0.71	0.14	1.89	6.13	2.61	8.37	0.18	0.57	4.52	0.25	3.87	3.03	0.85	1.62	2.31	0.18	1.13	2.72	0.63
2016	0.45	2.15	7.87	1.21	4.57	0.45	0.17	2.32	8.44	2.76	10.13	0.21	0.76	4.80	0.25	3.44	1.57	0.81	1.58	2.39	0.23	2.02	2.57	0.70
2017	0.38	2.50	7.88	1.77	5.48	0.85	0.20	1.46	4.60	2.32	7.29	0.46	1.43	1.22	0.21	3.17	1.52	0.90	1.42	2.30	0.16	1.82	5.99	0.59
2018	0.49	3.44	12.27	2.80	10.26	1.13	0.37	1.72	6.74	2.91	11.36	0.45	1.82	20.59	0.36	2.76	1.17	1.09	1.58	2.63	0.19	1.68	3.69	0.84
2019	0.59	2.02	9.917	0.99	4.836	0.72	0.28	1.83	8.985	2.55	12.511	0.66	3.241	31.31	0.37	1.27	0.67	0.98	1.58	2.56	0.18	1.67	5.22	0.96
2020	0.51	1.87	7.992	0.79	3.358	1.34	0.41	1.21	5.186	2.55	10.906	0.88	3.750	117.81	0.37	1.76	0.86	1.00	1.50	2.46	0.40	1.84	7.64	0.88
2021	0.59	1.63	8.052	2.84	14.036	1.86	0.54	1.63	8.063	3.15	15.528	0.68	3.348	461.08	0.45	2.41	1.89	1.96	2.03	3.77	0.57	1.58	236.15	1.04
2022	0.46	2.40	9.166	2.08	7.940	1.74	0.63	1.39	5.301	2.82	10.799	1.16	4.428	2.40	0.39	2.06	2.35	1.37	1.83	2.71	0.37	2.43	234.03	0.85
2014-2022																								
Max	0.588	4.187	12.274	3.529	10.261	1.338	0.500	2.319	8.985	3.287	12.511	0.877	3.750	117.814	0.368	3.869	4.221	1.091	2.275	3.097	0.399	2.021	7.643	
Avg	0.447	2.758	9.717	1.770	6.071	0.914	0.294	1.789	6.578	2.712	10.002	0.441	1.762	25.933	0.292		1.864	0.922	1.650	2.535	0.224	1.567	4.481	
Min	0.335	1.870	7.865	0.786	3.358	0.446	0.136	1.214	4.597	2.316	7.286	0.176	0.571	1.225	0.208	1.271	0.666	0.810	1.423	2.305	0.164	0.794	2.569	
											Max Load (Ib													
1									TN: 1501.3 TP: 450.0															



Data 3: Influent Sample Spreadsheet

Cent	treville WW	24 hr Com	posite Influ	ent Samplii	ng 2023			Centr	eville WW	24 hr Comp	osite Influe	ent Sampling 201	7	
Sample Date	BOD	TSS	Ammonia	TKN	Nitrate/Nitrite		Sample Date	BOD	TSS	Ammonia	TKN	Nitrate/Nitrite	pН	TP
3.20.23	148	76	28.2	36.4	0.24	1	9.19.17	71.53	57	26.3	26.41	<0.065	7.06	1.5
3.22.23	115	121	27.5	36.7	1.91	2	9.20.17	79.8	57	26.5	29.13	<0.042	7.12	1.73
3.24.23	135	137	21	16.2	<0.10	3	10.03.17	184.8	358	39.8	46.46	<0.065	7.57	7.97
3.27.23	129	124	21.1	34.4	<0.10	4	10.04.17	101.1	70	28.4	32.88	<0.042	7.34	3.63
3.29.23	122	189	26	41.3	<0.10	5	10.10.17	132.2	60	36.2	31.85	<0.042	7.19	3.57
3.31.23	109	50	37.4	38.2	1.01	6	10.11.17	199.3	120	30.3	38.46	<0.042	7.27	5.57
						7	10.17.17	96.7	75	32.1	29.45	0.304	7.26	2.4
						8	10.18.17	106.5	62.5	36.9	30.27	<0.065	7.22	2.13
						9	10.24.17	180.4	73	35.9	37.12	<0.065	7.2	3.53



Appendix D

# Major Process Equipment Catalog Information



Item 1: Influent Screening



# **BUDGETARY PROPOSAL**

May 02, 2023

## **CENTREVILLE WWTP**

**TOWN OF CENTREVILLE MARYLAND** 

Ovivo<sup>®</sup> Ozzy<sup>™</sup> Cup Screen

# **PREPARED FOR**

Whitman, Requardt & Associates, LLP

David Nixson, P.E.

# **AREA REPRESENTATIVE**

Sherwood Logan & Associates

**Andrew Kreider** 

# **PREPARED BY:**

### **RICHARD QUICK**

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Ovivo USA, LLC is pleased to submit a budgetary proposal for the following equipment (the "Products") on the project indicated above (the "Project").

While every effort has been made to ensure this quotation captures the intent of the project, we do anticipate further discussion in order to clarify and/or finalize the scope, terms & conditions and other details prior to any formal agreement. We look forward to your favorable review of our offer to further discussions on this important project.

THIS BUDGETARY PROPOSAL CONSTITUTES A NON-BINDING ESTIMATE OF PRICE(S) FOR CERTAIN GOODS AND/OR SERVICES THAT MAY BE PROVIDED BY OVIVO USA, LLC FROM TIME TO TIME, BUT SHALL NOT BE CONSTRUED AS A CONTRACTUAL OFFER FOR OVIVO USA, LLC TO PROVIDE SUCH GOODS AND/OR SERVICES. ANY CONTRACTUAL OFFER FOR THE SUPPLY OF GOODS AND/OR SERVICES BY OVIVO USA, LLC SHALL BE CONVEYED TO CUSTOMER IN THE FORM OF OVIVO USA, LLC STANDARD PROPOSAL DOCUMENT, WHICH INCLUDES, BUT IS NOT LIMITED TO, ITS STANDARD TERMS AND CONDITIONS OF SALE. SUCH PROPOSAL FORM MAY BE PROVIDED TO CUSTOMER UPON REQUEST.

### **Budgetary Pricing for Proposed Equipment:**

ITEM	EQUIPMENT	PRICE
I	Ovivo® Ozzy™ Cup Screen, 6' Diameter, Troughing, Controls, Model 250 Compactor, and Field Service	** *\$503,00

\*Please see your local Ovivo Rep for:

- Explosion proof environments
- Special Spec Requirements or Testing

\*\* Pricing is only valid for 30 days

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# **ITEM I STANDARD SCOPE OF SUPPLY**

### ITEMS INCLUDED:

- Two (2) Ovivo<sup>®</sup> Ozzy<sup>™</sup> Cup Screens, 304 SS Fabrication, 5 MGD max flow each screen, to include:
- Drum Screen width: 3'.
- Drum screen diameter: 6'.
- 1HP, 1800 RPM, TEFC helical gear motor suitable for 460/3/60 supply, Outdoor (TEFC).
- Standard nylon rack and pinion gear drive.
- Spray wash hood and nozzles.
- 2mm Ovivo ProPaPanel<sup>®</sup>.
- Underflow spray wash.
- Seal and diverter plate in SS with UHMW Seal plates for flow path.
- 1.0" brass solenoid valve and pressure gauge.
- Wash water requirement of 25 GPM @ 45 psi minimum.
- Anchor and Assembly Fasteners.
- Trough between Ozzy and Compactor, 9" x 9" x 10' Long.
- Two (2) Shafted Screw Compactor, Ovivo Model 250 in 304 SS, to include:
- Capacity: 64 cubic feet per hour.
- Motor size: 3HP,1800 RPM, screw compactor motor suitable for 460/3/60 supply, Outdoor (TEFC).
- Shafted screw in ASTM A36 carbon steel.
- Screw brush on periphery of screw flights- Nylon.
- U-shaped screw housing /drainage trough approximately 1mm smaller diameter than drum screen.
- Self-aligning thrust and radial load bearing to support the screw at the inlet end.
- High performance plastic sleeve bearing at the outlet end of the screw.
- Screw compactor reject drain connection: 4-inch diameter.
- Wash water requirement for screening rinse: 6-16 GPM @ 16 psig, 1.0-inch NPT with brass solenoid.
- Tubular 304 stainless steel compactor discharge chute angled at a minimum of 45 degrees.
- 304 SS stainless steel discharge chute supports.
- Anchor and Assembly Fasteners
- Two (2) Standard NEMA 4 Control Panel:
  - 460 VAC System.
  - Main Disconnect.
  - H-O-A Switch.
  - Motor Starters with Timers.
  - HI and HI HI Float Switches.
  - Solenoid Valve Control.
  - Emergency Stop Pushbutton.

Freight, FCA to job site.

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#### BUDGET PROPOSAL, PATENTED OZZY CUP SCREEN

#### ITEMS NOT INCLUDED (But not limited to the following):

- Access ladder, platform, or stairs.
- Concrete, grout, or concrete design.
- Consumables.
- Control panel mounting and field wire terminations.
- Disposal of any kind.
- Dumpster.
- Field wire and field conduit
- Field or shop paint.
- Grating.
- Installation.
- Lubricants.
- Man lifts or cranes.
- Offloading at job site.
- Piping and piping insulation.
- Recordings of training sessions.
- Spares.
- Special tools.
- Special site PPE.
- Storage.
- Taxes.

### FIELD SERVICE OPTION:

- One (1) Trip / Three (3) Days at the site to assist in adjusting, servicing, and checking out these mechanisms, and in training the operators in maintenance, troubleshooting, and repair of the equipment.
- Additional service days can be purchased at the current rate.

### **TYPICAL LEAD TIMES:**

Submittals: 10 weeks after Purchaser's receipt of Ovivo's written acknowledgement of an approved purchase order.

Shipping: 24 weeks after receipt of approved drawings from Purchaser.

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# **SIZING INFORMATION**

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Step 1 OZZY Sizing				DATE:	5/2/2023
REVN. #	Α	(INITIAL RELEASE)		SHEET REV:	0
PROJECT NUMBER / ORDER NUMBER	Centreville WWT	ſP			
CLIENT	Town of Centrev	ille			
OVIVO ENGINEER:	RQ			SITE:	
OVIVO MANAGER	JRH			SCREEN LOCATION:	IN DOOR
PRODUCT NAME:	Ozzy Single Entry	y Drum Screen	L	IQUID BEING SCREENED:	SEWAGE WATER
# OF DUTY & STAND-BY SCREENS	1 Duty + 1 Stand	by		FLOW CONDITIONS:	PEAK
SCREEN SERVICE LOCATION	MEMBRANE PRO	DTECTION		WEATHER:	WET

THIS INFORMATION IS CONFIDENTIAL & PROPRIETARY IN NATURE AS IT CONTAINS TECHNIQUES USED BY OVIVO FOR SCREEN DESIGNS FROM OVER 90 YEARS OF DEVELOPMENT. THIS DOCUMENT IS NOT TO BE REPRODUCED IN ANY FORM WITHOUT THE EXPRESS WRITTEN CONSENT OF OVIVO USA, INC.

#### SUMMARY OF GENERAL INFORMATION

VELOCITY CALCULATIONS FOR DRUM SCREEN (WITH 100	% CLEAN N	/IESH):			MESH SIZE 2.00	mm
FLOW	7.74	ft <sup>3</sup> /s	0.219	m³/s		
FLOW (MGD)	5.00	MGD				
FLOW (GPM-US)	3,472	GPM (US)				
INLET WATER DEPTH	2.75	ft	0.838	m		
VELOCITIES THROUGH INLET V <sub>SE</sub> @ SPECIFIED WATER LEVEL	1.90	ft/s	0.580	m/s		
VELOCITIES THRU' MESH @100% CLEAN						
DRUM SCREEN WIDTH:	2.77	ft	0.845	m		
V <sub>MESH</sub> @ SPECIFIED WATER LEVEL	1.41	ft/s	0.430	m/s		

#### SUMMARY OF TOTAL HEAD-LOSSES FOR DRUM SCREEN:

HEADLOSS CALCULATIONS ARE DERIVED FROM THE STANDARD FLUID EQUATION  $V = C(2gH)^{1/2}$ , THIS EQUATION HAS BEEN REDUCED TO THE FOLLOWING STANDARDS BASED ON YEARS OF INVESTIGATION.

ACROSS MESH  $\Delta H = 116*V^2$ SEALING WALL  $\Delta H = 135*V^2$ 

BASED ON FOLLOWING UNITS:  $\Delta H$  = HEADLOSS (mm) V = VELOCITY (m/s)

#### THE BELOW IS A SUMMATION OF THE TOTAL HEADLOSS ACROSS THE SCREEN @ THE WATER LEVEL & % CLEAN

WATER LEVEL CONDITION: PEAK	%CLEAN	INCH		mm
HEADLOSS ACROSS MESH	100%	0.8	$\Delta H$	21
HEADLOSS ACROSS SEALING WALL	100%	1.8	$\Delta H$	45
TOTAL HEADLOSS @LEVEL & % CLEAN	100%	2.6	$\Delta H$	67
TOTAL HEADLOSS @LEVEL & % CLEAN	75%	3.3	$\Delta H$	84
TOTAL HEADLOSS @LEVEL & % CLEAN	50%	5.2	$\Delta H$	131
TOTAL HEADLOSS @LEVEL & % CLEAN	25%	15.3	ΔH	389

#### IMPORTANT: THE RESULTANT HEADLOSSES ARE BASED ON THE FOLLOWING INPUTS:

SCREEN SERVICE	MEMBRANE PR	OTECTION		
FLOW	7.74	ft3/s	0.219	m3/s
FLOW	5.00	MGD	3472	GPM(US)
DECK ELEVATION	3.50	ft	1.07	m
CHANNEL DEPTH (w/o drum invert)	3.50	ft	1.07	m
CHANNEL WIDTH (minimum)	4.50	ft	1.37	m
VELOCITY THRU' INLET @ SPECIFIED FLOW	1.90	ft/s	0.58	m/s
VELOCITY THRU' INLET CILLS	1.86	ft/s	0.57	m/s
VELOCITY THRU' MESH	1.41	ft/s	0.43	m/s
VELOCITY THRU' EXIT	1.11	ft/s	0.34	m/s
PROPAPANEL HOLE DIAMETER	0.08	in	2.00	mm
Total Mesh % opening	0.35	%	0.35	%
CORRESPONDING DRAWING DIMENSIONS:				
DRUM SCREEN LEVELS (ELEVATIONS):				
WATER ELEVATION (FROM UNDER DRUM INVERT)	2.75	ft	0.84	m
SCREEN CENTRELINE LEVEL	3.78	ft	1.15	m
UNDER DRUM INVERT	0.00	ft	0.00	m
			NDITION: PEA	
INLET WATER DEPTH	2.75	ft	0.838	m
INLET ELEVATION	0.00	ft	0.000	m
DRUM SCREEN DETAILS:				
DIAMETER OF SCREEN	6.00	ft	1.829	
				m
DRUM SCREEN OUTLET CHAMBER WIDTH	3.00	ft	0.914	m
DRUM SCREEN WIDTH	2.77	ft	0.845	m
DRUM SCREEN DIMENSIONS (A Frame):				
Min. Free Board "A"	0.75	ft	0.229	m
PEAK Trial Immersion Level "B"	1.97	ft	0.600	m
	,			
Drum Center to Deck Level "C"	0.28	ft	0.451	m
Nominal Drum Diameter "D"	6.00	ft	1.829	m
Height Under Drum "E"	0.78	ft	0.238	m
Screen Width Over all "F"	3.00	ft	0.914	m
	0.00		0.014	
Deck Level to Top of Drum "G"	3.70	ft	1.128	m
Diverter Plate Inlet Width "H"	1.50	ft	0.457	m
Channel Width (minimum) "J"	4.50	ft	1.372	m
Length of Drum Chamber (minimum) "K"	10.58	ft	3.225	m

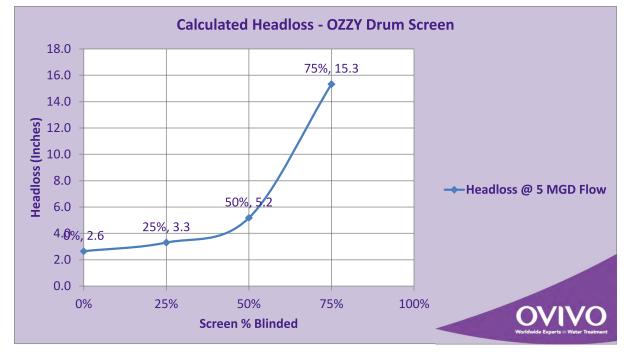
# **OZZY** Sizing

Sheet Rev: 0

### **GRAPHICAL REPRESENTATION OF % BLOCKED MESH & HEADLOSS IN INCH**

PERCENTAGE CLEAN MESH (%)	PERCENTAGE BLOCKED MESH (%)	HEADLOSS (INCH)
25%	75%	15.3
50%	50%	5.2
75%	25%	3.3
100%	0%	2.6

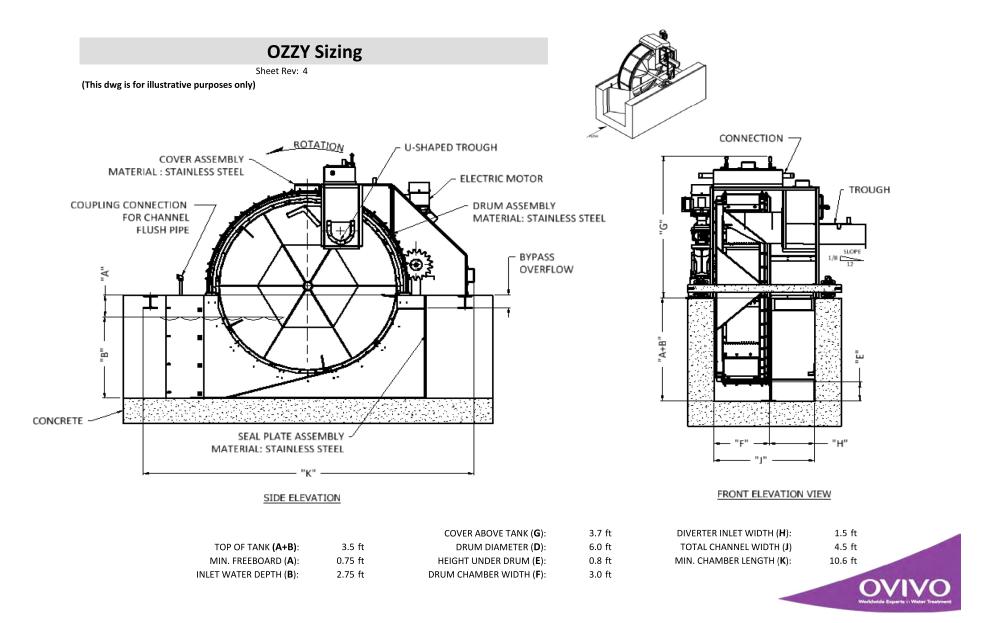
FLOW DESCRIPTION	5 MGD Flow And 2.8 feet Inlet Wate	er Depth.
GRAPH LINE LABEL	Headloss @ 5 MGD Flow	
FLOW IN MGD	5.0	MGD
UPSTREAM FLUID DEPTH	2.8	ft

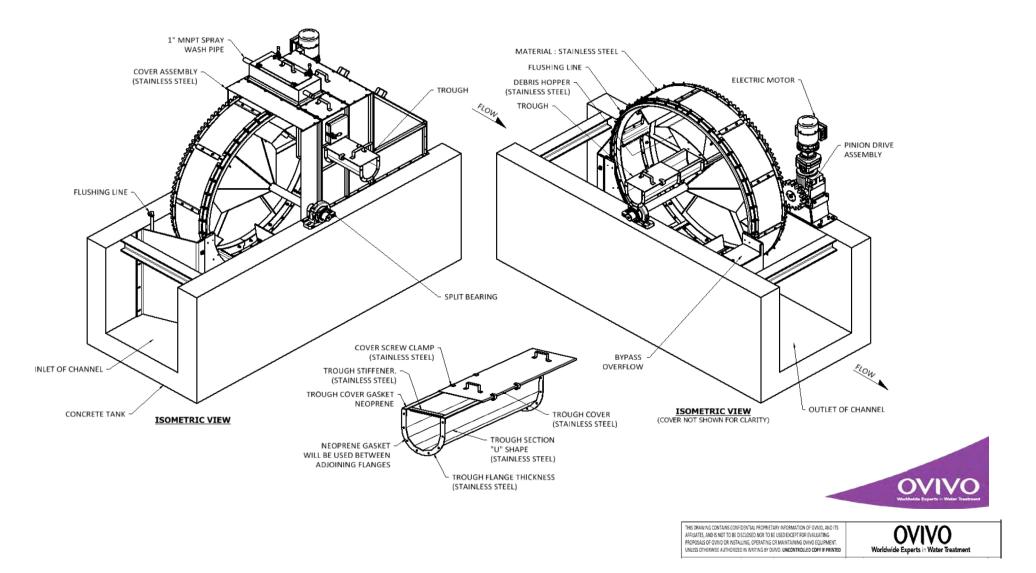


THIS INFORMATION IS CONFIDENTIAL & PROPRIETARY IN NATURE AS IT CONTAINS TECHNIQUES USED BY OVIVO FOR SCREEN DESIGNS FROM OVER 90 YEARS OF DEVELOPMENT. THIS DOCUMENT IS NOT TO BE REPRODUCED IN ANY FORM WITHOUT THE EXPRESS WRITTEN CONSENT OF OVIVO USA, INC.

# **DRAWINGS**

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# **BROCHURES**

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# PROVEN OVIVO DRUM SCREEN TECHNOLOGY NOW SMALL, AND IN CHANNEL

High performance capture ratios

Straight channel design

Exceptional solids handling capabilities

Retrofitable to existing channels

No maintenance below grade

6mm to 0.5mm apertures available

Up to 5' deep channels

High reliability for constant flows

# OVIVO® OZZY CUP SCREEN

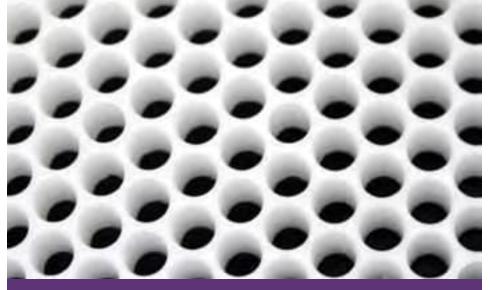
Interested in maximizing the life of your downstream equipment?

Call 1-855-GO-OVIVO to speak with an Ovivo Expert.

www.ovivowater.com Copyright © 2019 Ovivo, All rights reserved. 1-855-GO-OVIVO

# OVIVO'S HIGH PERFORMANCE, LOW MAINTENANCE, ELITE SCREENING TECHNOLOGY

Ovivo's Ozzy drum screens are designed to meet the increasing demand for high capacity coarse and fine screening of raw or wastewater coupled with a robust low maintenance operation.



Close up of Ovivo's ProPaPanel® technology

#### THE OZZY CUP SCREEN IS THE **RESULT OF DECADES OF EXPE-RIENCE DEVELOPING SOME OF** THE LARGEST DRUM SCREENS IN THE WORLD

- Low capital and maintenance cost
- Low energy usage
- Simple, slow rotating mechanism
- Simple to maintain
- Paired with the Ovivo's ProPaPanel to reduced hair-pinning, and maximize corrosion resistance and durability.

# **HOW IT WORKS**

The Ovivo Ozzy Cup screen consists of a robustly constructed drum structure with a solid horizontal main shaft, which revolves slowly in heavy duty, self-aligning roller bearings.

Water flows from the inside to the outside of the drum through mesh panels arranged around its periphery. Mesh panels are cleaned by spray wash nozzles mounted on the outer side of the drum screen. The screenings are then caught by a screening hopper and conveyed to the screw compactor through a sluice trough.

The screen is driven by a simple drive unit positioned at deck level. The final drive is a nylon pinion, which engages with a gear ring on the outside of the drum.

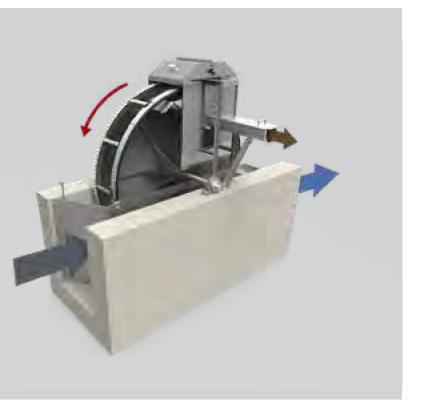
The drum screen structure can be designed to support high differential loading without failure of the mesh panels, thereby ensuring that the downstream plant does not become contaminated by unscreened water and debris.

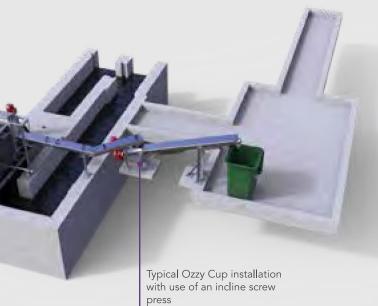






### MUNICIPAL WASTEWATER | INLET WORKS





Design allows for the Ozzy Cup to be installed in new or existing straight channels

### OVIVO® Ozzy Cup Drum Screen

#### **AVAILABLE SIZES**

Diameter\*: 4'-8' (2'-4' channels depths)

Width\*: 0.5'-3'+

Aperture diameter: 0.5, 1, 2, 3, 5, 6mm

Flow range up to 10 MGD at 150mg/L TSS\*

\*For specific flow capacity and sizing, please contact your local Ovivo Representative.



# **ENGINEERING SERVICES**

#### **DESIGN AND ANALYSIS**

Ovivo advanced 3D graphics and modeling, products are designed for different operating conditions and requirements for its customers.

#### INSTALL, COMMISSION, MAINTAIN

Ovivo's service engineers can install, commission, maintain all machines and will visit sites around the world to advise on all aspects of our products.

#### **SPARE PARTS**

All spares supplied are genuine, guaranteed and supported by our detailed knowledge of all historical modifications or upgrades.

#### TRAINING

As a supplier of engineered capital equipment, we offer our end users onsite or in-house training courses. Contact our spares and service managers for details of the courses available.





#### ALL OF YOUR MANUALS, ALL OF YOUR KNOWLEDGE, ALL IN ONE PLACE.

Get your team on the same page. Upload and share documents & media. Create and manage service logs and maintenance schedules.

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### 1-855-GO-OVIVO

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# **Centreville**, **MD**

Equipment: HUBER Perforated Plate Screen ROTAMAT<sup>®</sup> RPPS 1200/2

### **Represented by:**

**JDGET PROPOSAL** 

~

Sherwood Logan Associates Andrew Kreider (410) 274-3716 akreider@sherwoodlogan.com

### **Regional Sales Director:**

Brian Baker 704-840-3085 Brian.Baker@hhusa.net

Project Number:	497687
<b>Revision:</b>	0
Date:	5/9/2023

HUBER Technology, Inc. 1009 Airlie Pkwy, Denver, NC 28037 704-949-1010 | www.huber-technology.com



### **Design Information**

Т	echnical Data	
Peak Waste Water Design Flow Per Screen	2.5	MGD
TSS Concentration	250	mg/L
Screen Basket Spacing	2	mm
Maximum Upstream WaterLevel	31.61	inch
Screen Type	Perforated Plate	-
Screen Basket Diameter	1200	mm
Sealing between Stationary Baffle Plate and Rotating Drum	Polyurethane Seal	-
Installation type	Channel	-
Screen Angle	35	0
Wash Water Pressure	75	psi
Wash Water Consumption	39	gpm

### **Equipment Details**

Model	HUBER Perforated Plate Screen ROTAMAT <sup>®</sup> RPPS 1200/2
Quantity	1 (including 1 standby unit)
Material	304L stainless steel construction; pickled and passivated in acid bath
Screen Design	Shafted screw with integrated maintenance free bearing and inclined auger tube
Screenings Wash	One (1) solenoid valve (s) for screenings wash, 1-inch, 120 VAC, 2-way, Class 1 Division 1, Brass body
Spray Bar	One (1) solenoid valve (s) for spraybar wash, 1-inch, 120 VAC, 2-way, Class 1 Division 1, Brass body
Press Zone	One (1) solenoid valve (s) for compaction wash, 1-inch, 120 VAC, 2-way, Class 1 Division 1, Brass body
Cleaning Brush	Stainless steel backed nylon brush with bristles for perforated plate basket cleaning
Motor Data	2 HP, 480 VAC, 3ph, 60 Hz, S.F. 1.15, Class 1 Division 1
Sensor	Level sensor for waterlevel measurement
Supports	304L Stainless Steel Construction
Anchor Bolts	M12, 316L, Included
Screen Basket Cover	Screen basket with hinged lid, 304L stainless steel

### **Control Details**

Two (2) Main Control Panels				
Enclosure	NEMA 4X, Stainless Steel			
PLC	Allen Bradley MicroLogix			
НМІ	Allen Bradley PanelView Plus 800			
Pre-programmed and Factory Tested				



### Pricing

Equipment	Model	Quantity	Pricing
HUBER Perforated Plate Screen	ROTAMAT <sup>®</sup> RPPS 1200/2	2	Included
HUBER Control Panel	HUBER Standard	2	Included
Freight and Startup Services	Standard HUBER Start-up Services	3 day(s), 1 trip(s)	Included
TOTAL:			\$320,000.00

Standard delivery is 22-30 weeks from approval of submittals.

Thank you for your interest in HUBER Technology, Inc. If you have any questions, please do not hesitate to contact our Regional Sales Director or our local sales representative.

This proposal has been reviewed for accuracy and approved for issue by: JW

#### **Notes and Technical Clarifications**

- 1. Equipment specification and drawings are available upon request.
- 2. If there are site-specific hydraulic constraints that must be applied, please consult the manufacturer's representative to ensure compatibility with the proposed system.
- 3. Electrical motor disconnects required per local NEC code are not included in this proposal.
- 4. All electrical interconnections, wirings, junction boxes, and terminations between the equipment and electrical components are to be provided by installing contractor.
- 5. HUBER Technology warrants all components of the system against faulty workmanship and materials for a period of 12 months from date of startup or 18 months after shipment, whichever occurs first.
- 6. Budget estimate is based on HUBER Technology's standard Terms & Conditions and is quoted in US dollars unless otherwise stated.
- 7. Equipment recommendations are based on information provided to Huber Technology. Subsequent information which differs from what has been provided may alter the equipment recommendation.
- 8. Any item not specifically listed is not considered part of this scope of supply. Please contact the HUBER Technology representative listed for further clarification.
- 9. HUBER will ship all equipment to site inside of 20', 40' or 40'OT ocean containers as deemed appropriate by our factory. HUBER will not ship any equipment on flatbed truck. Flatbed truck shipping means that the equipment would need to be transferred at port from factory packaged containers to the flatbed. This process it out of HUBER's control and it is our experience that equipment always gets damaged during this process.
- 10. Equipment that is broken out in "Pricing" tab are only valid when packaged together.
- 11. All piping to and from the equipment is to be supplied by the installing contractor.

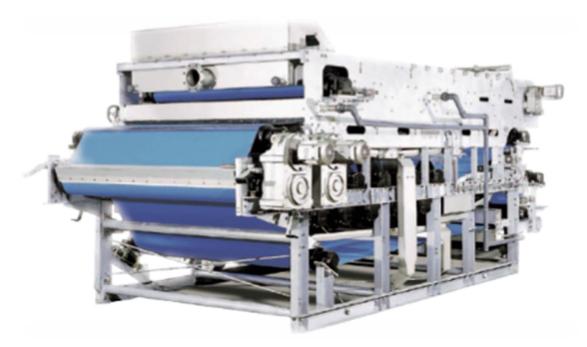


Item 2: Dewatering Solids



### Project Name: Town of Centreville, MD WWTP

### Alfa Laval AS-H Extended Belt Press G3 200 – 3 Belt (Klampress®) for Sludge Dewatering



Alfa Laval Reference No. 0496181 Rev 0 May 12, 2023 Quote Validity: 30 days

### Prepared by:

Brian Ayres Applications Engineer Brian.Ayres@alfalaval.com

Alfa Laval, Inc. 804-222-5300 5400 International Trade Drive Richmond, VA 23231 David R. Nixson, P.E. Associate dnixson@wrallp.com

Whitman, Requardt & Associates, LLP 443-224-1641 801 South Caroline Street Baltimore, MD 21231

#### Prepared for:

Alfa Laval USA Inc. Ref.: 0496181 May 12, 2023



PROJECT NAME ALFA LAVAL REFERENCE Town of Centreville, MD WWTP 0496181

### Dear Mr. Nixson

Thank you for your enquiry. On behalf of Alfa Laval and our local representative Sherwood Logan & Associates, Inc., we are pleased to enclose our non-binding Budget Quotation for **One (1) Alfa** Laval AS-H Extended Belt Press G3 200 – 3 Belt (Klampress<sup>®</sup>), 2Meter Belt Filter Press (BFP) (3 Belt) for the Town of Centreville, MD WWTP project.

The Alfa Laval AS-H Belt Press G3 is the next generation dewatering belt filter press that was developed from tried-and-true Klampress design. It is suitable for all municipal biosolids and residual sludge types and a wide variety of industrial solid / liquid separation applications. It incorporates variable energy mixing, flocculation, gravity drainage and pressure filtration within a single mechanical framework. The G3 belt press offers the versatility of a wide size range (up to 3 meters) and extensive modular options to meet individual process requirements. In summary...

- Flexible design easily upgraded and reconfigured as your needs change
- High precision variable-orifice polymer mixer
- High volume, high cake solids performance
- Low maintenance, operator friendly design features
- Alfa Laval offers unrivalled 24-hour service agreements.

As requested, we have included the scope of supply and applicable process guarantees based on the defined influent sludge parameters. Technical details along with dimensional drawing for the proposed belt press including weights, are enclosed in the proposal.

Alfa Laval recommends the described equipment per the outlined technical specifications, and additional clarifications for greater understanding of the offer. We trust that we have interpreted your requirements correctly and shall be pleased to provide any additional information which may be required in support of our proposal.

Note: Kindly indicate our Quotation Reference in your Purchase Order/ Letter of Acceptance/ Sales Contract and all our correspondences if the order is confirmed to us.

Regards, Mark Schlitzkus

Mark Schlitzkus Regional Sales Manager Alfa Laval Inc.

CC: Andrew Kreider, P.E. | Sherwood Logan & Associates, Inc.



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- 1. BASIS OF DESIGN
- 2. <u>PROPOSAL</u>
- <u>COMMERCIAL TERMS</u>
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   3.2 Payment Terms
   3.3 Delivery Time
- 4. <u>ALFA LAVAL</u>
- 5. <u>BELT FILTER PRESS G3 GENERAL DESCRIPTION</u>
- 6. <u>ELECTRICAL ASSEMBLY AND CONTROLS</u>
- 7. <u>SERVICE</u>
- 8. TERMS & CONDITIONS OF SALE



#### 1. BASIS OF DESIGN.

### **General Data**

Sludge Origin:	Municipal Wastewater Treatment Plant
Duty:	Dewatering

### Sizing Data

### Number of units

Operating:	1			
Standby:	0			

Customer requirements: 2 m press with gravity deck.



### 2. PROPOSAL

- 2.1. One (1) Alfa Laval AS-H Extended Belt Press G3 200 3 Belt (Klampress®), will come complete and include the following scope of supply:
  - Components fabricated of the finest corrosion-resistant material
    - the frame will be carbon steel, hot dipped galvanized
    - o chicane rods and holders shall be carbon steel galvanized
    - o all sheet metal components will be Type 316L stainless steel
  - Extended gravity section
  - Rollers. See description under 5.4.4 below
    - Eight (8) solid rollers
    - o Perforated roller
  - Bearings greased lubricated
  - Three (3) belts
  - Hydraulic actuated belt alignment and positioning system
  - Hydraulic power unit
    - One-gallon reservoir
    - Hydraulic oil pump and drive motor
    - Other parts to make a complete operational system
  - Belt wash system
    - Washwater requirements
    - o 120 GPM at minimum of 85 PSI at the BFP
  - Power requirements
    - One (1) 2 HP GRAVITY ZONE DRIVE UNIT
    - One (1) 3 HP BELT DRIVE UNIT
    - One (1) 1 HP HYDRAULIC UNIT

### 2.2. One (1) Main Control Panel

- One (1) Enclosure, NEMA 4X, 304SS, wall mount enclosure
- One (1) Main disconnect breaker with operating handle
- One (1) Control power transformer
- One (1) 480V surge suppressor
- One (1) 120V surge suppressor
- One (1) AB Power flex 525, 3HP VFD, 480VAC with fuse protection (PRESSURE SECTION DRIVE)
- One (1) AB Power flex 525, 2HP VFD, 480VAC with fuse protection (GRAVITY SECTION DRIVE)
- One (1) Motor starters Non-reversing, IEC, 1HP rated with circuit protection (HPU)
- One (1) Motor starters Non-reversing, IEC, 10HP rated with circuit protection (WWBP)
- One (1) 24VDC power supply
- One (1) Allen Bradley, CompactLogix L30ER controller with I/O as required.
- One (1) Unmanaged ethernet switch
- Five (5) Red Line PID controller with window kit
- One (1) Alarm horn
- One (1) Elapsed time meter
- One (1) Ground bar
- One (1) Internal cooling fan



- One (1) Panel heater with thermostat
- One (1) Alarm horn
- LOT of Pilot Operators
- LOT of Terminal blocks, relays, dry contacts, and supplementary circuit protection as required

### 2.3. One (1) Inline, Non-clog, Variable Orifice Mixer

• Complete with an injection manifold system and a four-port vortex polymer injection ring.

### 2.4. One (1) Washwater Booster Pump, which shall have the following specs:

- Goulds Centrifugal Pump
- Flow rate 120 GPM
- Max Pressure 120 psi

### **2.5.** One (1) Lot Spare Parts, which shall be provided as follows:

- One (1) set of filter belts
- Two (2) sets of doctor blades
- Two (2) sets of rubber seals for the gravity zone & wash box
- One (1) set of bearings of each size used

### 2.6. Service time as follows:

- One (1) Field Service Engineer,
- up to eight (8) days, @ 10 hr./day, with up to three (3) round trips, per unit for startup, commissioning, and training.
- Any additional service time resulting from non-Alfa Laval-warranty delays, will be charged at the rate in effect at the time of service.

### 2.7. Also included with pricing:

• Warranty: Per the enclosed Alfa Laval's Standard Terms & Condition of Sale. Alfa Laval reserves the right to review operating and maintenance records to ensure compliance.

Each unit is warranted to be free from defects in materials and workmanship for a period of twelve months after successful completion of Acceptance Testing, beneficial use, or for a period not to exceed eighteen months from shipment, whichever occurs first. Alfa Laval reserves the right to review operating and maintenance records to ensure compliance.

- We are offering this FCA Incoterms 2020
- Electronic Submittal and O&M Manual

### 2.8. Dimensioned drawing (See Appendix A)

### 2.9. Notes of Clarification



- Scope of supply is per Alfa Laval standard BFP configuration, and in accordance with typical specifications and drawings. Any additional items not explicitly stated in this proposal or standard to Alfa Laval's typical specifications are not included in this quotation. The specified equipment is intended for installation within a non-hazardous safe area.
- Equipment to be supplied by Alfa Laval (and /or sub-supplier), as specified in this quotation, are standard machines. Any modifications / additions other than those expressly specified in the quotation shall incur extra engineering cost, material cost and delivery time.
- Technical submittal documentation shall be per Alfa Laval's (and /or sub-supplier) standards, delivered electronically, in English language. Additional documentation requirements shall incur extra engineering cost, material cost and delivery time.
- The enclosed quotation is a non-binding budgetary quotation. Therefore, price, scope and other terms contained within this budgetary quotation are subject to considerable variations when preparing our binding quotation. All scope of supply modifications / additions requires prior agreement by both parties and written acknowledgement by Alfa Laval.

### 2.10. Escalation Charges:

- In the event that delivery of equipment cannot be made on the scheduled delivery date agreed upon between Alfa Laval and Purchaser and as evidenced by the terms of the contract, due to Purchaser delay, Alfa Laval reserves the right to assess reasonable escalation charges to the project at the rate of 1% per month of the contract value for material price escalation for each month that the project is delayed.
- Given the current volatility in steel prices over the past twelve months, Alfa Laval has made this offer based upon shipment of the offered products contained herein within the schedule dictated above. Should the projected shipment schedule fall outside this period for any reason, pricing shall be subject to review and revision.

### **2.11.** Exclusions from this quotation:

- All mechanical & electrical Installation
- Equipment offloading and placement
- Field wiring, conduit, and electrical flexible connections...etc.; contractor shall remain responsible for meeting all relevant electrical codes
- Pipes, valves, and fittings...etc.
- Sludge Hopper with Level Probes/Sensors
- Feed Pump, Booster pump, strainers, etc.
- Associated equipment, i.e., sludge macerators, feed pumps, polymer preparation & dosing unit, cake conveyors, centrate tanks and pumps...etc.
- Measuring instruments between equipment and associated equipment
- Noise abatement enclosures
- Odor control equipment



- Inspection and access platforms or ladders
- Utilities and consumables (polymer, power, water, and other consumables required during testing, start-up and commissioning)
- Lab services fees for the performance test and startup
- Storage and handling fees
- Detailed or project specific related engineering
- Duties, taxes, bonds...etc.
- Freight to jobsite

### 2.12. Process performance is per specified basis of design.

• The belt filter press performance (cake solids, loading, hydraulic throughput, etc.) is verified through onsite analysis of representative sampling during equipment commissioning. Variation of sludge feed may impact performance.

### 3. COMMERCIAL TERMS

### 3.1. Pricing

Item	Description	Qty.	Unit Price	Extended Price
1	Belt Filter Press G3 200 – 3 belt Extended	1	Included	Included
2	Set of Controls	1	Included	Included
3	Set of Ancillaries	1	Included	Included
4	Commissioning	1	Included	Included
5	Booster Pump	1	Included	Included
Total Budget Price			<u>\$385,500.00</u>	

### 3.2. Payment Terms.

- 10% with PO, N10 days
- 10% upon Alfa Laval Submittal Delivery, N30 days
- 75% upon delivery or availability to deliver should owner encounter delays, N30 days
- 5% upon acceptance or beneficial use, whichever comes first, N30 days, but not later than 120 days from shipment.

### 3.3. Estimated Delivery Time

- Submittals: 8 -12 weeks from fully executed PO
- Belt Filter Press: 20 24 weeks from receipt of approved submittals and/or release to manufacture

### 3.4. Quotation validity

• 30 days



### 4. ALFA LAVAL

### 4.1. About us

Alfa Laval is a leading global provider of separation, heat transfer, and fluid handling technology. Founded in 1883 and for more than 130 years, we have built a global presence with service centers and partners in nearly 100 countries. This offers local expertise, supported by the global breadth and depth of Alfa Laval. With these as its base, Alfa Laval aims to help enhance the productivity and competitiveness of its customers in various industries all over the world. Alfa Laval – Our Company.



### 4.2. Wastewater Separation Technologies

We remain committed to being the technology leader in design innovations, delivering reduced power & polymer consumption, increased cake dryness, and increased capacity within the same footprint. <u>Alfa Laval - Municipal wastewater treatment</u>

- Decanter Centrifuge
- Belt Filter Press
- Gravity Belt Thickener
- Rotary Drum Thickener
- SBR / MBR / Pkg. Plants

### 4.3. Lab & Pilot Testing

Alfa Laval's DNA is to continuously bring value to our customers. Our state-of-the-art wastewater laboratory, located in the Houston, TX service center; allows Alfa Laval to analyze the optimal technology for your specific separation requirements. Additionally, Alfa Laval provides separation equipment available for on-site field testing and demonstration. These include decanter centrifuge, rotary drum filter, and belt press.





### 4.4. Always at Your Service:

- 24/7 Support
- 75+ Authorized Service Providers
- 4 USA Service Centers -
- Indianapolis US Parts Distribution Center
- OEM Parts 450,000+ Spare Parts in Stock
- 50+ Field Technicians

Alfa Laval - Service and support in the USA

### 4.5. Spare Parts

### • A smart choice

Boost productivity and maximize uptime with quality genuine parts from Alfa Laval. With easy access to a broad range of long-lasting high-quality parts, you can lower your total cost of ownership and preserves the value of your equipment throughout its entire life cycle.

# Available everywhere Through our global service network

Through our global service network, you have easy access to our extensive genuine spare parts inventory through 11 major Alfa Laval distribution centers.

Alfa Laval maintains an extensive inventory of spare parts that supports our current product range as well as some legacy parts, which are up to 100 years old. Our parts inventory system contains specific information, such as technical details and availability, for more than 450,000 parts, and we have more than 50,000 unique items in stock.

The Americas are conveniently served through the American Distribution Center (AMDC), which is centrally located in Greenwood, IN, USA. Alfa Laval AMDC 200 South Park Blvd Greenwood, IN 46143

### Unmatched quality

Designed for durability, reliability and productivity, our parts deliver outstanding performance time and time again. Manufactured to precise specifications, Alfa Laval parts have proven performance in our material and test laboratories as well as in process lines around the world.

### • Traceability and certification

Parts are continuously improved to meet the highest standards and comply with various certification requirements and regulations, such as REACH. <u>Alfa Laval - Spare parts</u>





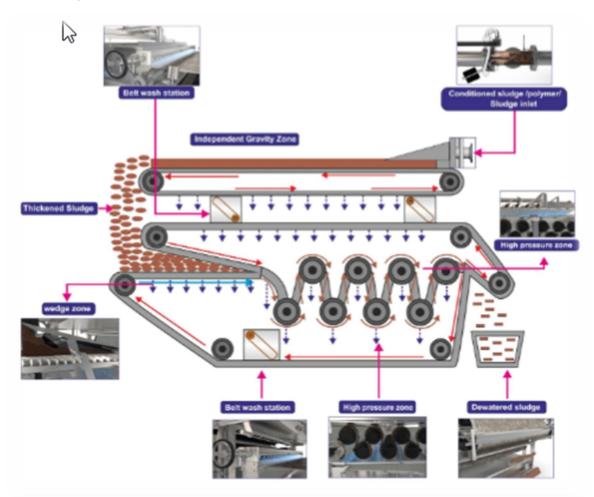




### 5. ALFA LAVAL AS-H BELT PRESS G3 – GENERAL DESCRIPTION

See how it works in less than 3 min. <u>Alfa Laval AS-H Belt Press G3</u> The Alfa Laval AS-H Belt Press G3 is considered the industry standard for superior value, performance and durability for sludge dewatering. The G3 belt press is designed for low polymer consumption, high throughput rates, and high solids content and is available in a wide size range and extensive modular options to meet individual process requirements.

### 5.1. Working Principle



The Belt Filter Press is furnished with an independent gravity drainage section with manual tensioning, and it can be operated on demand, as either as thickening device only or as a pre-thickening device prior to dewatering. This unit has separate speed control on the gravity section.

The thickened sludge is then sandwiched by a second filter belt before further dewatering by a series of decreasing diameter rollers. Final moisture removal is achieved by shear rollers arranged to give minimum 180-degree belt wrap in order to optimize dewatering.

## 5.2. Benefits

- Thorough uniform mixing of polymer into sludge
- Higher volumetric throughput and solids loading
- Higher cake dry solids
- Low power consumption
- Low polymer usage
- Better filtrate quality
- Low maintenance requirements
- Long life design
- Modular design allows upgrades to add more rollers in the pressure zone or an extended gravity zone

### 5.3. Features

- Available in 8 roller and 12 roller designs in the pressure section
- Extended gravity deck model for thinner sludges Sludge
- Open frame design allows for maximum access for normal maintenance
- Adjustable wedge dewatering zone for process optimization
- Pre-installed hydraulic system for automatic belt tensioning and steering
- Radial grid and perforated roller to accelerate dewatering

### 5.4. Mechanical Requirements

### 5.4.1. Sludge Conditioning System

Sludge/polymer mixer valve

- Variable orifice, in-line polymer mixer that combines polymer and sludge instantly.
- Optimizes polymer effectiveness and minimized polymer consumption.

### 5.4.2. Gravity Drainage Section

Even sludge distribution prior to a two-stage high efficiency gravity drainage areas fitted with easy to operate and maintain sludge ploughs and provisely arranged support grid to optimize filtrate r

precisely arranged support grid to optimize filtrate removal.

### 5.4.3. Pressure Section

- Adjustable wedge dewatering zone
  - $\circ$   $\;$  Initiates application of pressure to the dewatering process.

Classified by Alfa Laval as: Business

• Adjustable during operation.









- Radial pressure dewatering zone
  - Radial grid and perforated roller to prevent pressure-shock of sludge in the pressure zone.
- Full pressure dewatering zone
  - Optional number of pressure rollers depending on dewatering requirements.
  - Belt wrap of 180 degrees or greater maximizing cake dry solids

### 5.4.4. Roller design

- Specialized forged end construction.
- Rubber coated drive roller and thermoplastic nylon coated pressure rollers for corrosion resistance.

### 5.4.5. Bearings

- Bearings with triple labyrinth seal and specially designed shaft mounted splash guards.
- Extended lubrication cycle (every 6 months).

### 5.4.6. Belt Drive

• Input power to the drive roller shaft shall be supplied through an A.C., variable frequency drive unit. Speed shall be controlled through cyclical variation in motor current, which is operator set at the control panel. The drive roller speed reduction is obtained through a helical gear reducer.

### 5.4.7. Belt Alignment System

- Autosensing, hydraulic steering system.
- Continuous, smooth guidance control without the need for operator intervention.

### 5.4.8. Belt Tensioning System

- Pre-installed and press-mounted to minimize on site installation requirements.
- Hydraulically controlled and adjustable for continuous operation, reduced belt wear and optimum performance for a prolonged belt life.











### 5.4.9. Belt Wash System

- Efficient and continuous washing of top and bottom belts.
- Split spray bar option for easy removal and maintenance in rooms with limited space

### 5.4.10. Safety Features

• E-Stop: Trip Cord





## 6. ELECTRICAL ASSEMBLY AND CONTROLS



## 6.1. General Considerations

- The control panel shall accept a 460 VAC, 60 hertz, 3-phase power input. A main disconnect circuit breaker and operator mechanism shall be included. When the disconnect is in the open position, all power shall be removed from the control system. IEC rated motor starters shall be provided for the hydraulic unit and washwater pump. A VFD will be supplied for the belt drive. A control power transformer shall be included that will provide 120 VAC control power to the system. All logic functions for the system shall be performed by an industrial programmable logic controller (PLC) located in the control panel.
- Located on the front of the control panel shall be a CONTROL POWER OFF/ON switch. When in the ON position, the CONTROL POWER ON pilot light will be illuminated and control power shall be distributed to the control system. When in the OFF position, the control system shall be held de energized. Also located on the control panel shall be an EMERGENCY STOP pushbutton. It shall be an illuminated mushroom head style pushbutton that when depressed shall immediately de energize all moving equipment in the system. An alarm horn shall be included for audible alarm annunciation.

## 6.2. Programmable Logic Controller (PLC)

• The PLC shall be a modular type with discrete and analog capabilities. The CPU shall have 4K minimum RAM for user instructions. The unit shall have battery backed RAM and EEPROM backup. The PLC shall be an Allen Bradley CompactLogix or equal.

#### 6.3. Variable Frequency Drive (VFD)

• The VFD shall be UL listed and shall be Allen Bradley Powerflex type or approved equal.

## 6.4. System Operation

- As a minimum, the following control pilot devices or functionality shall be located on the front of the control panel or available via an HMI screen:
  - HAND/OFF/AUTO MODE selector switch
  - HAND MODE indicator
  - o AUTO MODE indicator
  - AUTO START pushbutton
  - AUTO STOP pushbutton
  - SYSTEM RESET pushbutton
  - ALARM SILENCE pushbutton
  - LAMP TEST pushbutton
  - PRESS READY indicator
  - o DEWATERING OFF/ON selector switch
  - WASHDOWN CYCLE ON indicator
  - BELT INSTALLATION OFF/ON selector switch



- WASHWATER PUMP START pushbutton
- WASHWATER PUMP STOP pushbutton
- WASHWATER PUMP RUNNING indicator
- HYDRAULIC PUMP START pushbutton
- HYDRAULIC PUMP STOP pushbutton
- HYDRAULIC PUMP RUNNING indicator
- o BELT DRIVE START pushbutton
- BELT DRIVE STOP pushbutton
- BELT DRIVE RUNNING indicator
- BELT DRIVE SPEED controller (0-100%)
- CONVEYOR START pushbutton
- CONVEYOR STOP pushbutton
- CONVEYOR RUNNING indicator
- SLUDGE PUMP START pushbutton
- SLUDGE PUMP STOP pushbutton
- o SLUDGE PUMP RUNNING indicator
- SLUDGE PUMP SPEED controller (0-100%)
- POLYMER PUMP START pushbutton
- POLYMER PUMP STOP pushbutton
- POLYMER PUMP RUNNING indicator
- POLYMER PUMP SPEED controller (0-100%)
- o LOW WASHWATER PRESSURE indicator
- LOW HYDRAULIC PRESSURE indicator
- BELT MISALIGNED indicator
- o BELT BROKEN indicator
- NO CAKE indicator
- EMERGENCY STOPPED indicator
- o BELT DRIVE FAIL indicator

## 7. SERVICE

## 7.1. 360° Service Portfolio

Alfa Laval partners with you for the entire life cycle of your equipment – from start-up, through operation, monitoring and maintenance, all the way to reconditioning and eventual redesign. Our goal is to ensure that our equipment continuously gives you optimized process performance.



## 7.2. Alfa Laval Service Centers:

You can trust Alfa Laval service technicians to maintain your equipment in peak performance and minimize the risk of unscheduled production stops. Our local service centers are equipped with the tools and expertise to improve the performance of your rotating drum thickeners. Join us on a virtual tour of our state-of-the-art facilities.

Alfa Laval - Houston service center



## 7.3. Commissioning

Alfa Laval specialists commission equipment to ensure optimal performance. Services consist of installation review, performance checks, process optimization and operator training. The commissioning process ends with a handover or acceptance certificate and is often the first day of warranty.

Services consist of:

- installation review
- performance checks
- process optimization
- operator training



The commissioning process ends with a handover or acceptance certificate and is often the first day of warranty.

The commissioning:

- Enables trouble-free start-up and process fine-tuning.
- Advice on optimizing process conditions.
- Checks on surrounding components, systems and controls and optimization recommendations.
- Help to reduce maintenance costs with a customized proposal to optimize maintenance.

## 7.4. Preventive Maintenance

Highly experienced Alfa Laval specialists can formulate and implement an optimal maintenance plan for your equipment.

Service intervals are determined by various factors, including type of application as well as the usage and condition of the equipment.

The service can be performed on site or in the Alfa Laval Service Center located in Houston, Texas.

The preventive maintenance:

- Delivers peace of mind and operational reliability
- Secures maximum throughput
- Increases overall equipment lifetime and provides good cost control
- Maintains safe equipment operation

## 7.5. <u>Rebuilds and upgrades</u>

#### 7.5.1. Repair

Alfa Laval specialists repair the equipment according to your needs, replacing unsafe or worn parts as required, and then reassemble the equipment.

- Minimizes downtime
- Maximizes production performance
- Extends the lifetime of equipment
- Prevents equipment from consequential damage and accidents

#### 7.5.2. Equipment Upgrades

- There is a wide range of upgrade solutions available to ensure your Alfa Laval equipment features the latest technical developments.
- As operating conditions change over time, new challenges can call for a review of the current installations.
- Equipment Upgrades can also include control upgrades that improve equipment automation.



## 8. TERMS AND CONDITIONS OF SALES

These Terms and Conditions of Sale ("Terms and Conditions") apply to all quotations, orders, and contracts for Alfa Laval Inc. products (hereafter "Equipment") and associated services ("Services") as used in these Terms and Conditions, the word "Equipment" includes all hardware, parts, components, software, and options.

1. **ACCEPTANCE**: Our sale to you is limited to and expressly made conditional on your assent to these Terms and Conditions and, if applicable, on the attendant quotation, both of which form a part of the contract between us and which supersede and reject all prior agreements, representations, discussions, or negotiations, whether written or oral, with respect to this sale and any conflicting terms and conditions of yours, whether signed by you. Any terms and conditions contained in your purchase order or request for quotation or other form which are different from, in addition to, or vary from these Terms and Conditions are expressly rejected, shall not be binding upon us, and are void and of no force or effect. These Terms and Conditions may not be changed except by the written agreement of both parties.

2. **PRICES**: Unless otherwise specified in writing, all quoted prices are in U.S. Dollars and are firm for thirty (30) days from the date of offer. Prices quoted are exclusive of taxes, freight and insurance, and you agree to pay any and all sales, revenue, excise or other taxes (exclusive of taxes based on our net income) applicable to the purchase of Equipment. If you claim an exemption from any such taxes, you shall provide us with a tax exemption certificate acceptable to the taxing authorities.

3. DELIVERY; FORCE MAJEURE: Dates for the furnishing of Services and/or delivery or shipment of Equipment are approximate only and are subject to change. Quoted lead times are figured from the date of receipt of complete technical data and approved drawings as such may be necessary. We shall not be liable, directly, or indirectly, for any delay in delivery or failure to deliver caused by carriers or by labor difficulties, shortages, strikes or stoppages of any sort, or difficulties in obtaining materials from ordinary sources and suppliers. In addition, we shall not be liable for any such delays or for any failure to perform our obligations under an order or contract due to any one or more of the following events, whether foreseeable or not: war, hostilities, military operations, terrorism, riots, disorder, accidents, floods, storms, natural disasters, fires, acts of God, epidemics and/or pandemics (and specifically in relation hereto and notwithstanding anything else stated herein, whether or not outbreak of such epidemic or pandemic has occurred prior to acceptance of this order or execution of a contract for the Services), governmental, judicial or administrative decisions, decrees or orders, embargoes or blockades, or any causes beyond our reasonable control. Unless otherwise specifically agreed in writing by us, in no event shall we be liable for any damages or penalties whatsoever, or however designated, resulting from our failure to perform or delay in performing due to any of the causes specified in this paragraph 3.

4. **SHIPMENT, RISK OF LOSS, TITLE**: All sales are made F.O.B. Alfa Laval shipping point, unless otherwise noted. Duty, brokerage fees, insurance, packing and handling as applicable are not included unless otherwise noted. Our liability for delivery ceases upon making delivery of Equipment to the carrier at the shipping point in good condition. The carrier shall be your agent. Risk of loss shall pass to you upon such delivery. Regardless of the delivery term specified, we shall retain title to the Equipment until final payment thereof has been made.

5. **CREDIT AND PAYMENT**: Payment terms are (30) days net, unless agreed otherwise by us in writing. *Pro rata* payments shall become due with partial shipments. Any discount period which may be granted by us begins on the invoice date and all payments are due 30 days after the invoice date. All payments shall be made without deduction, deferment, set-off, lien or counterclaim of any nature.



All amounts due not paid within 30 days after the date such amounts are due and payable shall bear interest at the lesser of 1.5 percent per month or the maximum rate of interest allowed by law. We reserve the right at any time to suspend credit or to change credit terms provided herein, when, in our sole opinion, your financial condition so warrants. Failure to pay invoices when such invoices are due and payable, at our election, shall make all subsequent invoices immediately due and payable irrespective of terms, and we may withhold all subsequent deliveries until the full account is settled. We shall not, in such event, be liable for delay of performance or nonperformance of contract in whole or in part subsequent to such event.

6. **SECURITY AGREEMENT:** You hereby grant us a security interest in the Equipment, including a purchase money security interest, and in such materials, proceeds and accessories thereof, to secure payment of the purchase price of the Equipment. You authorize us to file or record a purchase order or copy thereof or any UCC financing statement

showing our interest in the Equipment in all jurisdictions where we may determine filing to be appropriate, and you agree to sign all such documents reasonably related thereto promptly following our request. You will not encumber the Equipment with any mortgage, lien, pledge or other attachment prior to payment in full of the price therefor.

7. **CANCELLATIONS AND CHANGES**: Orders which have been accepted by us are not subject to cancellation or changes in specification except upon prior written agreement by us and upon terms that will indemnify us against all losses resulting from or arising out of such cancellation or change in specifications. In the absence of such indemnification, we shall be entitled to recover all damages and costs of whatever nature permitted by the Uniform Commercial Code.

8. **DEFERRED SHIPMENT**: If shipment is deferred at your request, payment of the contract price shall become due when you are notified that the Equipment is ready for shipment. If you fail to make payment or furnish shipping instructions, we may either extend the time for so doing or cancel the contract. In case of deferred shipment at your request, storage and other reasonable expenses attributable to such delay shall be payable by you.

## 9. EQUIPMENT WARRANTY AND REMEDY:

(a) For new Equipment only, we warrant to you that the Equipment that is the subject of this sale is free from defects in design (provided that we have design responsibility), material and workmanship. The duration of this warranty is twelve (12) months from start-up or eighteen (18) months from delivery to you, whichever occurs first (the "Warranty Period"). If you discover within the Warranty Period a defect in design, material, or workmanship, you must promptly notify us in writing. Within a reasonable time after such notification, we shall repair, replace, or, at our option, refund you the price of the defective Equipment or part thereof.

(b) For repairs, parts and Services provided by us, we warrant to you that the repairs, parts and Services we provide to you will be free from defects in material and workmanship. The duration of this warranty is ninety (90) days from as applicable (i) the date the Equipment which required the repairs, parts or Services is returned to you by us, (ii) the date of your receipt of the part, or (iii) the date of completion of the repair or other Services, if performed at your facility. If during this ninety-day period you discover a defect in the repairs, parts or Services you must promptly notify us in writing, and we shall correct such defect with either new or used replacement parts or reperform the Services as applicable. If we are unable to correct the defect after a reasonable number of attempts, we will provide a refund of the price paid for the defective repair, parts or Services.

(c) All warranty service is subject to our prior examination and approval and will be performed by us at your facility or at service centers designated by us. All transportation to and from the designated service center will be at our expense. The remedies set forth above are your exclusive remedies for breach of warranty. Unless otherwise agreed in writing by us, our warranty extends only to you and



is not assignable to or assumable by any subsequent purchaser, in whole or in part, and any such attempted transfer shall render all warranties provided hereunder null and void and of no further force or effect.

(d) The warranties set forth above are inapplicable to and exclude any product, components or parts not manufactured by us or covered by the warranty of another manufacturer. We shall have no responsibility for defects, loss or damage to the extent caused by (i) normal wear and tear, (ii) your failure to follow all installation and operation instructions or manuals or to provide normal maintenance, (iii) repairs or modifications by you or by others not under our direct supervision, or (iv) a product or component part which we did not design, manufacture, supply, or repair.

(e) **DISCLAIMER OF IMPLIED WARRANTIES**. THE WARRANTIES SET FORTH ABOVE AND IN SECTION 12 BELOW ARE IN LIEU OF ALL OTHER WARRANTIES, EXPRESS OR IMPLIED, INCLUDING BUT NOT LIMITED TO ANY IMPLIED WARRANTY OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE.

10. LIMITATION OF LIABILITY: In no event shall we be liable, and you hereby waive any claims against us and release us from liability to you, for any indirect, special, punitive, incidental, or consequential damages whatsoever based upon breach of warranty, breach of contract, negligence, strict tort, or any other legal theory. In no circumstance, shall we be liable for, however such damages are characterized, loss of profits, loss of savings or revenue, loss of use of the Equipment or any associated equipment, cost of capital, cost of any substitute Equipment, facilities or services, downtime, or loss of prospective economic advantage. OUR AGGREGATE LIABILITY FOR FAILURE TO PERFORM, BREACH OF WARRANTY OR BREACH OF OTHER CONTRACTUAL OBLIGATIONS SHALL NOT EXCEED THE TOTAL PRICE PAID TO US FOR THE EQUIPMENT AND SERVICES THAT ARE THE SUBJECT OF ANY CLAIM BY YOU.

11. **OWNERSHIP:** All drawings, designs, specifications, data and other proprietary rights supplied by us (including without limitation in connection with the Equipment) have been prepared or assembled by us and are (and shall remain) exclusively our property, and upon our request you agree to execute any additional documents needed to give effect to the foregoing. Such drawings, designs and specifications have been furnished in order to provide full documentation and on the condition that they shall not be disclosed, reproduced or copied in any manner whatsoever, in whole or in part, except for your internal use as necessary, and upon the further condition that, as our sole property, they shall not be used for furnishing information and/or disclosed, in whole or in part, to others or otherwise for any purpose not specifically authorized in a writing signed by one of our corporate officers.

## 12. PATENT INFRINGEMENT

(a) We make no express or implied warranties of non-infringement with respect to the Equipment. We will, however, defend, indemnify and hold you harmless from any third party apparatus claims based upon an issued U.S. patent to the extent such claim relates to the Equipment supplied and sold to you; provided, however, that we undertake no indemnification in respect of third-party rights (i) where the alleged patent infringement is based upon or related to any method, process or design claims in third-party U.S. patents, any combination of the Equipment with other equipment not supplied by us, or any modifications of the Equipment made by you and not approved by us, or (ii) to the extent the alleged infringement is directly attributable to the negligence or intentional misconduct of you or otherwise for which you are obligated to indemnify us for under paragraph 12(c).

(b) We shall assume defense of a claim at our expense in accordance with these Terms and Conditions, provided you shall notify us within 30 days of your receipt of notice of an alleged thirdparty claim that you believe would entitle you to patent infringement indemnification pursuant to paragraph 12(a). You acknowledge and agree that we shall have the sole right to settle or otherwise



compromise such a third-party claim, including but not limited to the right to either (i) modify the Equipment to avoid infringement if you are agreeable to the modification, (ii) repurchase the Equipment from you at a price equal to the then-current fair market value of the Equipment, or (iii) secure rights by assignment or license to permit continued use of the Equipment.

(c) If a third-party charges us with patent infringement relating to Equipment sold by us to you, we shall have the right to either (i) modify the Equipment to avoid infringement if you are agreeable to the modification, (ii) repurchase the Equipment from you at a price equal to the then-current fair market value of the Equipment, or (iii) secure rights by assignment or license to permit continued use of the Equipment. If a third party charges us with patent infringement on the bases set forth in paragraph 12(a)(i) or (ii), you shall indemnify and hold us harmless for all expenses as well as any awards of damage assessed against us, and, without limiting any of our other rights and remedies available at law or in equity, we shall also have the right to modify or repurchase the Equipment or to secure rights for continued use by way of assignment or license as set forth in this paragraph.

13. **INSPECTION**: Upon prior written notice, you may make reasonable inspections of Equipment at our facility. We reserve the right to determine the reasonableness of the request and to select an appropriate time and location for such inspection. You agree to execute appropriate confidentiality provisions upon our request prior to visiting our facility. All costs of inspection shall be solely determined by us and shall be payable by you. No inspection or expediting by you at the facilities of our suppliers is authorized.

14. **SOFTWARE PROVISIONS**: If software is provided hereunder (whether such is integrated into the Equipment or otherwise operates alongside the same), you are hereby granted a non-exclusive, non-sublicensable, non-transferable, royalty free license to access and use such software as provided and as intended with our Equipment. Without limiting the foregoing, under the foregoing license you may specifically: (i) use our software in machine readable object code only and only with the Equipment provided; (ii) copy our software into any machine-readable object code form solely for back up purposes in support of your use of our software on the Equipment provided in accordance with these Terms and Conditions; and (iii) create one additional copy of the software for archival purposes only. This license may only be assigned, sublicensed, or otherwise transferred by you with our prior written consent. You hereby recognize and acknowledge that the software provided to you hereunder comprises valuable trade secret and/or copyright property of Alfa Laval (or its licensors) and you covenant that you will take adequate precautions against access to the software by, or disclosure of the software to, anyone not authorized hereunder to use or have access to the software as contemplated herein. The software is subject to the confidentiality obligations set forth below in paragraph 15.

15. **CONFIDENTIALITY:** Subject to any non-disclosure or confidentiality agreement already in effect between us, any drawings, data, software or other information exchanged between us is proprietary or confidential to us and shall not be used or disclosed by you without our prior written consent. Confidential information shall not be any information that (i) is known previously to you under no obligation of secrecy; (ii) becomes known to the public through no breach of an obligation of secrecy by you; or (iii) is independently developed by you without use or reference to any of the confidential information or materials provided to you by us.

16. **INAPPLICABILITY OF CISG:** The parties specifically agree that the United Nations Convention on Contracts for the International Sale of Goods shall not apply to any sale or order or the contract between us.



17. **GOVERNING LAW & VENUE**: These Terms and Conditions and any dispute or claim arising out of or related to an order or the contract between us shall be finally decided in accordance with the laws of the Commonwealth of Virginia, without giving effect to the provisions thereof relating to conflict of laws. You agree that the venue for any such dispute shall lie in the United States District Court for the Eastern District of Virginia, Richmond Division. In the event that federal jurisdiction cannot be established pursuant to 28 U.S.C. §§ 1331 or 1332, the venue for any such dispute shall lie in the Circuit Court of Henrico County, Virginia. You expressly submit and waive any objection to the sole and exclusive jurisdiction of such courts.

18. **GENERAL:** All previous agreements or understandings between us, either oral or written, with regard to the subject order, with the exception of a pre-existing non-disclosure agreement between us, are void and these Terms and Conditions constitute the entire agreement between us with respect to the matters addressed herein. Neither of us shall assign an order or contract to which these Terms and Conditions apply without the prior written consent of the other party, which consent shall not be unreasonably withheld. If any provision of these Terms and Conditions is held to be invalid or unenforceable, such holding shall not affect the validity or enforceability of any other provision herein. No waiver by either of us of any default or breach by the other party will operate as or be deemed a waiver of any subsequent default or breach.



# Alfa Laval AS-H Belt Press KPZ

## Sludge dewatering machine

#### Introduction

The Alfa Laval AS-H Belt Press KPZ is considered the industry standard for superior value, performance and durability for sludge dewatering.

The belt press KPZ is designed for low polymer consumption, high throughput rates, and high cake dry solids content and is available in a wide size range and extensive modular options to meet individual process requirements.

#### Application

The Alfa Laval AS-H Belt Press KPZ is a sludge dewatering machine suitable for all municipal wastewater sludge types and a wide variety of industrial solid / liquid separation applications, such as paper, petrochemical, mineral, food processing, pharmaceutical and chemical. The belt press KPZ incorporates variable energy mixing, flocculation, gravity drainage, adjustable wedge, extended pressure filtration area, and offers the versatility of a wide size range and extensive modular options to meet individual process requirements.

#### Benefits

- Thorough uniform mixing of polymer into sludge
- Higher volumetric throughput and solids loading
- Highest cake dry solids in class
- Low power consumption
- Low polymer usage
- High quality filtrate
- Low maintenance requirements
- Long life design
- Operator level gravity deck
- Elevated discharge height for ease of cake handling
- Reduced civil construction costs

#### Scope of supply

The sludge dewatering system will consist of an independently operating belt thickener and a belt press and all appurtenances. Each belt press will be a complete assembly consisting of a sludge conditioning system, gravity drainage section, vertical pressure section, a belt alignment and tensioning system and a belt washing system.



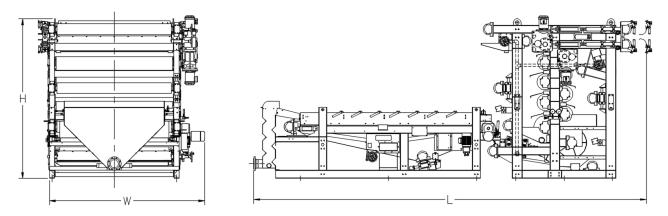
#### Working principle

Its operating principle is to condition the feed sludge with a polyelectrolyte and drain the flocculated sludge over an endless, horizontal porous filter belt. The thickened sludge is subjected to gradually increasing pressure between a pair of belts that pass through the adjustable wedge and roll grid zone before being further dewatered by a series of decreasing diameter rollers. Final moisture removal is achieved by shear rollers arranged to give minimum 180 degree belt wrap at up to 70 pli in order to optimize dewatering. Independent belts for each of the separate thickening and dewatering zones allow for optimized process control.

#### Options

- Clad 316SS rollers
- Stainless bearing housings
- Cascading washdown, self cleaning
- Paddle feed distribution

#### Dimensions



## Available in 8 roller and 10 roller designs in the vertical pressure section

#### 10 roller version

Model	Length		Width		Height	
	(mm)	(inches)	(mm)	(inches)	(mm)	(inches)
Belt Press KPZ 100	7,620	300	2,362	93	3,225	127
Belt Press KPZ 150	7,620	300	2,870	113	3,225	127
Belt Press KPZ 200	7,620	300	3,378	133	3,225	127

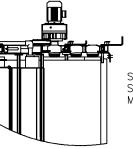
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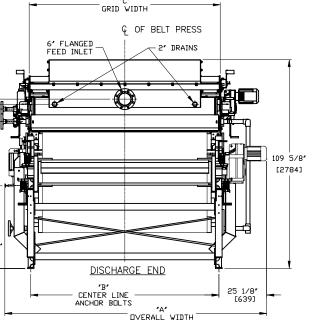
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How to contact Alfa Laval Up-to-date Alfa Laval contact details for all countries are always available on our website at www.alfalaval.com

MACHINE	SIZF	DIMENSION "A"	DIMENSION "B"	DIMENSION "C"	DIMENSION "	D"	DIMENSION "E"	DIMENSION "F"	ESTIMATED WT.					
> 1.5 MET		117 3/8" (2981)	78 7/8" (1978)	80 3/8" (2042)	67" (1700)		60 1/4" (1530)	26 3/4" (680)	19,000 lb	-				
2.0 MET		137 3/8" (3490)	98 7/8" (2511)	100 3/8" (2550)	86 5/8" (22		80 1/4" (2038)	36 7/8" (937)	22,500 lb	-				
2.5 MET	ER	159 3/8" (4049)	120 7/8" (3070)	122 3/8" (3118)	106 3/8"(2	2700)	102 1/4" (2600)	46"(1168)	26,500 lb					
2.5 MEI	<u>FK</u>	<u>159 3/8" (4049)</u>	'D' BELT VI FRONT	THICKENER BELT DRIVE MOTOR					OPTIONAL DUA				SINGLE DRIVE STANDARD ON 8 ROLL MACHINE.	
		CHUTE SI	THICKE DIVER PF WITH REVERSAL HOWN HERE. PRESS L PRESS LOWER BELT BREAKAGE SWITCH PF	HYDRAULIC MAN THICKENER RAMP ASSEMBLY PRESS UPPER BELT TENSIDNING HICKENER SCRAPER BLADE HANDLE NER/PRESS TER CHUTE RESS UPPER BELT BREAKAGE SWITCH UVER BELT TENSIDNING T 27 3/4 (706) T 27 3/4 (706) T RESS LDWER AY SHOWERS D WASH BDX	KENER SPRAY SHOW TENSIONING VALVI NIFOLD, AND GAUGE		HYDRAULIC PUMP- MITOR ASSEMBLY		AND TEF	NSIDNING ING LS AND EMERGENCY STOP BUTTON MINATED IN JUNCTION BOX PRESS UPPER SPRAY SHOWER AND WASH BOX LIFTING LUG 46 7/8' PRESS UPP BELT STEE PRESS UPP BELT STEE PRESS UPP BELT SCRA BLADE 99 3/8' [2524] 6 1/8' [1054] 16 5/8' [422] 6 1/8' [156]	PER RING PER 88 7/8' [2258] 86 1/4' [2124] 83 5/8' [2124] 83 5/8' [1959] IWER RAPER	6' FLANGED FEED INLET		
		<u> </u>					8 ROLLER CONFIGUE				IDRAWN IIIAIR	NAME		
		UNLESS NOTED	V DATE	REVISIONS	BYA	PP'D REV I	DATE	REVISIONS DESCRIPTION	BY	APP'D THIS DRAWING HAS NOT BEEN	MM 9/8/06 CHECKED DATE 0.00	NOTE: 1)DEBURR ALL SHARP	ALFA LAVAL INC 10470 Deer Trail Drive Phone: 281-	449-0322
FRACTION	INCHES $+/-1/3$			ARGE DIMENSIONS CORRECTE						APP'D THIS DRAWING HAS NOT BEEN PUBLISHED AND IS THE SOLE PROPERTY OF ALFA LAVAL INC IS LENT TO THE BORROWER FOR HIS CONFIDENTIAL USE ONLY. IN CONFIDENTIAL USE ONLY. IN	MM 9/8/06 APPROVED DATE 9/8/06 NEXT ASSY WEIGHT SEE TADLE	EDGES. 2)MARK WITH PART	Houston, Texas 77038 FAX: 281-	449-1324
X.	+/-0.100			DED 2.5m DIMENSIONS	MCA MCA N					THE BORROWER PROMISES TO	NEXT ASSY WEIGHT N/A SEE TABLE	NUMBER PER WORK OR PURCHASE ORDER	GENERAL ARRANGEMENT KLAMPRESS, 3 BELT OPTIC	
X.X	+/-0.030		DIATED	HYDRAULIC PUMP LOCATION	MCA N	1CA 9 10				RETURN IT UPON REQUEST AND AGREES THAT IT SHALL NOT BE REPRODUCED, COPIED, LENT, OR			1.5, 2.0, 2.5 METER, 8 ROLL C	
X.XX	+/-0.015		5			11				OTHERWISE DISPOSED OF.		LAVAL	SCALE 1/50 DWG. NO.	REV
x.xxx	+/-0.005		5			12				DIRECTLY OR INDIRECTLY, NOR USED FOR ANY PURPOSE OTHER THAN THAT FOR WHICH IT IS SPECIFICALLY FURNISHED.			CUSTOMER N/A SK003117	3
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Item 3: Sequence Batch Reactor



# **Process Design Report**

## **CENTREVILLE WWTP MD**

Design# 171293 Option: Preliminary SBR Design





May 03, 2023 Designed By: Xu Ye

6306 N. Alpine Rd Loves Park, IL 61111 (815) 654-2501 <u>www.aqua-aerobic.com</u>

## **Design Notes**

Project: CENTREVILLE WWTP MD

Option: Preliminary SBR Design

Designed by Xu Ye on Wednesday, May 3, 2023

## Design#: 171293



#### Upstream Recommendations

- Neutralization is required ahead of the biological system if the pH is expected to fall outside of 6.5-8.5 for significant durations.

- Coarse screening and grit removal is recommended (by others) ahead of the biological system.

- Elevated concentration of hydrogen sulfide can be detrimental to both civil and mechanical structures. If anaerobic conditions exist in the collection system, steps should be taken to eliminate hydrogen sulfide prior to the treatment system.

#### **Flow Considerations**

- The maximum flow, as shown on the design, has been assumed as an organic maximum that represents an increased organic load. An oxygen peaking factor of 1.5 has been included to accommodate this additional load while maintaining a residual DO concentration of 2 mg/l, which is the same approach as previous SBR design.

#### **Biological Process**

- The decanter performance is based upon a free-air discharge following the valve and immediately adjacent to the basin. Actual decanter performance depends upon the complete installation including specific liquid and piping elevations and any associated field piping losses to the final point of discharge. Modification of the high water level, low water level, centerline of discharge, and / or cycle structure may be required to achieve discharge of full batch volume based on actual site installation specifics.

#### **Aeration**

- The aeration system has been designed to provide 1.25 lbs. O2/lb. BOD5 applied and 4.6 lbs. O2/lb. TKN applied at the design average loading conditions, while maintaining a residual DO concentration of 2 mg/l.

- A common standby blower will be shared among the biological reactors.

- Depending on the actual yard piping from the blowers to the diffuser system and the heat losses associated with the yard piping, additional provisions for cooling of the air (i.e. incorporating heat exchangers) and/or modification of in-basin piping and/or diffuser sleeve material may be required. Aqua-Aerobic Systems, Inc. may need to modify the following equipment offering to ensure compatibility of all in-basin components with actual air temperatures.

#### **Digester**

- A supernatant pump has already been installed in the digester.

- The digester aeration system has been designed based on 2.0 lbs O2/lb VSS removed.

- The air supply for the digester system is based on each basin receiving 100% of the total sludge produced per day.

#### Process/Site

- The design loading parameters have been assumed to be the same as previous design loading conditions (engineer to verify).

- The anticipated effluent nitrogen requirement is predicated upon an influent waste temperature of 10 °C or greater. While lower temperatures may be acceptable for a short-term duration, nitrification and (if required) denitrification below 10 °C can be unpredictable, requiring special operator attention.

- Sufficient alkalinity is required for nitrification, as approximately 7.1 mg alkalinity (as CaCO3) is required for every mg of NH3-N nitrified. If the raw water alkalinity cannot support this consumption, while maintaining a residual concentration of 50 mg/l, supplemental alkalinity shall be provided (by others).

- This system has been designed to be expandable from a Phase I average flow of 0.5 MGD to an ultimate, Phase II average flow of 1.2 MGD. This expansion will utilize the Phase I, post-equalization basin as well as the Phase I digester.

- Phase I blowers may need to be re-belted and sheaved to meet phase II operating requirements (by others). The engineer should give thought to piping and site layout to facilitate the expansion.

## Design Notes

Project: CENTREVILLE WWTP MD

Option: Preliminary SBR Design Designed by Xu Ye on Wednesday, May 3, 2023



- The control panel for Phase I has been replaced incorporate Phase II equipment.

- Influent to the biological system is a typical municipal wastewater application. Influent TP shall be either in a particle associated form or in a reactive soluble phosphate form or in a soluble form that can be converted to reactive phosphorus in the biological system. Soluble hydrolyzable and organic phosphates are not removable by chemical precipitation with metal salts. A water quality analysis is required to determine the phosphorus speciation with respect to soluble and insoluble reactive, acid hydrolyzable and total phosphorus at the system Influent, point(s) of chemical addition, and final effluent.

- Chemical feed lines (i.e. metal salts) shall be furnished to each reactor, aerobic digester and dewatering supernatant streams as necessary.

- pH monitoring and control in a range of 6.5-8.5 of the biological reactor is required when adding metal salts.

- The average and maximum design flow and loading conditions, shown within the report, are based on maximum month average and maximum day conditions, respectively.

#### Post-Secondary Treatment

-The following processes follow the Biological process:

- Effluent flow equalization.

#### Equipment

- Changes in basin geometry may require alterations in the equipment recommendation.

- The basins are not included and shall be provided by others.

- Influent is assumed to enter the reactor above the water level, away from the decanter, and to avoid splashing or direct discharge in the immediate vicinity of other equipment. If the influent enters the basin below the water level, adequate hydraulic capacity shall be made in the headworks to prevent backflow from one reactor to the other during transition of influent.

- Based on the process requirements and selected equipment, the reactor wall height should be at least 25 ft in all basins.

- Scope of supply includes freight, installation supervision and start-up services.

- Equipment selection is based upon the use of Aqua-Aerobic Systems' standard materials of construction and electrical components, suitable for non-classified electrical environments.

- The system has been designed to fit within existing basin dimensions and add one extra basin.

- The basin dimensions reported on the design have been assumed based upon the required volumes and assumed basin geometry. Actual basin geometry may be circular, square or rectangular with construction materials including concrete or steel.

- The control panel does not include motor starters or VFDs, which should be provided in a separate MCC (by others).

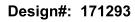
- Provisions should be made, by others, for overflows in each of the recommended basins.

- Aqua-Aerobic Systems, Inc. is familiar with various "Buy American" Acts (i.e. AIS, ARRA, Federal FAR 52.225, EXIM Bank, USAid, PA Steel Products Act, etc.). As the project develops Aqua-Aerobic Systems can work with you to ensure full compliance of our goods with various Buy American provisions if they are applicable/required for the project. When applicable, please provide us with the specifics of the project's "Buy American" provisions.

## AquaSBR® - Sequencing Batch Reactor - Design Summary

Project: CENTREVILLE WWTP MD Option: Preliminary SBR Design

Designed by Xu Ye on Wednesday, May 3, 2023





#### **DESIGN INFLUENT CONDITIONS**

Avg. Design Flow (ADF)	= 1.2 MGD = 3 MGD		= 4,542 m = 11,356 r					
Max Design Flow (MDF)	- 3 MGD		- 11,3001	n-/uay		Efflue	ent	
DESIGN PARAMETERS	Influent		mg/l		Required	<= mg/l	Anticipated	<= mg/l
Bio/Chem Oxygen Demand:	BOD	5	250		BOD5	10	BOD5	10
Total Suspended Solids:	TS	S	250		TSS	20	TSS	20
Total Kjeldahl Nitrogen:	TKI	N	40			-		
Ammonia Nitrogen:	-				NH3-N	1	NH3-N	1
Oxidized Nitrogen:	-	-			NOx-N	10	NOx-N	10
Total Phosphorus:	TI		8		TP	1	TP	1
SITE CONDITIONS	Maxim	num		Minim	um	Elevation (MSL)		
Ambient Air Temperatures:	85 F	29.4 C	;	30 F	-1.1 C	30 ft	_	
Influent Waste Temperatures:	68 F	20.0 C	į	50 F	10.0 C	9.1 m		
SBR BASIN DESIGN VALUES	2		Wat	ter Der	oth	в	asin Vol./Basin	

SBR BASIN DESIGN VALUES		vva	water Depth			Basin voi./Basin		
No./Basin Geometry:	= 3 Rectangı	ular Basin(s)	Min (LWL)	= 13.9 ft	= (4.2 m)	Min (VIwl)	= 0.39 MG	= (1,475.3 m³)
Freeboard:	= 2.0 ft	= (0.6 m)	Avg (AWL)	= 16.7 ft	= (5.1 m)	Avg (Vawl)	= 0.47 MG	= (1,778.1 m <sup>3</sup> )
Length of Basin:	= 70.5 ft	= (21.5 m)	Max (HWL)	= 21.0 ft	= (6.4 m)	Max (Vhwl)	= 0.59 MG	= (2,232.4 m <sup>3</sup> )
Width of Basin:	= 53.3 ft	= (16.2 m)						

Number of Cycles:	= 5 per day/basin (advances cycles beyond MDF)	
Cycle Duration:	= 4.8 hr/cycle	
Food/Mass (F/M) ratio:	= 0.064 lbs. BOD5/lb. MLSS-Day	
MLSS Concentration:	= 4,000 mg/l @ LWL	
Hydraulic Retention Time:	= 1.174 days @ AWL	
Solids Retention Time:	= 17.9 days	
Est. Net Sludge Yield:	= 0.793 lbs. WAS/lb. BOD5	
Est. Dry Solids Produced:	= 1,984.3 lbs. WAS/day	= (900.1 kg/day)
Est. Solids Flow Rate:	= 120 gpm (23,792 gal/day)	= (90.1 m³/day)
Decant Flow Rate @ MDF:	= 2,857 gpm (as avg. from HWL to LWL)	= (180.2 l/sec)
LWL to CenterLine Discharge:	= 1.0 ft	= (0.3 m)
Lbs. O2/lb. BOD5	= 1.25	
Lbs. O2/lb. TKN	= 4.6	
Peak O2 Factor:	= 1.5	
Actual Oxygen Required:	= 7,453 lbs./day	= (3,380.9 kg/day)
Air Flowrate/Basin:	= 1,670 SCFM	= (47.3 Sm³/min)
Max. Discharge Pressure:	= 10.7 PSIG	= (74 KPA)
Daily Max. Month Avg. Estimated Power*:	= 1,426.9 kWh/day	

\* Power consumption calculations in this document are based on maximum month conditions. Detailed power vs. loading calculations can be provided if requested.

Designed by Xu Ye on Wednesday, May 3, 2023



#### POST-SBR EQUALIZATION DESIGN PARAMETERS

Avg. Daily Flow (ADF):	= 1.2 MGD	= (4,542 m³/day)
Max. Daily Flow (MDF):	= 3 MGD	= (11,356 m³/day)
Decant Flow Rate from (Qd):	= 2,857 gpm	= (10.8 m <sup>3</sup> M)
Decant Duration (Td):	= 70 min	
Number Decants/Day:	= 15	
Time Between Start of Decants:	= 96 min	

#### POST-SBR EQUALIZATION VOLUME DETERMINATION

The volume required for equalization/storage shall be provided between the high and the low water levels of the basin(s). This Storage Volume (Vs) has been determined by the following:

Vs = [(Qd -(MDF x 694.4)] x Td = 54,157 gal = (7,240.2 ft<sup>3</sup>) = (205.0 m<sup>3</sup>)

The volumes determined in this summary reflect the minimum volumes necessary to achieve the desired results based upon the input provided to Aqua. If other hydraulic conditions exist that are not mentioned in this design summary or associated design notes, additional volume may be warranted.

Based upon liquid level inputs from each SBR reactor prior to decant, the rate of discharge from the Post-SBR Equalization basin shall be pre-determined to establish the proper number of pumps to be operated (or the correct valve position in the case of gravity flow). Level indication in the Post-SBR Equalization basin(s) shall override equipment operation.

#### POST-SBR EQUALIZATION BASIN DESIGN VALUES

No./Basin Geometry:	= 1 Rectangula	1 Rectangular Basin(s)						
Length of Basin:	= 52.8 ft	= (16.1 m)						
Width of Basin:	= 33.0 ft	= (10.1 m)						
Min. Water Depth:	= 1.5 ft	= (0.5 m)	Min. Basin Vol. Basin:	= 19,560.8 gal	= (74.1 m³)			
Max. Water Depth:	= 5.7 ft	= (1.7 m)	Max. Basin Vol. Basin:	= 73,717.5 gal	= (279.1 m <sup>3</sup> )			

#### POST-SBR EQUALIZATION EQUIPMENT CRITERIA

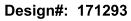
Mixing Energy with Diffusers:	= 0.1 SCFM/ft <sup>2</sup> of reactor	
SCFM Required to Mix:	= 209 SCFM/basin	= (355 Nm³/hr/basin)
Max. Discharge Pressure:	= 4.0 PSIG	= (27.67 KPA)
Avg. Power Required:	= 90.4 kW-hr/day	

## Aerobic Digester - Design Summary

Project: CENTREVILLE WWTP MD

Option: Preliminary SBR Design

Designed by Xu Ye on Wednesday, May 3, 2023





#### **AEROBIC DIGESTER DESIGN PARAMETERS**

Sludge Flowrate to the Digester	= 23,790.0 gal/day	= (90.1 m³/day)
Inlet Sludge Concentration	= 1.00%	
Solids Loading to the Digester	= 1,984.1 lb/day	= (900.0 kg/day)
Inlet Volatile Solids Fraction	= 73.4%	

#### AEROBIC DIGESTER BASIN DESIGN VALUES

No./Basin Geometry:	= 1 Rectangula	= 1 Rectangular Basin(s)						
Length of Basin:	= 52.8 ft	= (16.1 m)						
Width of Basin:	= 36.7 ft	= (11.2 m)						
Min. Water Depth:	= 14.7 ft	= (4.5 m)	Min. Basin Vol. Basin:	= 212,957.1 gal	= (806.2 m <sup>3</sup> )			
Max. Water Depth:	= 21 ft	= (6.4 m)	Max. Basin Vol. Basin:	= 304,224.4 gal	= (1,151.7 m³)			

#### AEROBIC DIGESTER PROCESS DESIGN PARAMETERS

Solids Retention Time:	= 25.6 days	
Digester Design Temperature:	= 20 C	
Volatile Solids Destruction:	= 40%	
Digester Solids Concentration:	= 2%	
Oxygen Supplied for Digestion:	= 2 lbs O2 per lb VSS Destroyed	
Oxygen Distribution Per Basin:	= 100.0%	
Actual Oxygen Required:	= 1,165.1 lb/day	= (528.5 kg/day)
Volatile Percentage After Digestion:	= 62.3%	
Estimated Dry Solids to be Removed:	= 1,401.6 lb/day	= (635.8 kg/day)
Volume of Solids to be Removed:	= 8,402.6 gal/day	= (31.81 m³/day)
Estimated Supernatant Volume:	= 91,267.3 gal/basin	= (345.48 m³/basin)
Assumed Supernatant Duration:	= 180 minutes	
Calculated Supernatant Flow:	= 507.0 gpm	= (32.0 l/sec)

1. The Volatile Solids Destruction listed above shall be used for determination of the oxygen demand during summer conditions. It should be noted that the actual VSS destruction will be dependent upon digester inlet condition, temperature, and operating conditions.

2. The Digester Solids Concentration is reflected as an average concentration, assuming the operations include frequent settling and supernating practices.

#### AEROBIC DIGESTER EQUALIZATION EQUIPMENT CRITERIA

Mixing Energy with Aerators:	= 140 HP/MG	= (27.58 W/m³)
NPHP Provided:	= 2.0	= (1.5 kW)
Mixing Energy with DDMs	= 40 HP/MG	= (7.88 W/m <sup>3</sup> )
NPHP Provided:	= 10	= (7.5 kW)
Max. Flow Rate Required Basin:	= 120 gpm	= (0.454 m³/min)
Avg. Power Required:	= 198.24 kW-hr/day	

Project: CENTREVILLE WWTP MD

Option: Preliminary SBR Design

Designed by Xu Ye on Wednesday, May 3, 2023

#### AquaSBR

#### Influent Valves

- 1 Influent Valve(s) will be provided as follows:
  - 16 inch DeZurik electrically operated Plug Valve(s).

#### <u>Mixers</u>

- 1 AquaDDM Direct Drive Mixer(s) will be provided as follows:
  - 20 HP Aqua-Aerobic Sstems Endura Series Model FSS DDM Mixer(s).

#### Mixer Mooring

#### 3 Mixer pivotal mooring assembly(ies) consisting of:

- 304 stainless steel pivotal mooring arm(s).
- #8 AWG four-conductor electrical service cable(s).
- Electrical cable strain relief grip(s), 2 eye, wire mesh.

#### **Decanters**

#### 1 Decanter assembly(ies) consisting of:

- 8x7 Aqua-Aerobics decanter(s) with fiberglass float, 304 stainless steel weir, galvanized restrained mooring frame, and painted steel power section with #14-10 conductor power cable.
- Decant pipe(s).
- 4" schedule 40 galvanized steel mooring post.
- Galvanized steel dewatering support post(s).
- 14 inch DeZurik electrically operated butterfly valve(s) with Limitorque actuator.

#### Transfer Pumps/Valves

#### 1 Submersible pump assembly(ies) consisting of the following items:

- 2.4 HP Submersible Pump(s) with painted cast iron pump housing, discharge elbow, and multi-conductor electrical cable.
- 3 inch diameter swing check valve.
- Upper guide bar bracket(s).
- 304 stainless steel guide bar(s).

#### **Retrievable Fine Bubble Diffusers**

#### 8 Retrievable Fine Bubble Diffuser Assembly(ies) consisting of:

- 25 diffuser tubes consisting of two flexible EPDM porous membrane sheaths mounted on a rigid support pipe with 304 stainless steel band clamps.

- 304 stainless steel manifold weldment.
- 304 stainless steel leveling angles.
- 304 stainless steel leveling studs.
- Galvanized vertical support beam.
- Galvanized vertical air column assembly.
- Galvanized upper vertical beam and pulley assembly.
- Galvanized top support bracket.
- 3" EPDM flexible air line with stainless steel quick disconnect end fittings.
- Galvanized threaded flange.
- 3" manual isolation butterfly valve with ductile iron body, NBR seat, ductile iron disk and one-piece stainless steel shaft.
- Quick disconnect cam lock adapter.
- 304 stainless steel adhesive anchors.
- Brace angles.



Design#: 171293

Project: CENTREVILLE WWTP MD

Option: Preliminary SBR Design

Designed by Xu Ye on Wednesday, May 3, 2023

## Design#: 171293



#### Positive Displacement Blowers

4 Positive displacement Blower Package(s), with each package consisting of:

- Aerzen 50HP Rotary Positive Displacement Blower(s).

#### Air Valves

#### 2 Air Control Valve(s) will be provided as follows:

- 8 inch DeZurik electrically operated butterfly valve(s) with Limitorque actuator.

#### Level Sensor Assemblies

- 1 Pressure Transducer Assembly(ies) each consisting of:
  - Pressure transducer(s).
  - Mounting bracket weldment(s).
  - Transducer mounting pipe weldment(s).
- 1 Level Sensor Assembly(ies) will be provided as follows:
  - Float switch(es).
  - Float switch mounting bracket(s).
  - Stainless steel anchors.

#### Instrumentation

- 1 Dissolved Oxygen Assembly(ies) consisting of:
  - DO probe(s).
- 1 Process Controller(s) consisting of:
  - Controller and display module(s).

#### <u>Controls</u>

#### **Controls wo/Starters**

- 1 Controls Package(s) will be provided as follows:
  - NEMA 12 panel enclosure suitable for indoor installation and constructed of painted steel.
  - Fuse(s) and fuse block(s).
  - Compactlogix Processor.
  - Operator interface(s).
  - Remote access Ethernet modem(s).



Item 4: Aerobic Granular Sludge



# **Process Design Report**

## **CENTREVILLE WWTP MD**

Design# 171290 Option: Preliminary AquaNereda Retrofit Design

## AquaNereda<sup>®</sup>

Aerobic Granular Sludge Technology



May 03, 2023 Designed By: Xu Ye

6306 N. Alpine Rd Loves Park, IL 61111 (815) 654-2501 <u>www.aqua-aerobic.com</u> Nereda<sup>®</sup> is a registered U.S. trademark of Royal HaskoningDHV © 2023 Aqua-Aerobic Systems, Inc

## Design Notes

Project: CENTREVILLE WWTP MD

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023

## Design#: 171290



#### Upstream Recommendations

- For primary influent designs, ¼ inch (6 mm) perforated plate-style screening and grit removal, consisting of 95% removal at 140 mesh, is required ahead of the AquaNereda system. For primary effluent designs, screening requirements may be relaxed at the discretion of Aqua-Aerobic Systems. If alternative screening and grit removal methods are planned ahead of the AquaNereda system, please discuss screening with Aqua-Aerobic Systems to understand the impacts of the approach.

- Neutralization is required ahead of the biological system if the pH is expected to fall outside of 6.5-8.5 for significant durations.

- Elevated concentration of hydrogen sulfide can be detrimental to both civil and mechanical structures. If anaerobic conditions exist in the collection system, steps should be taken to eliminate hydrogen sulfide prior to the treatment system.

- Flow equalization is required ahead of the biological reactor(s) to provide interruption of flow during the non-fill phases.

#### Flow Considerations

- The maximum flow, as shown on the design, has been assumed as an organic maximum that represents an increased organic load. An oxygen peaking factor of 1.5 has been included to accommodate this additional load while maintaining a residual DO concentration of 2 mg/l.

#### **Aeration**

- The aeration system has been designed to provide 1.25 lbs. O2/lb. BOD5 applied and 4.6 lbs. O2/lb. TKN applied at the design average loading conditions, while maintaining a residual DO concentration of 2 mg/l.

- A common standby blower will be shared among the biological reactors.

- Depending on the actual yard piping from the blowers to the diffuser system and the heat losses associated with the yard piping, additional provisions for cooling of the air (i.e. incorporating heat exchangers) and/or modification of in-basin piping and/or diffuser sleeve material may be required. Aqua-Aerobic Systems, Inc. may need to modify the following equipment offering to ensure compatibility of all in-basin components with actual air temperatures.

#### Process/Site

- The anticipated effluent nitrogen requirement is predicated upon an influent waste temperature of 10 °C or greater. While lower temperatures may be acceptable for a short-term duration, nitrification and (if required) denitrification below 10 °C can be unpredictable, requiring special operator attention.

- Sufficient alkalinity is required for nitrification, as approximately 7.1 mg alkalinity (as CaCO3) is required for every mg of NH3-N nitrified. If the raw water alkalinity cannot support this consumption, while maintaining a residual concentration of 50 mg/l, supplemental alkalinity shall be provided (by others).

- This system has been designed to be expandable from a Phase I average flow of 0.5 MGD to an ultimate, Phase II average flow of 1.2 MGD. This expansion will utilize the Phase I, post-equalization basin as well as the Phase I digester. It will require splitting one reactor from phase I into two Nereda reactors, and using the other SBR reactor for ancillary basins. Existing influent valves and blowers will be kept for the retrofit.

- Phase I blowers may need to be re-belted and sheaved to meet phase II operating requirements (by others). The engineer should give thought to piping and site layout to facilitate the expansion.

- The control panel for Phase I has not been sized to incorporate Phase II equipment and will require additional hardware for future expansion.

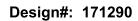
- Influent to the biological system is a typical municipal wastewater application. Influent TP shall be either in a particle associated form or in a reactive soluble phosphate form or in a soluble form that can be converted to reactive phosphorus in the biological system. Soluble hydrolyzable and organic phosphates are not removable by chemical precipitation with metal salts. A water quality analysis is required to determine the phosphorus speciation with respect to soluble and insoluble reactive, acid hydrolyzable and total phosphorus at the system Influent, point(s) of chemical addition, and final effluent.

- Chemical feed lines (i.e. metal salts) shall be furnished to each reactor, aerobic digester and dewatering supernatant streams as necessary.

## Design Notes

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023





- pH monitoring and control in a range of 6.5-8.5 of the biological reactor is required when adding metal salts.

- The average and maximum design flow and loading conditions, shown within the report, are based on maximum month average and maximum day conditions, respectively.

#### Post-Secondary Treatment

- -The following processes follow the Biological process:
  - Effluent flow equalization.

#### Equipment

- Changes in basin geometry may require alterations in the equipment recommendation.

- The basins are not included and shall be provided by others.

- The influent enters the basin near the reactor floor. Adequate hydraulic capacity shall be made in the headworks to prevent backflow from one reactor to the other during transition of influent.

- Based on the process requirements and selected equipment, the reactor wall height should be at least 25 ft for all basins.

- Scope of supply includes freight, installation supervision and start-up services.

- Equipment selection is based upon the use of Aqua-Aerobic Systems' standard materials of construction and electrical components, suitable for non-classified electrical environments.

- The post-EQ pump was not included in our previous scope of supply. Engineer should make sure the post-EQ pumps have the capacity of 1,100 gpm.

- The system has been designed to fit within existing basin dimensions. The ancillary basins are based on assumed basin dimensions.

- The control panel does not include motor starters or VFDs, which should be provided in a separate MCC (by others).

- Provisions should be made, by others, for overflows in each of the recommended basins.

- Aqua-Aerobic Systems, Inc. is familiar with various "Buy American" Acts (i.e. AIS, ARRA, Federal FAR 52.225, EXIM Bank, USAid, PA Steel Products Act, etc.). As the project develops Aqua-Aerobic Systems can work with you to ensure full compliance of our goods with various Buy American provisions if they are applicable/required for the project. When applicable, please provide us with the specifics of the project's "Buy American" provisions.

## Influent Buffer - Design Summary

Project: CENTREVILLE WWTP MD Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023

## Design#: 171290



#### **INFLUENT BUFFER DESIGN PARAMETERS**

Avg. Daily Flow:	= 1.20 MGD	= 4,542 m3/day
Max. Daily Flow:	= 3.00 MGD	= 11,356 m3/day
No. of AGS Reactors:		= 2

#### **INFLUENT BUFFER VOLUME DETERMINATION**

The volumes determined in this summary reflect the minimum volumes necessary to achieve the desired results based upon the input provided to Aqua. If other hydraulic conditions exist that are not mentioned in this design summary or associated design notes, additional volume may be warranted.

#### **INFLUENT BUFFER BASIN DESIGN VALUES**

No./Basin Geometry:	= 1 Rectangular Basin(s)			
Min. Water Depth:	= 0.0 ft	= (0.0 m)		
Max. Water Depth:	= 14.4 ft	= (4.4 m)		
Min. Basin Vol. Basin:	= 0 gallons	= (0.0 m <sup>3</sup> )		
Max. Basin Vol. Basin:	= 124,161.0 gallo	ons = (470.0 m <sup>3</sup> )		

## AquaNereda® - Aerobic Granular Sludge Reactor - Design Summary

Project: CENTREVILLE WWTP MD

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023



Design#: 171290

#### **DESIGN INFLUENT CONDITIONS**

Avg. Design Flow	= 1.20 MGD	= 4,542 m3/day
Max Design Flow	= 3.00 MGD	= 11,356 m3/day

			Effluent			
DESIGN PARAMETERS	Influent	mg/l	Required	<= mg/l	Anticipated	<= mg/l
Bio/Chem Oxygen Demand:	BOD5	250	BOD5	10	BOD5	10
Total Suspended Solids:	TSS	250	TSS	20	TSS	20
Total Kjeldahl Nitrogen:	TKN	40	TKN		TKN	
Total Ammonia Nitrogen:			NH3-N	1.0	NH3-N	1.0
Total Nitrate Nitrogen:			NOxN	10	NOxN	10
Phosphorus:	Total P	8	Total P	1.0	Total P	1.0

SITE CONDITIONS	Maxim	um	Minim	um	Elevation (MSL)
Ambient Air Temperatures: Influent	85 F	29.0 C	 30 F	-1.0 C	30 ft
Waste Temperatures:	68 F	20.0 C	50 F	10.0 C	9.0 m

<u>AGS BASIN DESIGN V</u>	ALUES		Water Depth			Basin Vol./I	Basin
No./Basin Geometry:	2 Rectar	ngular Basin(s)	Process Level (PWL):	21.0 ft	(6.4 m)	0.29 MG	(1,100 m <sup>3</sup> )
Freeboard (from PWL):	2.8 ft	(0.9 m)	Discharge Level (DWL):	22.3 ft	(6.8 m)		
Length of Basin:	34.7 ft	(10.6 m)	Top of Wall (TOW):	24.0 ft	(7.3 m)		
Width of Basin:	53.3 ft	(16.2 m)					

#### **PROCESS DETAILS**

TROOLOG DETAILO		
Cycle Duration:	= 4.5 Hours/Cycle	
Food/Mass (F/M) ratio:	= 0.065 lbs. BOD5/lb. MLSS-Day	
MLSS Concentration:	= 8000 mg/l	
Hydraulic Retention Time:	= 0.48 Days	
Solids Retention Time:	= 17.78 Days	
Est. Net Sludge Yield:	= 0.81 Lbs. WAS/lb. BOD5	
Est. Dry Solids Produced:	= 2032.0 lbs. WAS/Day	= (921.7 kg/Day)
AERATION DETAILS		
Lbs. O2/lb. BOD5	= 1.25	
Lbs. O2/lb. TKN	= 4.60	
Peak O2 Factor:	= 1.50	
Actual Oxygen Required:	= 7453 lbs./Day	= (3380.7 kg/Day)
Max. Discharge Pressure:	= 10.66 PSIG	= (74 KPA)
Max. Air Flowrate/Basin:	= 1,183 SCFM	
Min. Air Flowrate/Basin:	= 296 SCFM	
Max. Simultaneous Air:	= 1,771 SCFM	
Min. Simultaneous Air:	= 566 SCFM	
RETURN FLOW ESTIMATES		
Daily Estimated Return Flow:	= 0.18 MGD	
Max. Instantaneous Return Flow:	= 325 GPM	
POWER CONSUMPTION		
Average Aeration Power Consumption:	= 541 kWh/day (at 80% design load)	

## Sludge Buffer - Design Summary

Project: CENTREVILLE WWTP MD

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023

## Design#: 171290



#### **SLUDGE BUFFER DESIGN VALUES**

No./Basins Geometry:	= 1 Rectangular Basin(s)	
Minimum Level:	= 1.0 ft	= (0.3 m)
Max. Level:	= 15.4 ft	= (4.7 m)
Max. Basin Volume:	= 25,399 gallons	= (96.0 m <sup>3</sup> )
Length of Basin:	= 17.0 ft	= (5.2 m)
Width of Basin:	= 13.0 ft	= (4.0 m)

#### **SLUDGE BUFFER VOLUME DETERMINATION**

The sludge buffer volume has been determined based on the sludge production and the concentration of sludge from the AquaNereda reactors. The Sludge from this basin will be pumped to the sludge handling system, and the supernatant back to the head of the plant.

#### **SLUDGE BUFFER EQUIPMENT CRITERIA**

Max. Sludge Flow Rate Required:	= 70 gpm	= (16 m³/hr)
Max. Supernatant Flow Rate Required:	= 282 gpm	= (64 m³/hr)
Average Power Consumption:	= 16 kWh/day (at 80% design load)	

## Post-Equalization - Design Summary

Designed by Xu Ye on Wednesday, May 3, 2023

## Design#: 171290



#### **POST-EQUALIZATION DESIGN PARAMETERS**

Avg. Daily Flow (ADF):	= 1.20 MGD	= (4,542 m³/day)
Max. Daily Flow (MDF):	= 3.00 MGD	= (11,356 m³/day)
Decant Flow Rate from (Qd):	= 3,381 gpm	= (768 m³/hr)
Decant Duration (Td):	= 50 min	

#### **POST-EQUALIZATION VOLUME DETERMINATION**

The volumes determined in this summary reflect the minimum volumes necessary to achieve the desired results based upon the input provided to Aqua-Aerobic. If other hydraulic conditions exist that are not mentioned in this design summary or associated design notes, additional volume may be warranted.

#### **POST- EQUALIZATION BASIN DESIGN VALUES**

No./Basin Geometry:	= 1 Rectangular Basin(s)		
Min. Basin Vol. Basin:	= 0 gal	= (0 m <sup>3</sup> )	
Max. Basin Vol. Basin:	= 72,571 gal	= (275 m³)	

#### **POST- EQUALIZATION EQUIPMENT CRITERIA**

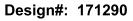
Max. Flow Rate Required Basin:	= 2,179.4 gpm	= (495.0 m³/hr)
Avg. Power Required:	= 136.8 kW-hr/day	

## Aerobic Digester - Design Summary

Project:	CENTREVILLE WWTP MD

Option:	Preliminary AquaNereda Retrofit Design	
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Designed by Xu Ye on Wednesday, May 3, 2023





#### **AEROBIC DIGESTER DESIGN PARAMETERS**

Sludge Flowrate to the Digester	= 23,790.0 gal/day	= (90.1 m³/day)
Inlet Sludge Concentration	= 1.00%	
Solids Loading to the Digester	= 1,984.1 lb/day	= (900.0 kg/day)
Inlet Volatile Solids Fraction	= 73.4%	

## AEROBIC DIGESTER BASIN DESIGN VALUES

No./Basin Geometry:	= 1 Rectangula	r Basin(s)			
Length of Basin:	= 52.8 ft	= (16.1 m)			
Width of Basin:	= 36.7 ft	= (11.2 m)			
Min. Water Depth:	= 14.7 ft	= (4.5 m)	Min. Basin Vol. Basin:	= 213,015.1 gal	= (806.4 m <sup>3</sup> )
Max. Water Depth:	= 21 ft	= (6.4 m)	Max. Basin Vol. Basin:	= 304,307.3 gal	= (1,152.0 m <sup>3</sup> )

#### **AEROBIC DIGESTER PROCESS DESIGN PARAMETERS**

Solids Retention Time:	= 25.6 days	
Digester Design Temperature:	= 20 C	
Volatile Solids Destruction:	= 40%	
Digester Solids Concentration:	= 2%	
Oxygen Supplied for Digestion:	= 2 lbs O2 per lb VSS Destroyed	
Oxygen Distribution Per Basin:	= 100.0%	
Actual Oxygen Required:	= 1,165.1 lb/day	= (528.5 kg/day)
Volatile Percentage After Digestion:	= 62.3%	
Estimated Dry Solids to be Removed:	= 1,401.6 lb/day	= (635.8 kg/day)
Volume of Solids to be Removed:	= 8,402.6 gal/day	= (31.81 m³/day)
Estimated Supernatant Volume:	= 91,292.2 gal/basin	= (345.58 m³/basin)
Assumed Supernatant Duration:	= 180 minutes	
Calculated Supernatant Flow:	= 507.2 gpm	= (32.0 l/sec)

1. The Volatile Solids Destruction listed above shall be used for determination of the oxygen demand during summer conditions. It should be noted that the actual VSS destruction will be dependent upon digester inlet condition, temperature, and operating conditions.

2. The Digester Solids Concentration is reflected as an average concentration, assuming the operations include frequent settling and supernating practices.

#### AEROBIC DIGESTER EQUIPMENT CRITERIA

Mixing Energy with Aerators:	= 140 HP/MG	= (27.58 W/m³)
NPHP Provided:	= 2.0	= (1.5 kW)
Mixing Energy with DDMs	= 40 HP/MG	= (7.88 W/m <sup>3</sup> )
NPHP Provided:	= 10	= (7.5 kW)
Max. Flow Rate Required Basin:	= 244 gpm	= (0.924 m³/min)
Avg. Power Required:	= 198.24 kW-hr/day	

Project: CENTREVILLE WWTP MD

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023

#### **AquaNereda**

#### Influent Distribution System

#### 2 Influent Distribution Assembly(ies) consisting of:

- Influent distribution system consisting of HDPE and PVC pipe with supports.

#### Effluent Weir Assembly

#### 2 Effluent Weir Assembly(ies) consisting of:

- Concrete main effluent channel(s) provided by others.
- Stainless steel weir assembliy(ies) with supports.

#### Sludge Removal System

#### 2 Solids Waste System(s) consisting of:

- HDPE or Stainless steel solids waste system(s).
- Pressure transmitter(s).

#### 2 Sludge Decant/WLC Valve Set(s) consisting of:

- Each reactor includes two (2) of the following automatic control valves and two (2) of the following manual throttling valves:
- 14 inch DeZurik electrically operated butterfly valve(s) with Limitorque actuator.
- 14 inch diameter DeZurik manual plug valve(s).

#### 2 Air Valve Set(s) consisting of:

- Each reactor includes two (2) of the following automatic valves and one (1) of the following manual valves:
- 4 inch DeZurik electrically operated butterfly valve(s) with Limitorque actuator.
- 4 inch manual butterfly valve(s).

#### Fixed Fine Bubble Diffusers

#### 2 Fixed Fine Bubble Diffuser Assembly(ies) consisting of:

- 304 SS, 12 Ga. drop pipe(s).
- PVC, Sch 40 Manifold(s) with connection to drop pipe.
- PVC, Air distributor(s) with connection to the manifold and required PVC pipe joint connections.
- 304 Stainless steel piping supports with vertical supports, clamps, adjusting mechanism and anchor bolts.
- Fine bubble diffuser assemblies.
- Air muffler(s).

#### **Positive Displacement Blowers**

#### 1 Positive displacement Blower Package(s), with each package consisting of:

- Aerzen 50HP Rotary Positive Displacement Blower(s).

#### Air Valves

#### 2 Air Control Valve(s) will be provided as follows:

- 6 inch DeZurik electrically operated butterfly valve(s) with Limitorque actuator.
- Auma actuator will be upgraded from open/close service to modulating service.
- Air flow meter(s).
- Flow conditioner(s).

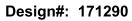


Design#: 171290

Project: CENTREVILLE WWTP MD

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023





#### Level Sensor Assemblies

- 2 Pressure Transducer Assembly(ies) each consisting of:
  - Pressure transducer(s).
  - Mounting bracket weldment(s).
  - Transducer mounting pipe weldment(s).
- 2 Level Sensor Assembly(ies) will be provided as follows:
  - Float switch(es).
  - Float switch mounting bracket(s).
  - Stainless steel anchors.

#### **Instrumentation**

- 1 Server Based Control and Monitoring System will be provided as follows:
  - Process Controller Server.
  - Small server monitor.
  - Process Operator Station.
- 2 Dissolved Oxygen Assembly(ies) consisting of:
  - DO probe(s).
- 2 TSS Sensor(s) will be provided as follows:
  - TSS probe(s).
- 2 ORP Sensor(s) will be provided as follows:
  - ORP sensor(s).
- 2 pH Sensor(s) will be provided as follows:

- pH probe(s).

1 Phosphorus Analyzer(s) will be provided as follows:

- Phosphate analyzer(s).

- 1 Filtrax Sampling System(s) will be provided as follows:
  - Sampling system.
- 1 Process Controller(s) consisting of:
  - Controller and display module(s).
- 2 Process Controller(s) consisting of:

- Controller(s).

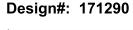
- 1 Process Control System will be provided as follows:
  - Hach SC1000 display module.
  - FRP enclosure(s) for SC1000 Display.
- 1 Ammonium Probe(s) will be provided as follows:
  - Ammonium probe(s).
  - Controller(s).

#### AquaNereda: Sludge Buffer

Project: CENTREVILLE WWTP MD

Option: Preliminary AquaNereda Retrofit Design

Designed by Xu Ye on Wednesday, May 3, 2023





#### Transfer Pumps/Valves

- 1 External Pump Assembly(ies) consisting of the following items:
  - 7.5HP Pump assembly(ies).
- 1 Sludge Valve(s) consisting of the following items:
  - 3 inch DeZurik electrically operated Plug Valve(s).
- 1 Supernatant Valve(s) consisting of the following items:
  - 6 inch DeZurik electrically operated Plug Valve(s).

#### Sludge Removal System

#### 1 Solids Removal Assembly(ies) consisting of:

- Solids removal assembly(ies) consisting of PVC and/or HDPE pipe with supports.

#### Level Sensor Assemblies

#### 1 Pressure Transducer Assembly(ies) each consisting of:

- Pressure transducer(s).
- Mounting bracket weldment(s).
- Transducer mounting pipe weldment(s).

#### 1 Level Sensor Assembly(ies) will be provided as follows:

- Float switch(es).
- Float switch mounting bracket(s).
- Stainless steel anchors.

#### **Instrumentation**

#### 1 Hach TSS WAS Sensor(s) will be provided as follows:

- Hach Solitax Inline sc stainless steel pipe isertion probe with stainless steel wiper and 33 ft electric cable. One (1) probe per basin.

#### 1 Process Controller(s) consisting of:

- Controller and display module(s).

#### AquaNereda: PLC Controls

#### **Controls wo/Starters**

#### 1 Controls Package(s) will be provided as follows:

- NEMA 12 panel enclosure suitable for indoor installation and constructed of painted steel.
- Fuse(s) and fuse block(s).
- Compactlogix Processor.
- Operator interface(s).
- Remote access Ethernet modem(s).



Item 5: Final Clarifier



**BUDGETARY PROPOSAL #WG08049** 

CENTREVILLE, MD - WRA - CENTREVILLE, MD

May 6, 2024

Attn: Irene Pais Geiger Pump & Equipment Company 830 Tryens Road Aston PA 19014 USA Phone: (610) 459-5747 Fax: (610) 459-3992 email: IPais@geigerinc.com

Re: Centreville, MD - WRA - Centreville, MD Polychem<sup>™</sup> Chain and Flight Sludge Collection System

#### **BUDGETARY PROPOSAL**

Brentwood Industries, Polychem Brand, proposes and offers to supply all materials and services as an Approved manufacturer and in general accordance with Brentwood's standard practices and specifications, clarifications, and information provided.

TECHNICAL SPECIFICATION(S):N/ASECTION(S):N/AADDENDA RECEIVED:N/A

#### BRENTWOOD PROPOSES TO FURNISH POLYCHEM CHAIN AND FLIGHT EQUIPMENT AS FOLLOWS:

Four (4) Secondary Longitudinal Collector Mechanisms, Approximately 60 FT Long x 16.75 FT Wide x 14 FT AWD, 4 Shaft System



Brentwood Industries, Inc. 500 Spring Ridge Dr., Reading PA 19610 brentwoodindustries.com



**BUDGETARY PROPOSAL #WG08049** 

CENTREVILLE, MD - WRA - CENTREVILLE, MD

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ITEM	DESCRIPTION / MATERIAL
Drive Chain	NH78, Reinforced Nylon Resin w/ 303 SS Pins
Collector Chain Pins and Retainer Clips	Glass Reinforced Nylon Pins w/ Acetal Retainer Clips
Collector Chain Links	NCS-720-S, Reinforced Thermoplastic Polyester Resin
Flight Attachment Links	NCS-720-S, Reinforced Thermoplastic Polyester Resin, F-22-8
Flights	3"x8" nominal C-Channel w/ Integral Lip, Fiberglass Reinforced Plastic, spaced at 10 Ft ( 3.05 m ) intervals
Wear Shoes	Nylon 6-6
Hardware	316 SS
Fillerblocks	Polypropylene
Headshaft Spindles	Cast Nylon-6
Headshaft(s)	Biaxially Wrapped Fiberglass Epoxy Tube(s) w/ Internal UHMW-PE Tubular Bearings
Driven Sprocket(s)	NH78, 40T, Cast Nylon-6, w/integral teeth
Collector Sprockets for Headshaft(s)	NCS-720-S, 23T, Cast Nylon-6
Set Collars	Split, Cast Nylon-6, w/ 316 SS Clamping Band
Headshaft Keys	Glass Reinforced Nylon 6-6
Collector Sprockets for Stub Shafts	NCS-720-S, 17T, Cast Nylon-6
Idler Stub Shafts	Cast Nylon-6 w/UHMW-PE Outer Journal Bearing
Retainer Plate for Stub Shafts	Polycarbonate
Wall Bracket Supports for Return Track	Glass Reinforced Nylon 6-6
Run Shoe to Splice Wall Bracket to Return Track	Nylon 6-6
Return Track	3" x 3" x 3/8" Angle, Fiberglass Reinforced Plastic



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CENTREVILLE, MD - WRA - CENTREVILLE, MD

*ITEMS INCLUDED (Continued):						
ITEM	DESCRIPTION / MATERIAL					
Wear Strip	UHMW-PE - 3/8" thick x 2-5/8" wide					
Chain Tightener(s) for Drive Chain	Nylon 6-6 7T Sprocket w/ Cast Nylon-6 Arm and FRP Adjustable Mounting Bracket					
Limit Switch	DPDT, Cutler Hammer, Zinc Die Cast, NEMA 4X, SS Arm					
Drive Sprocket Shear pin Assembly	11T Nylon Sprocket Mounted to 304 SS Shear Pin Hub					
Shear pin Kit(s)	Aluminum					
Drive Unit Output Shaft	304 SS					
Drive(s) - Single, Each Driving (1) Collector	SEW Eurodrive Helical-Bevel Gear box (DIN-ISO) with integral mount SEW Motor (IEC), 1/2 HP, 3 PH, 60 Hz, 230/460 VAC					
Base Plate for Drive Unit(s)	304 SS					
Chain Guard for Drive Chain	304 SS					
Deflector Rail (if required by equipment layout)	FRP Angle Rail w/UHMW-PE Wear Strip and Nylon 6-6 Wall Support Brackets					
Anchor System	316 SS					
Adhesive for Anchors w/ Dispenser	Hilti					
*	Above Item Descriptions/Materials may vary slightly after engineering and consultant review.					



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Fax: 610.685.0137



## CENTREVILLE, MD - WRA - CENTREVILLE, MD

The following total estimated spare parts will be furnished for this project. After engineering, quantities may vary from quantities listed below. Spare Parts will be packaged separately and plainly identified.

SPARE PARTS INCLUDED					
QTY	DESCRIPTION				
20	feet of drive chain				
10%	of all collector chain furnished				
10%	of all chain-to-flight attachment links furnished				
12	shear pins for every drive sprocket assembly furnished				
5	longitudinal flights complete with wear shoes, fillerblocks, and hardware				
1	replacement 11T drive sprocket (sprocket plate only)				

## ITEMS SPECIFICALLY NOT INCLUDED

- 1 SmartGuard Flight and Sprocket Monitoring System
- 2 Rotating Scum Troughs or Helical Skimmers
- 3 Control Panel(s)
- 4 Effluent Troughs, Weirs, Baffles
- 5 Seismic Calculations
- 6 Hold Down Rail, 304 SS
- 7 Tank Measurements
- 8 PE Stamp of Submittals
- 9 Triple or Right Angle Drives Operating Two (2) Common Longs & Cross Collector



Phone: 610.374.5109



## CENTREVILLE, MD - WRA - CENTREVILLE, MD

### **EXISTING CONCRETE STRUCTURE (IF APPLICABLE):**

Pricing and schedule are based on limited structural information provided at the time of quotation and assume the necessary existing tank dimensions will be provided by purchaser in a timely manner to facilitate the start of submittals. In lieu of customer supplied tank dimensions, purchaser may elect to procure Brentwood's Tank Measurement services. Should the verified tank dimensions and equipment conditions differ from the information provided for quotation, and/or require special bracketry or supporting structures, Brentwood reserves the right to revise pricing and schedule accordingly. Delays associated with receipt of complete tank measurements, incomplete information from RFI's, and release and approval to manufacture may result in changes to the price and schedule.

### **TANK MEASUREMENTS:**

Tank Measurements are NOT included in this price or proposal, but can be provided and billed per attached published field labor and expense rates. If measurement services are purchased, Brentwood will require the assistance of one (1) person while on site to support tank measurements, and tanks must be completely drained and cleaned before entrance. In addition, customer / contractor shall supply all necessary equipment to safely access tanks (ladders, lighting, etc.). Tank measurement services require a minimum 2 week notice and are based on technician availability.

### **SUBMITTALS:**

Shop drawing and submittal preparation will be in accordance with Brentwood's standard submittal practices, and will be based on one submittal for all tanks at one time. Should separate submittals for each tank be required at separate intervals, Brentwood reserves the right to revise pricing accordingly.

### TIME AND DELIVERY:

- 1. Brentwood will furnish initial submittal drawings approximately ten (10) Weeks after receipt of executed purchase order and field verified structural dimensions and information. PE review, calculations and stamp (if required) may be sent at a later date under separate cover.
- 2. Estimated Submittal Review: Brentwood estimates a four (4) week review period by consultant or customer.
- 3. We further propose to furnish the equipment approximately thirteen (13) weeks after receipt of final engineering approval and returned submittal drawings and release to manufacturing.

### FREIGHT:

Freight allowed, best way, point of manufacture to job site. Requests for specific methods of shipment will be at requestors' expense. On-site transportation, unloading, and storage costs by others.

### **WEIGHT AND VOLUME:**

Estimated weight is 11,300 Lbs. Estimated volume is One (1) Truck(s).

### TAXES:

Pricing does not include any States' sales tax if applicable, unless otherwise stated.





## CENTREVILLE, MD - WRA - CENTREVILLE, MD

### **SCHEDULE OF VALUES & PAYMENT TERMS:**

- 1. 15% with Shop drawing and submittal transmission; 35% with approved submittals and/or release to manufacture; 50% on material shipment. All payments 100% Net 30 days from invoice date. Payment terms subject to credit approval.
- 2. These terms are not contingent upon or in conjunction with any agreement purchaser has with other parties.
- 3. For Brentwood Water & Wastewater Standard Terms and Conditions visit: <u>https://www.brentwoodindustries.com/terms/</u>

### **ESCALATION:**

The price(s) quoted are subject to adjustment to reflect increases in material cost(s), should these increases in price exceed 3% during the specified Schedule of Construction. Increases are based on price indexes for PVC (ChemData) and Stainless Steel (MEPS International), which can be provided upon request. It is understood and agreed that it will be Brentwood's option whether to invoke escalation, should the price exceed this amount.

### **BILL AND HOLD:**

If Purchaser fails to take delivery on any scheduled delivery date based on the terms of the executed purchase Agreement, Brentwood reserves the right to reallocate any Product to other projects and reschedule production for the delayed Product. Purchaser will be required to accept any increase in price associated with the repurchase of material to fulfill the purchased Product requirements and the Product Delivery Date will be rescheduled in conjunction with current production schedules.

If the Purchaser requests that Brentwood holds Product in excess of an agreed upon delivery date and Brentwood agrees to hold the Product, Purchaser will provide written notification to Brentwood to store the Product at its facilities for a period of time prior to shipment ("Bill and Hold"). Brentwood will provide written confirmation of the Bill and Hold to Purchaser, including a Statement of Transfer of Title and invoice.

Payment for the Bill and Hold material is due in accordance with the agreed upon terms in the executed purchase Agreement except to the extent dates must be adjusted due to delivery rescheduling, in which case adjusted dates will be shown on the invoice. All payments will be made in accordance with the invoiced payment terms and instructions. For all Bill and Holds, Purchaser acknowledges that (i) they have made a fixed commitment to purchase the Product, (ii) risk of ownership for the Product passes to Purchaser upon signing Statement of Transfer, (iii) Purchaser has requested that the Product be on a Bill and Hold basis for legitimate business purposes, (iv) if no delivery date is determined at the time of invoicing and Statement of Transfer and Brentwood does not receive a request for delivery within two (2)months from the Bill and Hold invoice date, Brentwood has the right to release the shipment upon written notice to Purchaser any time following the two (2) month period from Bill and Hold invoice date. Brentwood shall be entitled to storage charges of 1 ½% per month of the purchase value of stored material beginning 30 days after Bill and Hold invoice date and continuing until the Product is picked up by Purchaser or shipped by Brentwood. Upon receipt of request from Purchaser to ship the stored Product, Brentwood shall use commercially reasonable efforts to ship the Product within two (2) to 4 (four) business weeks following confirmed receipt of such request.





## CENTREVILLE, MD - WRA - CENTREVILLE, MD

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### VALIDITY:

This proposal is valid for a period not to exceed 90 days from latest date shown above unless extended by Brentwood in writing. Pricing on this project is based upon shipment schedule as shown above. Extensions to delivery timelines or requests for staged shipments may require renegotiation of pricing.

### FIELD SERVICE STARTUP AND TRAINING:

The services of a qualified Brentwood field technician is included to assist in inspection of installed equipment, startup and field testing, certification, and operator training, if required by specification. Duration limited to Two (2) trip(s) for Four (4) man-day(s) on site total. Non use of contractual field service days does not generate a credit on this project. Field service requires a minimum 2 week notice and is based on technician availability. Less notice may be accommodated with additional costs.

### **OPERATION AND MAINTENANCE MANUALS:**

Unless otherwise specified, one (1) digital copy of our O&M manual and installation and layout drawings will be furnished on or before shipment of equipment. Digital copy can be downloaded from our FTP site or finished on a USB Flash drive. Digital copy of O&M shall be in Adobe pdf format and be locked and uneditable.

### WARRANTY:

Brentwood warrants material supplied on this project to be free from defects in workmanship or materials for a period of twelve (12) months from date of certification by an authorized Brentwood representative or eighteen (18) months from date of shipment, whichever shall occur first. Warranty excludes labor to install or remove parts. Chain and flight system is designed for continuous operation, and intermittent operation is not recommended due to potential for excess sludge build up. Damage resulting from intermittent operation of chain and flight equipment is not covered under this warranty.

### **PAINTING AND COATINGS:**

Stainless Steel and plastic equipment shall not be painted. Unless otherwise specified, all ferrous wetted components will be provided with a surface preparation of SSPC-SP10 Near White Metal and a shop primer 1 coat of Sherwin Williams Dura-Plate 235 Multi-Purpose Epoxy @ 4 Mils D.F.T. It is the responsibility of the contractor to ensure finish paint is compatible with specified primer. Any adhesion issues between coats are not the responsibility of Brentwood. The top coat must be applied within 6 months of the prime coat, otherwise the assembly surface will need to be abraded or the primer will need to be removed and surface preparation redone prior to application of the top coat, by others. OEM components above deck (drive units, bearings, actuators, etc.) shall be furnished with manufacturer's factory finish.

### **AMERICAN IRON AND STEEL ACT:**

Per Implementation of American Iron and Steel provisions of P.L. 113-76, Consolidated Appropriations Act, 2014, Brentwood's Polychem brand clarifier System and accessories is considered a mechanical system and is not considered construction material or structural steel subject to AIS requirements.



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**BUDGETARY PROPOSAL #WG08049** 

## CENTREVILLE, MD - WRA - CENTREVILLE, MD

### **GENERAL EXCLUSIONS\*:**

- 1. Contractor/customer shall be responsible for field verification of all dimensions.
- 2. Foundations, supports for Polychem equipment (diaphragm plates) or special mounting plates.
- 3. Bid, performance, supply, or maintenance bonds.
- 4. Installation of equipment and anchor systems, concrete, sealing compounds, shim stock or grout.
- 5. Grouting behind idler stub shafts, head shaft spindles, & return track wall brackets is not included, but is required for these systems.
- 6. Tools or spare parts (unless listed elsewhere in this Proposal).
- 7. All reducer oil, bearing grease, or other lubricants.
- 8. Field paint, touch-up, finish painting, or finish coatings.
- 9. Unloading, hauling, erection, and storage of equipment.
- 10. Grease line piping (unless listed elsewhere in this Proposal) or grease guns.
- 11. Any electrical components or controls not shown in items included section of this Proposal.
- 12. All control panels (unless listed elsewhere within this Proposal), unistrut supports / mounting for control panels, electrical conduit, wires, or wiring, wire fittings, or boxes.
- 13. Wall Sleeves for scum troughs, weirs, baffles, overflow weirs, effluent troughs.
- 14. Anchor pull out testing.
- 15. PI&D drawings
- 16. Conduit sizing or drawings.
- 17. Detailed specific storage plans or maintenance schedules for installed equipment outside of Brentwood's standard maintenance and preventative maintenance information.
- 18. Factory assembly of components.
- 19. Any component shown or described on a drawing and not included in the Items Included section of this Proposal, or any component or service not shown in this Proposal.

\*unless above items are listed as included elsewhere in this Proposal, they are excluded.





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BUDGETARY PROPOSAL #WG08049

CENTREVILLE, MD - WRA - CENTREVILLE, MD

**PRICING SUMMARY:** 

## LUMP SUM BUDGETARY BASE PRICE: \$311,700.00

ADDERS TO BASE OFFERING:

Four (4) 304 SS Rotating Scum Troughs, Manual Lever Operated, Approximately 12-Inch Diameter x 16.75FT Long: \$183,050.00

Proposal Submitted By:

Jonah Graciani

Jonah Graciani, Sales Estimator Brentwood Industries, Polychem Brand email: jonah.graciani@brentwoodindustries.com





# FIELD SERVICE RATES

2022 - 2025



## DOMESTIC DAILY RATES PER 8 HOUR DAY

SERVICE SPECIALIST	2022	2023	2024	2025
Straight Time	\$1,890.00	\$2,003.00	\$2,123.00	\$2,250.00
OT and Saturday	\$2,827.00	\$2,996.00	\$3,175.00	\$3,365.00
Sunday and Holiday	\$3,780.00	\$4,006.00	\$4,246.00	\$4,500.00

## INTERNATIONAL DAILY RATES PER 8 HOUR DAY

SERVICE SPECIALIST	2022	2023	2024	2025
Straight Time	\$2,268.00	\$2,404.00	\$2,548.00	\$2,701.00
OT and Saturday	\$3,402.00	\$3,606.00	\$3,822.00	\$4,051.00
Sunday and Holiday	\$4,538.00	\$4,810.00	\$5,099.00	\$5,404.00

## Definition of Labor Rates

Straight time applies to first eight (8) hours worked and traveled Monday through Friday. Any time worked over 8 hours, up to four (4) hours worked and traveled past eight (8) on Monday through Friday, first twelve (12) hours worked on Saturday will be charged at overtime rate. Standby time will be charged at the applicable rate. In case of long-term assignments, Field Service personnel will be rotated at Buyer's expense.

### Expenses

Meals, lodging, and incidental expenses will be billed at cost + 15%. Employee travel expenses will be charged at cost +15% for airfare, rental vehicles, taxis and freight. Mileage rate is \$0.95 per mile. Rental of lifting or other special equipment, outside inspection services, additional sub contracted services, etc. will be cost +15%.

### Notes:

- 1. This rate sheet supersedes all previously issued rate sheets.
- 2. All prices in US dollars.
- 3. Any "site-specific" training required will be billed as time worked.
- 4. Customer to furnish water, oils, solvents and will dispose of same. Customer will also furnish power and air, parts, ladders,
- access to job-site, overhead crane upon request, and all necessary work permits.
- 5. Rates are "Portal-to-Portal". Travel time, to and from the site, will be considered hours worked and billed at the applicable rate.
- 6. Stand-by time will be considered hours worked and billed at the applicable rates according to the following:
- a. Stand-by from home base 8 hours per day.
- b. Stand-by while mobilized and in the field 8 hours per day.
- 7. A 4-hour minimum will apply to all service work.
- Rates quoted are subject to adjustment without notice to conform to Seller's published rates in effect at the time service is performed.
- 9. This offer is subject to Buyer's acceptance of the Conditions above.

10. This offer and any work performed as a result are exclusively governed by our Terms and Conditions attached. Any additional or conflicting terms contained in any document or purchase order issued authorizing work are expressly objected to in advance and shall not apply, except with the express written consent from Brentwood Industries.



Brentwood Industries, Inc. 500 Spring Ridge Dr., Reading PA 19610 brentwoodindustries.com

Fax: 610.685.0137



#### Brentwood Water Group (Water & Wastewater) Standard Terms and Conditions of Sale

Applicability and Acceptance These terms and conditions of sale ("Terms) are the only terms which govern the sale of product ("Product") by Brentwood Industries, Inc. ("Brentwood") to Purchaser ("Purchaser"). Brentwood and Purchaser together are the "Parties" and each a "Party" herein. Brentwood's accompanying quotation or proposal (collectively "Proposal") and these Terms (collectively this "Agreement"), comprise the entire agreement between the Parties and supersede all understandings, agreements, negotiations, representations, or communications. In the event of a conflict between these Terms and a Proposal, the terms and conditions in the Proposal prevail. Brentwood's commencement of work or service does not constitute acceptance of any Purchase Order. No Purchase Orders will be binding upon Brentwood without express written acceptance by an authorized Brentwood employee. These Terms will be the sole, controlling terms for Purchaser's Purchase Order ("Purchase Order") and no other terms and

# conditions will apply. Pricing and Payment:

Payment to be 100% prepayment of goods before shipment unless a credit application has been completed and an extension of credit has been approved. Approved payment terms shall be due in full within thirty (30) days from invoice date. Pricing is in accordance with Brentwood's Proposal. Brentwood reserves the right to adjust the Proposal price at any future time due to raw material and/or labor cost fluctuations greater than +/- 3%.

#### Shipment and Title:

The shipment terms unless stated otherwise in Brentwood's Proposal will be EXWORKS. Risk of loss and title transfer at Brentwood's facility. Brentwood may, without liability or penalty, make partial shipments of Products to Purchaser.

#### Inspection and Claims:

Upon delivery of Product, Purchaser must inspect the Product for freight damage and must notify Brentwood in writing within five (5) days after delivery. Furthermore, Purchaser agrees to inspect and accept the Product within a reasonable timeframe. Brentwood may waive claims not made in accordance with the above terms in this section.

#### Default:

Purchaser's failure to make payment as agreed and according to invoices or Purchaser's failure to perform any of its other obligations under this Agreement constitutes a default. In the event of default, Brentwood will provide written Notice of the default (in accordance with the Notices section of this Agreement) to Purchaser. If Purchaser does not i) correct the default or ii) address how it plans to correct the default in writing to Brentwood within five (5) business days from receipt of Notice of default, Purchaser will remain in default and Brentwood may do any of the following, (i) exercise any and all other rights and remedies of a secured Party under Article 9 of the UCC or applicable law; (ii) suppend any further Product deliveries or provision of services until Purchaser pays its obligations in full; iii) be excused from any of its performance obligations under this Agreement resulting from Purchaser's delays or inability to complete its obligations; iv) send Purchaser's past due invoice(s) to collections for nonpayment of obligations and report Purchaser's non-payment to appropriate credit agency.

#### Delays: :

Delays in project schedule beyond the expected ship date not caused by Brentwood which result in additional costs not included in quoted price may be invoiced by Brentwood to Purchaser.

#### Storage Fees:

Unless otherwise agreed upon by Brentwood and Purchaser, in the event Purchaser notifies Brentwood it cannot take delivery on the agreed upon delivery date on the face of Purchaser's Purchase Order, Brentwood will store the Product free of charge for up to thirty (30) days after the initially agreed delivery date. After the thirtieth (30<sup>th</sup>) day, Purchaser agrees to pay a monthly storage fee equal to one and one-half (1.5%) percent of the invoice price of the Product. The monthly storage fee will be due in full upon receipt of invoice for the storage fee regardless of whether Purchaser has been invoiced or has paid for the Product.

#### Termination:

Brentwood or Purchaser may terminate this Agreement if either Party defaults by materially breaching its obligations in this Agreement, provided the breaching Party does not commence correction of the breach within five (5) business days from receipt of written notice of default. The Parties will agree upon a reasonable amount of time to correct the breach. In the event the Party in default fails to correct the breach within the agreed upon time frame, the other Party may terminate the Agreement by providing written notification to the Party in default. In the event of termination, the Purchaser agrees to pay Brentwood cancellation charees in accordance with the table below based on the Purchase Order Value.

Contracted	Contracted Elapsed Time - from date of Executed Purchase Order to date of Cancellation (weeks)															
Ship ment (weeks)	0 - 2	2.01 - 4	4.01 - 6	6.01 - 8	8.01 - 12	12.01 - 16	16.01 - 20	20.01 - 24	24.01 - 28	28.01 - 32	32.01 - 36	36.01 - 40	40.01 - 44	44.01 - 48	48.01 - 52	52.01 - 56
Up to 8	20	50	75	100												
8.01 - 12	15	40	60	80	100											
12.01 - 16	10	25	45	60	85	100										
16.01 - 20	10	15	25	45	65	85	100									
20.01 - 24	10	10	20	25	50	70	90	100								
24.01 - 28	10	10	15	20	25	50	70	90	100							
28.01 - 32	10	10	10	15	20	35	60	75	90	100						
32.01 - 36	10	10	10	15	20	25	50	60	85	95	100					
36.01 - 40	10	10	10	10	15	25	50	60	70	85	95	100				
40.01 - 44	10	10	10	10	15	25	45	55	65	80	90	95	100			
44.01 - 48	10	10	10	10	15	25	45	55	60	65	80	90	95	100		
48.01 - 52	10	10	10	10	15	20	40	50	55	60	70	85	90	95	100	
52.01 - 56	10	10	10	10	15	20	35	50	55	60	70	80	85	90	95	100

#### Changes:

Purchase Order changes are subject to Brentwood's written approval, and additional time and charges may apply. Brentwood will not be liable for any delays due to change order requests. Brentwood may make changes to its Product without obligation, apply or manufacture such changes in any Product manufactured prior thereto. Brentwood may make such changes to any ordered Product as does not, in Brentwood's reasonable judgment, interfere with the satisfactory operation of the Product.

#### Taxes:

All government charges upon the production, shipment or sale of the Product, including, without limitation, sales, use, occupation, export and import taxes, and any other impositions by any government whatsoever, direct or indirect, including those required to be collected by Brentwood, will be paid by Purchaser or, in lieu thereof, Purchaser will furnish Brentwood with an exemption certificate acceptable to the taxing authority. Brentwood reserves and Purchaser disclaims all rights to drawback of duties paid on materials used in the manufacture of the Product. Purchaser will supply Brentwood with proof of exportation and all other documents necessary and otherwise cooperate to obtain payment thereof.

#### Returns:

No Product may be returned for credit or otherwise unless Purchaser receives Brentwood's authorization. Product authorized for return or credit must be returned in good condition, in its original packaging with completed identification and with all supporting documentation detailing of any claimed defect as required by Brentwood. All shipping and freight charges shall be prepaid by the Purchaser. The returned Product may be subject to a restocking charge of 30%.



Brentwood Industries, Inc. 500 Spring Ridge Dr., Reading PA 19610 brentwoodindustries.com

Fax: 610.685.0137

#### Warranty:

Brentwood warrants against defects in materials and workmanship. Warranty coverage is contingent on proper storage, installation, use, operation, maintenance, and shutdown procedures, all occurring under ordinary conditions and in compliance with good industry standards, the approved design criteria, Brentwood's approved Submittal and Operation and Maintenance Manual. The Warranty period shall be limited to twelve (12) months from Product shipment. The terms of this Warranty shall be modified only through written agreement by an authorized Brentwood employee. The remedy for a covered defect during the Warranty period shall be limited, at Brentwood's option and control, to repair or replacement of defective Parts and Components, including shipping costs. The remedy excludes costs of labor, removal of non-conforming Products, and expenses related to installation of the replacement Products. THE TERMS OF THIS WARRANTY ARE THE SOLE AND EXCLUSIVE OBRENTWOOD TO PURCHASER OR THIRD PARKY FOR CLAIMS RELATED TO THE PRODUCT. UNDER NO CIRCUMSTANCE SHALL

THE TERMS OF THIS WARRANTY ARE THE SOLE AND EXCLUSIVE OBLIGATION OF BRENTWOOD TO PURCHASER OR THIRD PARTY FOR CLAIMS RELATED TO THE PRODUCT. UNDER NO CIRCUMSTANCE SHALL BRENTWOOD BE LIABLE TO ANY PERSON OR ENTITY FOR ANY INCIDENTIAL, CONSEQUENTIAL, SPECIAL, OR INDIRECT DAMAGES OR ANY OTHER LOSS, COST, OR EXPENSE OTHER THAN SPECIFICALLY STATED IN THIS WARRANTY. OTHER THAN THE EXPRESS LIMITED WARRANTIES MADE HERIN, BRENTWOOD EXPRESSLY DISCLAIMS ANY AND ALL OTHER WARRANTIES, EXPRESS OR IMPLIED BY LAW, WITH RESPECT TO ANY SERVICE OR DELIVERABLE, INCLUDING, WITHOUT LIMITATION, ANY IMPLIED WARRANTY OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE, AS WELL AS ANY WARRANTIES WHICH MAY ARISE FROM PRIOR COURSE OF DEALING, CUSTOM, TRADE USAGE, PROVISION OF SAMPLES, PRODUCT LITERATURE OR WEBSITE CONTENT.

#### Limitation of Liability:

REGARDLESS OF THE FORM OF ACTION, BRENTWOOD'S LIABILITY RELATING TO THE PRODUCT OR THE MANUFACTURE, SHIPPING, SALE OR USE OF THE PRODUCT SHALL NOT EXCEED THE PRICE PAID BY PURCHASER FOR THE SPECIFIC PRODUCT GIVING RISE TO THE CAUSE OF ACTION. BRENTWOOD, ITS AFFILIATES, AND THEIR OFFICERS, DIRECTORS, EMPLOYEES AND AGENTS SHALL NOT BE LIABLE FOR ANY INDIRECT, SPECIAL, INCIDENTAL, EXEMPLARY, PUNITIVE OR CONSEQUENTIAL DAMAGES, INCLUDING, WITHOUT LIMITATION, LOSS OF PROFITS, LOSS OF USE, DOWNTIME, FAILURE TO DETECT ANY FLAW IN ANY SUBJECT MATTER OF ANY TEST, LOSS OF GOODWILL, BUSINESS INTERRUPTION, DELAY IN PERFORMANCE, OR LOST OPPORTUNITIES. REGARDLESS OF THE FORM OF ACTION, WHETHER IN CONTRACT, TORT (INCLUDING NEGLIGENCE), STRICT PRODUCT LIABILITY OR OTHERWISE IN CONNECTION WITH THE SUPPLY OR SUBSEQUENT USE OR POSSIBILITY OF SUCH DAMAGES.

#### Indemnification:

Purchaser will at all times indemnify, defend and hold harmless Brentwood, its officers, directors, employees, agents, servants and representatives from and against any and all damages, liabilities, losses, claims, suits, penalties, fines, costs, and expenses, including attorneys' fees (collectively, "Claims") arising directly or indirectly out of or in connection with any (a) infringement or misappropriation of any patent, trademark, or other intellectual property right, including third Party rights, arising from Brentwood's adherence to Purchaser's Specifications; (b) use, operation or possession of Brentwood Product, except to the extent the Claim arises from the gross negligence or willful misconduct of Brentwood; or (c) breach by Purchaser of any provision of any Agreement with tor obligation to Brentwood.

Brentwood will at all times indemnify, defend and hold harmless Purchaser from and against loss, injury, damage and liability arising directly in connection with bodily injury death, or destruction of tangible or real property, including loss of use directly resulting from or caused by Brentwood or Brentwood's product, its negligent act, error, omission or for damages arising from Brentwood's gross negligence or willful misconduct in performance of its obligations under this Agreement. Claims and damages are limited to Brentwood's proportionate percentage of negligence and/or fault.

#### Insurance:

Brentwood will maintain and carry insurance including, but not limited to Commercial General Liability in a sum of \$1,000,000 per occurrence and Workers Compensation in amounts as required by applicable statute. Additional coverages may be available. Upon request, Brentwood will provide to Purchaser a certificate of insurance evidencing its coverages.

#### **Confidential Information:**

All non-public, confidential and proprietary information ("Confidential Information"), whether disclosed orally or reduced to writing, whether or not marked or otherwise designated or not identified as such. Confidential Information does not include information which: (i) is or becomes available to the public generally (other than as a result of a disclosure by the Purchaser in violation of this Agreement); (ii) is subject to public disclosure under any federal, state or local law, ordinance or regulation; (iii) becomes available to Purchaser on a non-confidential basis from a source other than Brentwood; or (iv) was known by or was available to Purchaser prior to or at the time Brentwood isclosed it.

Purchaser agrees to protect and safeguard all Confidential Information with at least the same degree of care as the Purchaser would protect its own Confidential Information, but in no event with less than a commercially reasonable degree of care. Purchaser shall hold all Confidential Information in confidence and shall disclose it only to its employees needing to use the Confidential Information for the limited purposes of this Agreement and said employees shall be bound to the confidential Information for any purpose of the Agreement. No other disclosure of Confidential Information is allowed unless written permission is granted by Brentwood'. Purchaser agrees not to use the Confidential Information in any manner to Brentwood's detriment, including without limitation, to reverse engineer, disassemble, analyze, decompile, copy, modify, develop, or design.

#### Force Majeure:

Brentwood shall not be liable or responsible to Purchaser, nor be deemed to have defaulted under or breached this Agreement, for any failure or delay in fulfilling or performing any term of this Agreement to the extent Brentwood's failure or delay is caused by or results from a force majeure event, including, acts of God; flood, fire, earthquake, pandemics, disease outbreaks, explosions or other natural disasters; war, invasion, hostilities, terrorist acts, civil unrest; government orders or actions; embargoes or blockades in effect on or after the date of this Agreement; national emergency; strikes, labor stoppages or slowdowns, or other industrial disturbances; shortage of adequate raw materials, labor, power, or transportation facilities; and other similar events beyond the reasonable control of Brentwood.

Brentwood shall give notice within fourteen (14) days of the force majeure event or as soon as reasonably practicable to Brentwood, stating the period of time the occurrence is expected to continue. Brentwood shall use diligent efforts to end the failure or delay and ensure the effects of such are minimized. Brentwood shall resume the performance of its obligations as soon as reasonably practicable after the removal of the cause. In the event Brentwood remains unable to perform its obligations within ten (10) weeks from notice of force majeure event Purchaser may terminate the Agreement.

#### Governing Law and Jurisdiction:

Phone: 610.374.5109

This Agreement shall be construed under the laws of the Commonwealth of Pennsylvania without reference to conflicts of law principles. The Parties hereby agree that disputes hereunder shall be subject to the exclusive jurisdiction and venue of the courts of Berks County, Pennsylvania, in either the Pennsylvania Court of Common Pleas or the United States District Court for the Eastern District of Pennsylvania. The Purchaser waives any objections based on personal or subject matter jurisdiction or venue.

#### **Export Control:**

Purchaser will not use, distribute, transfer, or transmit any Product, components or technical information (even if incorporated into other products) provided in connection with this transaction except in compliance with U.S. export laws and regulations (the "Export Laws"). Purchaser will not, directly or indirectly export or re-export the following items to any country which is in the then-current list of prohibited countries specified in any applicable Export Laws"). Purchaser will not, directly or indirectly export or re-export the following items to any country which is in the then-current list of prohibited countries specified in any applicable Export Laws: (a) the Product, components or technical data disclosed or provided to Purchaser by Brentwood; or (b) any improvements or variations of such Product, components or technical data disclosed or provided to Export Laws: (a) the Prometry inform Brentwood in writing of any written authorization issued by the U.S. Department of Commerce office of export licensing to export re-export any such items referenced in (a) or (b). The obligations stated above in this clause will survive the expiration, cancellation or termination of this Agreement.

#### Translation:

This document may be translated into one or more languages; however, the English translation shall be the official version and shall prevail over other translations. All dollar amounts are United States currency unless specified otherwise. Purchaser shall abide by the United States Foreign Corrupt Practices Act of 1997, as amended.

#### Assignment:

Purchaser shall not assign or delegate its obligation hereunder without Brentwood's written consent, and any attempted assignment or delegation without such written consent shall be void.

#### Waiver:

No waiver by Brentwood of any of the provisions of this Agreement is effective unless explicitly set forth in writing and signed by Brentwood. No failure to exercise, or delay in exercising, any right, remedy, power or privilege arising from this Agreement operates, or may be construed, as a waiver thereof. No single or partial exercise of any right, remedy, power or privilege hereunder precludes any other or further exercise thereof or the exercise of any other right, remedy, power or privilege.





#### Severability:

If any term or provision of this Agreement is invalid, illegal or unenforceable in any jurisdiction, such invalidity, illegality or unenforceability shall not affect any other term or provision of this Agreement or invalidate or render unenforceable such term or provision in any other jurisdiction.

#### Notices:

All notices, requests, consents, claims, demands, waivers and other communications hereunder (each, a "Notice") shall be in writing and addressed to the Parties at the addresses set forth on the face of the Proposal or to such other address that may be designated by the receiving Party in writing. All Notices shall be delivered by personal delivery, nationally recognized overnight courier (with all fees prepaid or certified or registered mail (in each case, read receipt requested, postage prepaid). Except as otherwise provided in this Agreement, a Notice is effective only (a) upon receipt of the receiving Party, and (b) if the Party giving the Notice has complied with the requirements of this Section.

#### Authority:

The individual assenting to or executing any documents or orders, whether as a hard copy or, on behalf of Purchaser acknowledges, represents and warrants that he or she has read and understands these Terms and Conditions and has been duly authorized by the Purchaser to execute such on behalf of the Purchaser and bind the Purchaser to these Terms and Conditions.

#### Relationship of the Parties:

The relationship between the Parties is that of independent contractors. Nothing contained in this Agreement shall be construed as creating any agency, partnership, joint venture or other form of joint enterprise, employment or fiduciary relationship between the Parties, and neither Party shall have authority to contract for or bind the other Party in any manner whatsoever.

#### Survival:

Provisions of this Agreement which by their nature should apply beyond their terms will remain in force after any termination or expiration of this Agreement.

#### Amendment and Modification:

This Agreement may only be amended or modified in writing by Brentwood and executed by an authorized representative of each Party.

By signing below both Parties accept Brentwood Water Group (Water and Wastewater) Standard Terms and Conditions of Sale.

**BRENTWOOD INDUSTRIES, INC.** 

By: \_\_\_\_\_\_ Print Name: \_\_\_\_\_\_ Title: \_\_\_\_\_\_ Brentwood Industries, Inc.

PURCHASER	
Ву:	
Print Name:	
Title:	
Company:	





Item 6: Membrane Bioreactor



# Budget Proposal for the Centreville, MD MBR ZeeWeed Membrane Bioreactor System

Submitted to: Sherwood-Logan & Associates Andrew Kreider (603) 848-3950 akreider@sherwoodlogan.com

June 29th, 2023

Veolia Proposal Number: 556484

Submitted by: **Graham Best-** Regional Manager Tel: (905) 465-3030 Ext. 3209 Email : graham.best@veolia.com



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## Veolia Water Technologies & Solutions Confidential and Proprietary Information

The enclosed materials are considered proprietary property of Veolia Water Technologies & Solutions (Veolia). No assignments either implied or expressed, of intellectual property rights, data, know-how, trade secrets or licenses of use thereof are given. All information is provided exclusively to the addressee and agents of the addressee for the purposes of evaluation and is not to be reproduced or divulged to other parties, nor used for manufacture or other means, without the express written consent of Veolia. The acceptance of this document will be construed as an acceptance of the foregoing.

\*The following are trademarks of Veolia Water Technologies & Solutions and may be registered in one or more countries: InSight, LEAPmbr, Z-MOD, ZeeWeed, and ZENON



# 1 Introduction to ZeeWeed Membrane Bioreactor (MBR) Technology

The proposed ZeeWeed Membrane filtration system for the Centreville, MD MBR is designed to ensure reliable long-term performance and to maximize operational flexibility. At the core of the MBR process is the ZeeWeed 500 series hollow fiber membrane. The ZeeWeed 500 series membrane is a reinforced hollow fiber ultrafiltration membrane that was designed specifically for high solids applications. The membrane fiber has a nominal pore size of 0.04 μm, a tensile strength of 135 lbs, (vs 3 lbs for non-reinforced fibers) and is highly resistant to chemicals, including acids, bases and chlorine, allowing for flexible cleaning regimes. The membrane material is both mechanically and chemically bonded to the porous supporting braid that provides the mechanical strength. This doublebonding means that the membrane will never separate from the braid. The relatively thin layer of membrane layer is the key to ensuring long-term permeability. Some membranes attempt to make up for their lack of a reinforced braid with a thicker membrane wall. The increase in strength is only marginal compared to a thinner non-reinforced fiber, and is still orders of magnitude less than a reinforced fiber. The cost of this approach comes from the tendency to trap organics and colloidal material inside the membrane material, rendering them permanently fouled.

The membrane is manufactured and assembled into discrete units called "modules" or small membrane subunits. These are the basic building blocks of the membrane system that are manifolded together to create a "cassette" or large membrane subunit. The cassette, proposed for the **Centreville, MD MBR**, is 52M ZeeWeed 500D cassette and each module in the cassette has 430 ft<sup>2</sup> of membrane area.



ZeeWeed 500 Cassette

In the ZeeWeed membrane filtration process, the membrane cassettes are immersed directly in the mixed liquor. A series of cassettes connected to a common permeate header is called a "membrane train". Each membrane train is connected to the suction side of a duty

pump for permeation. The pump creates a slight vacuum in the permeate header to draw treated water from the outside in through the hollow fiber membranes, leaving the

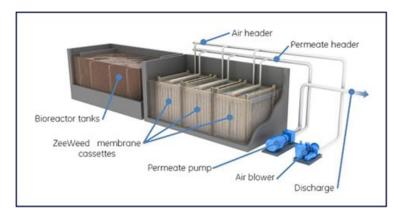
mixed liquor solids on the outside of the membrane. Permeate is then directed to downstream disinfection or discharge facilities. Air, in the form of large bubbles, is introduced below the bottom of the



ZeeWeed Membrane Trains and Cassette

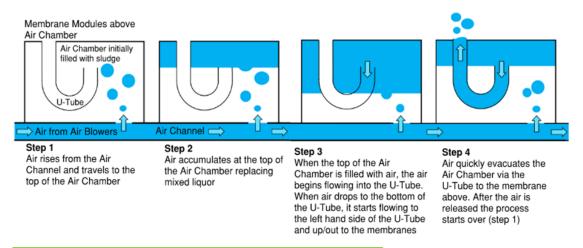
membrane modules, producing turbulence that scours the outer surface of the hollow fibers to keep them clean.





The proposed system design utilizes LEAPmbr Aeration, SUEZ's latest aeration technology for MBR systems. A specially designed and highly efficient aeration system is used to scour the outside surface of the membranes and move feed water solids away from the membrane fibers. LEAPmbr Aeration Technology uses factory installed aerators which are integrated into the base of each ZeeWeed 500 cassette.

LEAPmbr aerators use no moving parts within the membrane aeration system. A single air pipe and a single permeate pipe (per membrane train) provide the connection between the immersed membranes and the permeate pumps and blowers that comprise the rest of the ZeeWeed system. LEAPmbr aeration greatly simplifies the aeration system and reduces air requirements for the system.



LEAPmbr Aeration – Simple Energy Efficient Aeration

LEAPmbr aeration provides the most intense air scour, which is ideal for removing solids from a membrane bundle. This is ONLY possibly because of the increased strength that comes from the reinforcing braid. A single monofilament fiber with no supporting braid is cheaper to manufacture, but it cannot survive the mechanical stress of a large-bubble, high-shear air scour device. Similarly, flat plate and similar modified plates will not allow for the free movement of fibers



and the passage of large bubbles, which results in a tendency to sludge in unrecoverable fashion.

The combination of a robust, high-strength fiber and large-bubble air scour are the keys to long-term performance stability.

## **1.1 Benefits of Veolia System Design**

At Veolia, our goal is to create long term partnerships with our customers, which is why we design our systems with you in mind. Our approach to the proposed ZeeWeed membrane bioreactor system has been optimized around the following three key system attributes.

- robust design proven design parameters with scope and configuration options for a wide variety of conditions
- simple operations simple & automated operations coupled with Veolia support for the operating team
- lowest cost of ownership for the Owner

We are continuously striving to improve our system designs to provide optimal solutions for our customers. Highlighted below are several systems that we have optimized to meet your needs.

## 1.1.1 Pre-Engineered Z-MOD L Process Pump Skid

The Z-MOD L process pump skid is a pre-engineered equipment skid that helps simplify ZeeWeed membrane filtration system design and installation. The Z-MOD L skid is a



"plug and go" skid that incorporates most of dedicated membrane train equipment onto a single prefabricated equipment skid for simple onsite installation.

The Z-MOD L skid is designed to handle all membrane train flow conditions and includes a bi-directional process pump that performs both permeation and backpulse duty. A traindedicated remote I/O panel is installed on the Z-MOD L skid, with all skidded equipment and instrumentation pre-wired and tested within the panel.

## **1.1.2 Membrane Aeration System Design**

Aeration is one of the most important operating parameters for successful long term MBR operations and is a significant component of operating cost.

Veolia MBR system utilizes a very simple aeration strategy which minimizes the amount of instrumentation and controls required to achieve energy efficient membrane aeration.

No complex control loops or complicated airflow measurement devices are required for LEAPmbr aeration technology to achieve energy efficiency.



## **1.1.3 Membrane Cleaning Systems**

Veolia has developed membrane design principles based on best engineering practices that ensure the permeability of the membrane is maintained over the life of the membranes.

A fully automated suite of membrane maintenance procedures will ensure long-term, successful operation, including:

- in-situ chemical membrane cleaning performed directly in the membrane process tanks so your operators don't waste time moving cassettes.
- the ability to increase or decrease the frequency of chemical cleans to fit the operating conditions.
- the ability to backpulse, when needed, to greatly improve your operator's ability to recover from non-design conditions.

The above cleaning systems can be automated, resulting in operators having available a full suite of comprehensive cleaning systems which are simple to use and initiate.



# 2 Design

The proposed ZeeWeed membrane filtration system for the **Centreville, MD MBR** is offered based on using the design parameters summarized in the following sections.

## 2.1 Influent Flow Data

The influent design flows are summarized in the table below.

Flow Conditions <sup>1</sup>	Capacity	Units
Average day flow (ADF)	0.75	MGD
Max month flow (MMF) <sup>1</sup>	0.93	MGD
Max Day flow (MDF) <sup>1</sup>	2.25	MGD
Peak hour flow (PHF)	2.5	MGD
Maximum flow with one train offline for maintenance or cleaning (less than 24 hours)	2.25	MGD

**Note 1:** Any flow conditions that exceed the above-noted flow limits should be equalized prior to treatment in the ZeeWeed membrane filtration system.

Note 2: The flow definitions as seen in the table above are as follows:

- ADF the average flow rate occurring over a 24-hour period based on annual flow rate data.
- MMF the maximum monthly flow rate sustained less than one month period based on annual flow rate data.
- MDF the maximum daily flow rate sustained over a 24-hour period based on annual flow rate data.
- PHF the maximum flow rate sustained over a 2-hour period based on annual flow rate data.

## 2.2 Influent Quality

The design solution proposed is based on the wastewater characteristics detailed below. The concentrations listed below are specific to the flow used for the biological design as listed in Section 2.1 below.

Influent Design Parameters	Value	Unit
design influent temperature	10	°C
BOD₅	175	mg/L
TSS	200	mg/L
inert solids fraction <sup>1</sup>	20	%
NH3-N	28 <sup>1</sup>	mg/L
TKN	40	mg/L
ТР	8	mg/L
Alkalinity <sup>1,2</sup>	250	mg/L as CaCO <sub>3</sub>

Note 1: Parameter value assumed.

**Note 2:** Veolia is assuming that sufficient influent alkalinity is available for the proper performance of the biological system. Should influent alkalinity not be sufficient, chemical addition by the buyer will be required.

## 2.3 Effluent Quality

The following performance parameters are expected upon equipment startup and once the biological system has stabilized based on the data listed in Sections 1.1 and 1.2.



Effluent Design Parameters	Value	Unit
BOD <sub>5</sub>	≤ 5	mg/L
TSS	≤ 5	mg/L
NH <sub>3</sub> -N	≤ 1	mg/L
TN <sup>1</sup>	≤ 3	mg/L
ТР	≤ 0.3	mg/L
turbidity	≤ 1	NTU

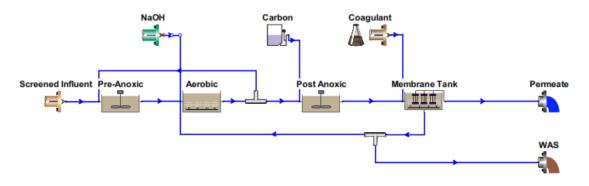
Note 1: TN  $\leq$  3 mg/L corresponds to a minimum design temperature of 10°C and < 0.1 mg/L recalcitrant dissolved organic nitrogen in the influent.

## 2.4 Influent Variability

Influent wastewater flows or loads in excess of the design criteria defined above should be equalized prior to entering the membrane tanks. In the event that the influent exceeds the specifications used in engineering this proposal, or the source of influent changes, the ability of the treatment system to produce the designed treated water quality and/or quantity may be impaired. Buyer may choose to continue to operate the system but assumes the risk of damage to the system and/or additional costs due to increased membrane cleaning frequency, potential for biological upset and/or increased consumables usage.

## 2.5 Biological System Design

For the **Centreville, MD MBR** project, the screened influent wastewater first enters the pre-anoxic tank for denitrification and alkalinity recovery. Mixed liquor will then be transferred to aerobic tanks, where BOD is oxidized and most of NH3-N is converted into NO3-N. The post-anoxic tank is set up with organic carbon dosing for further denitrification. Finally, the mixed liquor enters the membrane tanks where biomass is separated from the mixed liquor by the ZeeWeed 500 membranes. The coagulant will be dosed to the MBR system for further TP removal to meet the TP effluent target. The flow sheet is shown below.



A permeate pump draws permeate through the membrane which is then pumped through a disinfection system by others.



Waste sludge is diverted from the RAS line to the sludge holding tank (by others). The frequency of wasting is a function of influent characteristics, reactor design and operator preferences.

The following table is a summary of the biological design.

Biological Design Parameters	Value	Unit
flow basis for biological design	0.93	mgd
total pre-anoxic tank working volume	70,000	gal
total aerobic working volume	280,000	gal
Total post-anoxic tank working volume	110,000	gal
total bioreactor working volume (excluding membranes)	460,000	gal
total design HRT (including bioreactors and membrane tanks)	11.9	hours
aerobic design SRT (excluding membrane tanks)	13	days
waste sludge removal (based on MMF and 10 g/L)	18,000	gpd
design MLSS concentration in bioreactor	≤ 8,000	mg/L
Alum addition	150	gpd
Methanol Addition <sup>1</sup>	40	gpd
design liquid depth in bioreactor	18	ft

Note 1: Alternate carbon sources can be utilized such as Micro-C.

## 2.6 Membrane System Design

Membrane Design Parameters	Design
Number of membrane trains	3
Number of ZMODL skids	3
Number of cassette spaces per train	3
Number of cassettes installed per train	3
Type of cassette (modules per cassette)	ZeeWeed 500D, 430 ft <sup>2</sup> , 52M
Module design per train	(1x52) + (2x40)
Total number of modules installed per train	132
Total number of modules installed per plant	396
Total number of cassettes installed per plant	9
Spare space	33.3%
Membrane tank internal dimensions (one train) L x W x H (ft)	21.7' × 9' × 13'

Note 1: Tank dimensions and volumes are preliminary only and may change slightly once final detail design commences.

Note 2: The ultrafiltration system is designed for installation within concrete tanks supplied by buyer.



# 3 Scope of Supply

## 3.1 Scope of Supply by Veolia

The following table provides a summary of the main equipment included with the supply of the ZeeWeed MBR System.

Quantity	Description <sup>(1)</sup>
Membrane	Blower & Associated Equipment
3+1	Inlet filters and silencers
3+1	PD membrane blowers
3+1	Sound enclosures
3+1	Discharge silencers
3+1	Discharge pressure relief valves
3+1	Discharge pressure indicators c/w isolation valves
3+1	Discharge check valves
3+1	Discharge flexible connectors with clamps
3+1	Blower discharge low flow switches
3+1	Membrane blower isolation valves
Process B	ower & Associated Equipment
2+1	Inlet filters and silencers
2+1	PD membrane blowers
2+1	Sound enclosures
2+1	Discharge silencers
2+1	Discharge pressure relief valves
2+1	Discharge pressure indicators c/w isolation valves
2+1	Discharge check valves
2+1	Discharge flexible connectors with clamps
2+1	Blower discharge low flow switches
2+1	Membrane blower isolation valves
Biological	Equipment
2	Pre-anoxic mixers – 1 per tank
2	Post-anoxic mixers – 1 per tank
2	Fine bubble system for process aeration - loose shipped (with tank downcomer piping, 2 aerobic zones)
2	Submersible RAS pumps, used to transfer mixed liquor from the aerobic tanks to the pre-anoxic tanks - including isolation valves and associated instruments
2	Biological tank controllers, each with associated one (1) pH sensor and one (1) DO sensor
MBR ZeeW	leed Membrane & Associated Equipment
1 lot	Membrane cassette installation assemblies



9	ZeeWeed 500D 52-module membrane cassettes
396	ZeeWeed 500D 430 ft <sup>2</sup> membrane modules
6	Membrane tank level switches
3	Membrane tank level transmitters
3	Pressure transmitters
3	Ejector assemblies
Permeate	Pump Skid (L1120)
3	Membrane equipment skids – epoxy-coated carbon steel Each skid includes: • One (1) permeate pump – reversible rotary lobe pump • One (1) magnetic flow meter • Two (2) pressure gauges • One (1) turbidity probe • One (1) RIO panel • Associated piping and valves
Backpulse	e System
-	Permeate pumps will also provide backpulse duty
1	Backpulse tank and associated level transmitter and valves
1	Temperature transmitter on the common permeate discharge line
RAS Pum	ps & Associated Equipment
3+1	RAS pump suction isolation valves
3+1	RAS pump suction pressure gauges w/hand isolation valves
3+1	RAS pump suction drain valves
3+1	Centrifugal RAS pumps
3+1	RAS pump discharge pressure gauges w/hand isolation valves
3+1	RAS pump discharge check valves
3+1	RAS pump discharge drain valves
3+1	RAS pump magnetic flow meters
3+1	RAS pump discharge isolation valves
1	<ul> <li>Sludge wasting system, including</li> <li>One (1) on/of automatic valve</li> <li>One (1) magnetic flow meter</li> <li>One (1) isolation valve</li> </ul>
Process C	hemical Dosing System
1	Skid-mounted sodium hydroxide dosing system, including 1+1 chemical dosing pumps, and associated valves, instruments, and piping
1	Movable level switch for sodium hydroxide day tank
1	Skid-mounted coagulant dosing system, including 1+1 chemical dosing pumps, and associated valves, instruments, and piping
1	Movable level switch for coagulant day tank
1	Skid-mounted carbon dosing system, including 1+1 chemical dosing pumps, and associated valves, instruments, and piping



1	Movable level switch for carbon day tank		
Membran	e Cleaning System		
1	Skid-mounted sodium hypochlorite dosing system, including 1 chemical dosing pump, and associated valves, instruments, and piping		
1	Movable level switch for sodium hypochlorite day tank		
1	Skid-mounted citric acid dosing system, including 1 chemical dosing pump, and associated valves, instruments, and piping		
1	Movable level switch for citric acid day tank		
1	Common shelf spare chemical dosing pump		
Compres	sed Air System		
1+1	Air compressors, each compressor mounted on a horizontal/vertical receiver tank		
	Compressed air assembly (loose shipped) includes:		
	one (1) coalescing filter		
1	one (1) low air pressure switch		
I	one (1) pressure regulator		
	one (1) low-low air pressure switch		
	associated valves		
1+1	Refrigerated air driers and associated valves		
Electrical	and Control Equipment		
1	Main control panel (MCP, NEMA12) with Allen Bradley PLC and touch screen HMI		
Miscellan	eous		
1	Membrane cassette lifting bracket		
General			
Incl.	Equipment general arrangement and layout drawings		
Incl.	Operating & maintenance manuals		
Incl.	Field service and start-up assistance <sup>(2)</sup> - 40 days support over 4 site visits from Veolia Water field-service professionals for commissioning, plant start- up/commissioning, and operator training		
Incl.	24/7 emergency phone support – 1 year		
Incl.	Veolia insight Basic on-line monitoring service – 1 year		
Incl.	Equipment mechanical warranty – 1 year		
Incl.	Membrane warranty – 10-year pro-rated membrane warranty (2-year full replacement warranty and the following 8-year pro-rated membrane warranty)		
Notes:			

Notes:

1) All Veolia-supplied equipment is designed for installation in an unclassified area except specified otherwise.

2) Additional field service hours will be billed separately from the proposed system capital cost at a rate plus living and traveling expenses. Detailed Veolia service rates are available upon request.

## 3.2 Scope of Supply by Others

The following items are for supply by buyer and will include, but are not limited to:

- Overall plant design responsibility
- Installation on site of all Veolia-supplied skids and loose-shipped equipment



- review and approval of design parameters related to the biological process and membrane separation system
- Review and approval of Veolia supplied equipment drawings and specifications
- Detail drawings of all termination points where Veolia equipment or materials tie into equipment or materials supplied by others
- Equipment foundations, civil work, full floor coverage equipment contact pads, buildings, etc.
- Receiving, unloading and safe storage of Veolia-supplied equipment at site until ready for installation
- HVAC equipment design, specifications and installation (where applicable)
- UPS, Power Conditioner, Emergency power supply and specification (where applicable)
- Lifting devices including crane able to lift 10,000 lbs for membrane removal, lifting davits, hoists and guide rails for submersible mixers and pumps, etc.
- MCC, VFDs, or starters for 3-ph motors, including loose ship Veolia-supplied equipment
- □ 2mm opening fine screen
- Equalization tank and associated equipment as required
- Influent pumps and associated valves and instrument
- Biological and membrane tanks
- All chemical storage tanks, day tanks, and containment
- Treated water storage tank as required
- Process and utilities piping, pipe supports, hangers, valves, etc. including but not limited to:
  - piping, pipe supports and valves between Veolia-supplied equipment and other plant process equipment
  - piping between any loose-supplied Veolia equipment
  - process tank aeration system air piping, equalization tank system piping, etc.
- Interconnecting pipe between Veolia-supplied skids and tanks (as applicable)
- □ Electrical wiring, conduit and other appurtenances required to provide power connections as required from the electrical power source to the Veolia control panel and from the control panel to any electrical equipment, pump motors and instruments external to the Veolia-supplied enclosure
- Suitable, secure remote internet connection for 24/7 emergency telephone technical support service and InSight remote monitoring & diagnostics service
- All bolts, brackets and fasteners to install Veolia-supplied equipment. Seismic structural analysis and anchor bolt sizing



- Alignment of rotating equipment
- Lubricant oil for all rotating equipment
- Raw materials, chemicals, and utilities during equipment start-up and operation
- Supply of seed sludge for biological process start-up purposes
- Disposal of initial start-up wastewater and associated chemicals
- □ Weather protection as required for all Veolia supplied equipment. Skids and electrical panels are designed for indoor operation and will need shelter from the elements.
- Laboratory services, operating and maintenance personnel during equipment checkout, start-up and operation
- □ Touch up primer and finish paint surfaces on equipment as required at the completion of the project
- All permits



# 4 Commercial

## 4.1 System Pricing

Pricing for the proposed equipment and services, as outlined in Section 3, is summarized in the table below. All pricing is based on the design operating conditions and influent characteristics detailed in Section 1. The pricing herein is for budgetary purposes only and does not constitute an offer of sale. No sales, consumer use, or other similar taxes or duties are included in the pricing below.

Price: All Equipment & Service	
Z-MOD-L Membrane Bioreactor System, as per Section 3.1	\$2,780,000 USD

## 4.2 Freight Terms

The following freight terms used are as defined by INCOTERMS 2020.

All pricing is CIP project site.

## 4.3 Equipment Shipment and Delivery

Veolia has provided a timeline for the major milestones below. The buyer and seller will arrange a kick-off meeting after contract acceptance to develop a firm shipment schedule.

- Seller: Shop Drawing Package 12-16 weeks after the PO is accepted
  - Partial submittals recommended
    - P&IDs
    - Mechanical (includes Bill of Material, cut sheets, membrane tank GA)
    - Electrical
- Seller: Shipment of Equipment 40-52 weeks from NTP with Manufacture of Equipment (partial shipments allowed)
- Seller: Shipment of Membranes Membranes will ship immediately prior to their installation on-site and commissioning

## 4.4 Terms and Conditions of Sale

This proposal has been prepared and is submitted based on the seller's standard terms and conditions of sale.



Item 7: Denitrifying Filters



# **Budget Proposal**

# WWTP

Centreville, MD



prepared for:

Centreville, MD

5/2/2023



Xylem Water Solutions USA, Inc. 108 Tomlinson Dr Zelienople, PA 16063 Mr. Chris Ball Direct: 724-453-2109 Mobile: 724-713-7145 Email: chris.ball@xylem.com

5/2/2023

## Project name : Centreville, MD WWTP Project number : I23178

To Whom It May Concern,

Based on your inquiry, we are pleased to forward the following proposal to your attention. Thank you for the opportunity to offer our equipment and services for the Centreville, MD WWTP.

We hope that our proposal comes up to your expectation. If you have any questions please do not hesitate to contact us.

Respectfully,

Chris Ball Senior Sales Engineer



# **1 Xylem Overview**

Xylem is a leading global water technology provider, enabling customers to transport, treat, test and efficiently use water in public utility, residential and commercial building services, industrial and agricultural settings. The company does business in more than 150 countries through a number of market-leading product brands, and its people bring broad applications expertise with a strong focus on finding local solutions to the world's most challenging water and wastewater problems.



Xylem's treatment business offers a portfolio of products and systems designed to effectively meet the demands and challenges of treating water and wastewater. From smarter aeration to advanced filtration to chemical-free disinfection, Xylem leverages its well-known Treatment brands, Flygt, Leopold, Sanitaire, and Wedeco, to offer hundreds of solutions backed by a comprehensive, integrated portfolio of services designed to ensure we can meet our customers' needs in a number of different industries including municipal water and wastewater, aquaculture, biogas and agriculture, food and beverages, pharmaceuticals, and mining.

Our scientists and engineers utilize their deep applications expertise and continually listen and learn from our customers' situations to create solutions that not only use less energy and reduce life-cycle costs, but also promote the smarter use of water.

Leopold has long been a worldwide leader in the water and wastewater treatment industry



supplying both filtration and clarification systems. Leopold both designs and supplies systems for gravity filtration, clarification, denitrification, sludge collection and backwash water recovery. Leopold solutions are ideal for algae, contaminant, and nutrient removal, desalination pretreatment, reuse, SDI, and taste and odor reduction. Since its establishment in 1924, Leopold has pioneered

and acquired a number of innovative technologies aimed at improving the quality of water while reducing costs. With over 8,000 installations, customers from around the world have come to rely on Leopold's expertise and technological leadership in water and wastewater treatment.

Since 1924 Leopold has been designing and manufacturing rapid gravity media filtration and clarification solutions for treating water and wastewater.





Leopold supplies potable drinking water treatment plants with media filtration, backwash water recovery, reuse and desalination pretreatment solutions, while supplying wastewater treatment plants with tertiary filtration and denitrification solutions. They also supply both potable and wastewater treatment plants with dissolved air flotation (DAF) clarification, ozone enhanced biologically active filtration systems and sludge collection solutions.

Leopold engineers are available to help analyze, evaluate and design all aspects of a complete filtration system, including evaluating influent water qualities, determining optimal loading rates and best design configuration, selecting the best media characteristics, and designing the backwash process. Xylem's Leopold Filterworx performance filter system comes complete with flume, underdrains, integral media support, engineered media, backwash water troughs, and system controls. The result is a cost effective, efficient, high-performance system designed to meet customer requirements.





Leopold also offers sludge collection solutions with the Clari-VAC floating sludge collector and the CT2 submerged sludge collector. These systems are used in final clarifiers to remove the sludge solids. For those areas where nitrogen and phosphorus removal is required, Leopold provides elimi-NITE denitrification systems which convert the filters to become biologically active so that the effluent meets the mandated nitrate and phosphorus levels.

For more information please visit us on our homepage:

http://www.xylem.com/treatment/us/brands/leopold





# **2** General Process Description

## 2.1 PROCESS DESCRIPTION

## elimi-NITE® Denitrification System General Process Description

The elimi-NITE Denitrification System is an attached growth, microbiological process. This gravity, downflow, packed-bed denitrification system is physically identical to a deep-bed downflow sand filter. Denitrifying microorganisms attach to the filter media, which provides the support system for their growth. A carbon source such as methanol, acetic acid, molasses, etc. is added upstream of the packed-bed filter and a nitrified influent is filtered through the media. The packed-bed filter system is well suited for denitrification because it provides the necessary hydraulic detention time for the biological reaction to take place. The filter media is composed of a coarse, hard, predominately siliceous material. This media can filter out solids and serve as a support system for the denitrifying microorganisms. The downflow packed-bed system eliminates the requirement for downstream filtration or clarification required of other denitrification systems.

As denitrification occurs, nitrogen gas accumulates in the filter media, which increases the headloss over the headloss due to the accumulation of solids. The nitrogen gas bubbles are periodically released from the media by taking the filter off line and applying backwash water for a few minutes. This process is called the nitrogen release cycle or filter bumping. The frequency of the nitrogen release cycle is a function of both nitrate removal and a minimum acceptable time between cycles, typically less than one hour. Usually a filter needs to be bumped once every four to eight hours, again depending on the nitrogen loading rate. The bumps are usually set on a time basis. After a bump the headloss in the filter is reduced or recovered. However, when the liquid level in the filter reaches a designated high level, signifying that the bumps are not effective in reducing headloss, a full backwash is performed on the filter.

The elimi-NITE Denitrification System is comprised of the following basic principles:

- A packed deep-bed layer of sand for biomass attachment and retention of suspended solids
- A Leopold Universal<sup>®</sup> Type S<sup>®</sup> Filter System for distribution of air and water for superior backwashing of the elimi-NITE filter module.
- A complete chemical feed system of the carbon source for denitrification (future)
- Automated backwash sequence and controls optimized for each applications requirement utilizing Leopold FilterWorx<sup>™</sup> Control System.

The full backwash consists of the following sequence:

- Influent and effluent valves are closed
- Waste valve is opened
- Blower is started
- Air isolation valve is opened, vent valve is closed and air only wash for approximately one minute
- Backwash pump is started
- Backwash isolation valve is opened and air/water backwash for approximately 15 minutes
- Air isolation valve is closed, vent valve is open and the blower is stopped





- Water only backwash continues for approximately 5 minutes to purge air from the filter
- Backwash isolation valve is closed and the backwash pump is stopped
- Waste valve is closed
- Influent and effluent valves are opened

Gases such as nitrogen or dissolved oxygen will build-up high levels in the filter and cause air binding. In this case the filters are water-only "bumped." The bump consists of isolating the filters from the influent flow, closing the effluent valve, starting the backwash pump, opening the backwash valve, opening the waste valve (optional if the water depth stays below the effluent launder) and backwashing the filter for approximately 2-5 minutes. This reversal of flow allows the built-up gases to escape the filter. The filter is then put back on-line. The bumps can be programmed to occur either on time or on level and are site specific.





# **3** Technical Description

## 3.1 DESIGN CRITERIA:

The elimi-NITE Denitrification System described here-in is a wastewater treatment system designed for the removal of nitrate-nitrogen.

The elimi-NITE Denitrification System that shall be furnished and installed is described in Section 3.2 - Scope of Supply.

The system has been designed based on Leopold's standard specifications using the following criteria:

Plant Flow	MGD
AAF MMF PHF	1.00 1.20 3.30

## Note: Please define the following parameters to help optimize the denitrification process.

The elimi-NITE Denitrification System is based on treating the influent the filters with the following characteristics:

Influent Parameter	mg/L	Given	Assumed
Tatal Supported Calida (TSS)	20		
Total Suspended Solids (TSS)	30	X	
Nitrates	8.0	Х	
N-Ammonia			
Minimum Water Temperature (°C)	12	Х	

The elimi-NITE Denitrification System is designed to achieve the following monthly average effluent quality:

Effluent Parameters	mg/L	Given	Assumed
Total Suspended Solids (TSS)	<5.0		Х
Nitrates	1.0	Х	
Total Nitrogen	3.0		Х

The external carbon source for the elimi-NITE Denitrification System that will be provided by others is methanol.





If Phosphorous removal is required, the phosphorous must be in an insoluble form. This may require the use of coagulants upstream of the Filtration System.

## Elimi-NITE Denitrification System Design Criteria

<b>Total number of filters</b>	Three (3)
Active Filtration volume	2,592 ft <sup>3</sup>
Total filtration area	432 ft <sup>2</sup>
Individual filter sizing Area Length Width Media Depth Media volume	144 ft <sup>2</sup> 12'-0" 12'-0" 72" 864 ft <sup>3</sup>

## Media Type

Coarse Silica Sand 1/8" x No.12 – 72"

Loading Rates	Filter Loading Rate (Three) with one	e in backwash (Two)
At 1.00 MGD (AAF)	1.61 gpm/ft <sup>2</sup>	2.41 gpm/ft <sup>2</sup>
At 1.20 MGD (MMF)	1.93 gpm/ft <sup>2</sup>	2.89 gpm/ft <sup>2</sup>
At 3.30 MGD (PHF)	5.31 gpm/ft <sup>2</sup>	7.96 gpm/ft <sup>2</sup>
Backwash Rates Design concurrent w Design concurrent ai Design high water ra	r rate	6 gpm/ft <sup>2</sup> 5 scfm/ft <sup>2</sup> 6 gpm/ft <sup>2</sup>
Designed Driving Head		8'-0"

## 3.2 SCOPE OF SUPPLY

Xylem Water Solutions USA, Inc will supply only the items specifically detailed within this proposal.

**Filter Internals:** 





- Three (3) **Complete elimi-NITE filters**, 432 square feet effective filtration area total, 12'-0" by 12'-0" each inside filter dimensions utilizing a front flume arrangement including:
- 432 square feet Leopold Universal® Type XA® Underdrain of the Dual/Parallel Lateral type, manufactured from corrosion resistant, high-density polyethylene supplied with necessary "O"-rings and carbon steel "L" anchor rods and clips. Epoxy, sealant, bonding agents, or other similar materials used during installation are not included and to be provided by others.
- 432 square feet **I.M.S® 1000 MEDIA RETAINER** will be furnished. The scope includes molded thermoplastic I.M.S® 1000 media retainer factory installed onto the proposed underdrain block prior to shipment.
- Three (3) sets **Air Header Assemblies** shall be manufactured from schedule 10, type 304 stainless steel pipe. The air header pipe shall measure 6" in diameter and will run the width of the filter cell. The air header shall commence with a flange approximately 6" inside the filter cell. Mating flange and hardware is to be supplied by others. The air header pipe will have j-risers to provide air to each of the individual filter laterals.
- Six (6) **WASH TROUGHS:** Under this section, we propose to furnish six (6) Leopold Reinforced Fiberglass Troughs, Leo-Lite No. 87, measuring 12" wide x 12" deep x 12'-0" long, round bottom construction. Also included is the standard end hanger assembly fabricated from type 316 stainless steel and type 18-8 stainless steel hardware. Also included with the above troughs are reinforced fiberglass matched-die straight edge weir plates attached to the troughs with type 18-8 stainless steel fasteners. Also included shall be type 304 stainless steel stabilizers for stabilization of wash water troughs. Wash troughs shall have one closed end and one open discharge end with waterstop.

#### Media:

2,592 cubic feet

Coarse Silica Sand – 72" Depth Effective Size: 1/8" x No. 12 134 Tons

### FilterWorx<sup>™</sup> Control System:

Under this section, we propose to furnish the following FilterWorx<sup>™</sup> Automatic Control System for the subject project for controlling the filtration and backwashing operations of three (3) filters. The system will consist of the following equipment:

Three (3) **Leopold model AFC-5000 Single Filter Control Panels.** The panels shall be housed in a NEMA 4X rated, 316 stainless steel enclosure. The panels shall include provisions for the automatic, semi-automatic, and manual control of the filtration and backwashing operations of one (1) filter. Logic functions shall be performed by an Allen Bradley Compact Logix Series PLC. Manual operation shall be independent of the PLC. Operator interface shall be via an Allen Bradley Panelview Plus 1000 touchscreen and Square D type ZB4 selector switches, pushbuttons and pilot lights.





- Three (3) Siemens Hydroranger 200 Ultrasonic filter level transmitters
- One (1) Siemens Hydroranger 200 Clearwell Level Transmitter
- One (1) Siemens Hydroranger 200 Mudwell Level Transmitter
- Two (2) Hach Nitratax Sensors and SC1000 Controllers (One Influent & One Effluent)
- One (1) Hach Dissolved Oxygen Sensor
- Two (2) Hach Phosphate Analyzers
- One (1) Siemens 5100W 8" magnetic flow meter for filter influent

One (1) Lot **Spare Equipment** consisting of:

One (1) PLC DI module

One (1) PLC DO module

One (1) PLC AI module

One (1) PLC AO module

Two (2) of each type of relay, selector switch, pushbutton, and pilot light used.

#### Automatic Valves:

Under this section we propose to furnish the following 150 lb. Class flanged butterfly valves conforming to AWWA C-504. The valves shall be flanged with EDPM seats, 316 stainless steel shafts and cast iron bodies per ASTM A126. Shaft seals should be self-compensating split V-type or O-ring packing made of BUNA-N per AWWA C-504 class B. The valves shall be supplied with the listed electric operators.

Quantity	Function	Size	Service
Three (3)	Influent	6-inch	open/close
Three (3)	Effluent	10-inch	open/close
Three (3)	BW Inlet	8-inch	open/close
Three (3)	BW Waste	10-inch	open/close
Three (3)	Air Inlet	6-inch	open/close
One (1)	Backwash Control	8-Inch	modulating
One (1)	Air Vent	2-inch	open/close

#### Pumps:

Two (2) **Submersible Backwash Pumps**. The pumps shall be rated for 864 gpm at an estimated 30 feet of head. Accessories shall include a 50' cable, leakage sensor, discharge connection and hardware, guide bar brackets and stainless steel lift chains. The pump motor shall be 25 hp, 60 Hz, 460v, 3 phase and have a cast iron housing, volute and impeller. Also included shall be a manual isolation butterfly valve and an air cushioned swing check valve. **The stainless steel guide bars shall be supplied by the contractor.** 

Two (2) **Submersible Mudwell Pumps**. The pumps shall be rated for 188 gpm at an estimated 30 feet of head. Accessories shall include a 50' cable, leakage sensor, discharge





connection and hardware, guide bar brackets and stainless steel lift chains. The pump motor shall be 6.5 hp, 60 Hz, 460v, 3 phase and have a cast iron housing, volute and impeller. Also included shall be a manual isolation butterfly valve and an air cushioned swing check valve. **The stainless steel guide bars shall be supplied by the contractor.** 

#### **Blowers and Appurtenances:**

Two (2) **Positive Displacement Blower Packages** The blower packages shall be capable of supplying air to the filters during backwash at a rate of 720 scfm. Included with the blower package are TEFC motor, silencer, filter, pressure relief valve, flexible connections, pressure gauges, temperature gauges, discharge check valve and discharge butterfly valve. The blower shall have a 460 volts, 3 phase, 60 hertz, TEFC motor. An acoustical enclosure will be included.

## 3.3 SERVICES

The services of a qualified Leopold technical representative to instruct the Contractor's personnel about the proper installation technique of the mechanical **filter equipment** will be provided for a period of nine (9) days (8 hr/day) on site plus six (6) days travel time to and from the job-site in three (3) trips.

The services of a qualified Leopold technical representative for **filter control system startup and operator training** will be provided for a period of twelve (12) days (8 hr/day) on site plus eight (8) days travel time to and from the job-site in four (4) trips.

Additional services may be obtained at the current prevailing rate plus living and travel expenses.

Should our service representative be scheduled and arrive on site at the time requested by the contractor/purchaser and the equipment is not ready, our standard per diem rate, plus travel and living expenses will apply.

# **4** Technical Clarification & Deviations

#### MEDIA:

#### Submittals:

Materials meet and/or exceed American Water Works Association Standard B100 (latest revision) for Filtering Material. Typical samples and/or test reports detailing the physical and chemical characteristics of the filtering material will be provided for review and approval as required by the specification. If independent testing is required per specification, test reports of the actual material produced will be submitted for approval prior to release for shipment.

### Packaging and Placement of Materials:





Material will be packaged in semi-bulk containers, "Super Bags," with lifting sleeves and bottom discharge spout, containing approximately 2,000 to 4,000 pounds per sack.

#### Quantities:

Quantities indicated above are Xylem Water Solutions USA, Inc best calculations of the quantity requirements. Loss of gravel due to storage or handling is not covered by this proposal.

#### **ITEMS NOT INCLUDED:**

The following items, while not comprehensive, are not included in the elimi-NITE Denitrification System:

- Receiving, unloading, storing, and proper installation of supplied equipment and materials.
- Concrete for filter, building/architectural work and engineering thereof.
- Grout between and under the underdrain laterals in filters.
- Platforms, ladders, or walkways.
- Lubricants for mechanical equipment.
- Interconnecting piping, piping supports, and wall sleeves/pipes including flanges, bolts, nuts, and gaskets.
- Instrument air pipe, isolation valves, tubing, and engineering thereof.
- Electrical starters, circuit breakers, motor control center, conduit, and interconnecting wiring and engineering thereof, and 480 VAC, 3 phase, 60 HZ power.
- Water supply/disposal for flushing of filter internals, media installation or backwash testing.
- Lab services for performance guarantee testing.

# 5 Price & Scope of Supply

## 5.1 MAIN SCOPE

### BASIS of PRICING:

Any items and/or accessories not specifically called out in this quotation must be construed as being furnished by others.





This quotation is considered firm for 90 days. Orders received more than 90 days after the date of this quotation is reviewed by Xylem Water Solutions USA, Inc before acceptance and is subject to changes in prices or delivery depending on conditions existing at the time of entry. Quoted prices are firm for delivery within 12 months from the delivery date stipulated in the plans & specifications or mutually agreed upon by Xylem Water Solutions USA, Inc. and Purchase Order issuer at time of order placement.

We do not include any applicable taxes.

Orders resulting from this quotation should be addresses to Xylem Water Solutions USA, Inc. 108 Tomlinson Dr., Zelienople, PA, 16063, USA.

We propose to furnish the material described in this document for **a total budget selling price** of :

<u>\$\_\_\_\_\_</u>

Pricing for the equipment and field services outlined in this proposal, DAP Jobsite per Incoterms 2020.

For further information pertaining to the equipment contained in this proposal, please contact our area representative, who is:

Sherwood-Logan & Associates, Inc. 2140 Renard Ct. Annapolis, MD 21401 Phone: (410) 274-3716 Email: AKreider@sherwoodlogan.com

Attention: Andrew Kreider

Pricing is based on the following payment terms (net 30 days): 10% following initial submittal for approval 80% following the date of the respective shipments of the product

5% following installation, not to exceed 150 days after shipment of the product (whichever comes first)

5% following start-up, not to exceed 180 days after shipment of the product (whichever comes first)

## 6 Commercial Terms & Conditions

### 6.1 DELIVERY SCHEDULE

#### 6.1.1 Delivery time

Delivery of fabricated items and filter media 24 to 45 weeks after drawing approval. Delivery of filter valves and control 30 to 60 weeks after drawing approval.

### 6.1.2 Production schedule





Submittal of PID's and mechanical drawings for approval 8 to 10 weeks after order acceptance.

Submittal of EIC drawings for approval 8 to 12 weeks after order acceptance.

## 6.2 TERMS AND CONDITIONS OF SALE – NORTH AMERICA

This order is subject to the Standard Terms and Conditions of Sale - Xylem Americas effective on the date the order is accepted. Terms are available at http://www.xyleminc.com/enus/Pages/terms-conditions-of-sale.aspx and incorporated herein by reference and made a part of the agreement between parties.

Different terms are hereby rejected unless expressly assented to in writing.

AGREEMENT TO PURCHASE: BUYER agrees to purchase the equipment and services herein in accordance with the terms and conditions set forth above.	ACCEPTANCE: SELLER hereby accepts BUYER'S offer to purchase.
(BUYER)	Xylem Water Solutions USA, Inc.
BY:	BY:
, 20	, 20





Item 8: UV Disinfection Unit



## ETS - UV SYSTEM

Project Name:	Centerville WWTP
Project Location:	Centerville, MD
Proposal No.:	23 UV 37 PB0
Proposal Date:	25-Apr-2023
Proposal Expires:	24-Jul-2023
Applications Engineer:	Martin Smith
Sales Manager:	Joe Ciurlino
Manufacturers Rep:	Envirep/TLC
Contact:	Dwight Swan
Phone:	(717) 503-4639
Email:	dswan@envirep.com
Consultant:	Whitman, Requardt & Associates, LLP
Consultant:	David R. Nixson, P.E.
Phone:	(443) 824-1620
Email:	dnixson@wrallp.com

## SCOPE OF SUPPLY

Qty	Description
QLY	DESIGN CONSIDERATIONS
	Peak flowrate: 3.3 MGD
	Transmittance (1 cm light cell): 65%
	TSS: <10 mg/l
	Iron concentration: <0.1 mg/l Manganese concentration: <0.1 mg/l
	Influent e. coli: <40,000 MPN/100 ml
	Effluent e. coli: <116 MPN/100 ml
	Configuration: 2 parallel SW-835-14 (100% redundancy)
	Configuration. 2 parallel SW-635-14 (100% reduitdancy)
	SW-835-14
	UV Chamber
2	ETS-UV SW-835-14 UV system complete with:
	14" ANSI flange connections, 316L SS
	(8) 3.5 kW medium pressure UV lamps perpendicular to flow
	(8) Quartz thimbles
	Temperature sensor
	Automatic/Mechanical cleaning
	Access hatch
	(1) UV intensity sensor
	(1) Operation and maintenance manual
	Power/Control Cabinet
2	Free standing power/control cabinet, epoxy coated painted steel, complete with:
	Electronic ballast lamp drive
	Junction box (located nearby reactor, supplied by others)
	Spectra 3, 7" touch screen
	Dimensions: H 52 x W 30 x D 26-in
	Power supply: 480V, 3-Ph, 60Hz
	NEMA12 enclosure
	Cable - UV chamber to power/control cabinet
16	30 ft molded lamp cable
2	30 ft cable kit (sensors / motor)
	Supplied Spares
8	UV lamps
1	Electronic ballast
8	Quartz thimbles
8	Thimble seals
8	Wiper rings
1	Wiper flap for UV intensity monitor



#### **ETS - UV SYSTEM**

### SCOPE OF ENGINEERING

The following documentation shall be provided by Evoqua:

- Shop Drawing Submittal
  - Detailed Scope of Supply
  - Comments & Clarifications
  - Project Schedule
  - Technical Information / Equipment / Drawings
    - Catalog Cutsheets
      - Dimensional Drawings / General Assembly Drawings
      - Functional Schematics / Piping and Instrumentation Diagrams (when applicable) Electrical Schematics (when applicable)
      - Control Panel Layouts, Ladder Logic Diagrams (when applicable)
    - Receiving, Handling and Storage
  - Warranty Statement
- Operation and Maintenance Manuals
  - Ordering Information
  - Warranty Statement
  - Introduction
  - Safety Precautions
  - Preventive Maintenance General Information
  - Maintenance Record Card
  - Regional Offices
  - Technical Data
  - Installation
  - Operation
  - Service
  - Illustrations
  - Preventive Maintenance Kits and Spare Parts List
  - Additional Literature

<u>NOTE</u> - In an effort to be environmentally responsible, one (1) hard copy of the submittal and O+M will be supplied and up to eight (8) copies will be supplied on flash drive(s). Additional hardcopies of the submittal and O+M can be supplied at a cost of \$50.00 each.

### **CLARIFICATIONS & EXCEPTIONS**

Section	Part	Description
NOTICE		The scope of supply and pricing are based on Evoqua's standard equipment selection, standard terms of sale and warranty terms. Any variations from these standards may affect this quotation.

#### **ITEMS NOT INCLUDED IN SCOPE**

- Mechanical and electrical installation labor
- Civil work including supply of anchor bolts
- Interconnecting piping
- Interconnecting wiring (unless detailed above)
- Valves, fittings, appurtenances not specifically listed above
- Installation supervision
- All taxes, fees, lien waivers, certificates, bonds and licenses
- Room ventilation, air conditioning, or lighting
- Videotaping (unless a videotape agreement is signed)



#### **ETS - UV SYSTEM**

### COMMERCIAL OFFERING

Payment Terms:	<ul> <li>30% Due on Approval of Submittals</li> <li>60% Due on Shipment of Equipment</li> <li>10% Due on Startup (not to exceed 90 days after Equipment Shipment)</li> <li>All payments are due 30 days from date of invoice and are not subject to retention</li> </ul>		
EXW:	Factory		
Freight to Job Site:	Included		
Submittal:	4-6 weeks after receipt and approval of purchase order		
Shipment:	16-20 weeks after receipt of full information and approved drawings (when required)		
Startup:	4 On-site day(s) included over 2 Trip(s)		
Training:	Concurrent with startup		
Extended Warranty:	Not Included		
Price:	<u>\$264,352</u>		

#### **Other Conditions:**

- Evoqua Water Technolgies, LLC (Evoqua) proposes to furnish materials, and/or equipment for the project identified at the beginning of this proposal. Any items not shown above as detailed under (i) 'SCOPE OF SUPPLY', (ii) 'SCOPE OF ENGINEERING', or (iii) other attachments to this proposal, are EXCLUDED. In addition:
  - a. Evoqua' price will be held valid for a period of 90 days from the date of this proposal ("Proposal Date"); provided, however, in the event (A) Evoqua receives an order from Buyer within 90 days from the Proposal Date and the percentage change in the U.S. Department of Labor Consumer's Price Index (all items) (the "Index") as it existed two months prior to the Proposal Date and the Index as it existed two months preceding the month in which Evoqua receives Buyer's order is greater than 10%, then Evoqua shall have the right to reprice this proposal or (B) Buyer's order is received more than 90 days beyond the Proposal Date, then Evoqua shall have the right to reprice this proposal.
  - b. Prices are in US Dollars.
  - c. Local or state taxes are not included in this proposal.
- 2) This proposal by Evoqua is contingent upon: (i) Evoqua' written acceptance of the purchase order or other contractual document issued in response to this proposal; and (ii) Evoqua' satisfactory completion of an anti-corruption due diligence review, as applicable; and (iii) the enclosed terms and conditions contained in the following page(s) of this proposal, such terms to take precedence in the event of conflict with any other terms or documents incorporated into the contract arising out of this proposal unless otherwise agreed in writing.
- 3) All of the information supplied by Evoqua in connection with this proposal (including drawings, designs and specifications) (the "Information") is confidential and/or proprietary and has been prepared for your use solely in evaluating the purchase of the equipment and/or services described herein. Transmission of all or any part of the Information to others, or use by you for any purpose other than such evaluation, is expressly prohibited without Evoqua' prior written consent.
- 4) Please address & send your purchase order to:

Neptune Benson Inc. 334 Knight St Ste 3100 Warwick, RI 02886-1286 Attn: Martin Smith ph: 401.262.4731 fax: 401.821.7129 email: martin.smith@evoqua.com



**ETS - UV SYSTEM** 

#### Standard Terms & Conditions of Sale

1. Applicable Terms. These terms govern the purchase and sale of equipment, products, related services, leased products, and media goods if any (collectively herein "Work"), referred to in Seller's proposal ("Seller's Documentation"). Whether these terms are included in an offer or an acceptance by Seller, such offer or acceptance is expressly conditioned on Buyer's assent to these terms. Seller rejects all additional or different terms in any of Buyer's forms or documents.

2. Payment. Buyer shall pay Seller the full purchase price as set forth in Seller's Documentation. Unless Seller's Documentation specifically provides otherwise, freight, storage, insurance and all taxes, levies, duties, tariffs, permits or license fees or other governmental charges relating to the Work or any incremental increases thereto shall be paid by Buyer. If Seller is required to pay any such charges, Buyer shall immediately reimburse Seller. If Buyer claims a tax or other exemption or direct payment permit, it shall provide Seller with a valid exemption certificate or permit and indemnify, defend and hold Seller harmless from any taxes, costs and penalties arising out of same. All payments are due within 30 days after receipt of invoice. Buyer shall be charged the lower of 1 ½% interest per month or the maximum legal rate on all amounts not received by the due date and shall pay all of Seller's reasonable costs (including attorneys' fees) of collecting amounts due but unpaid. All orders are subject to credit approval by Seller. Back charges without Seller's prior written approval shall not be accepted.

3. Delivery. Delivery of the Work shall be in material compliance with the schedule in Seller's Documentation. Unless Seller's Documentation provides otherwise, delivery terms are ExWorks Seller's factory (Incoterms 2010). Title to all Work shall pass upon receipt of payment for the Work under the respective invoice. Unless otherwise agreed to in writing by Seller, shipping dates are approximate only and Seller shall not be liable for any loss or expense (consequential or otherwise) incurred by Buyer or Buyer's customer if Seller fails to meet the specified delivery schedule.

4. Ownership of Materials and Licenses. All devices, designs (including drawings, plans and specifications), estimates, prices, notes, electronic data, software and other documents or information prepared or disclosed by Seller, and all related intellectual property rights, shall remain Seller's property. Seller grants Buyer a non-exclusive, non-transferable license to use any such material solely for Buyer's use of the Work. Buyer shall not disclose any such material to third parties without Seller's prior written consent. Buyer grants Seller a non-exclusive, non-transferable license to use Buyer's name and logo for marketing purposes, including but not limited to, press releases, marketing and promotional materials, and web site content.

5. Changes. Neither party shall implement any changes in the scope of Work described in Seller's Documentation without a mutually agreed upon change order. Any change to the scope of the Work, delivery schedule for the Work, any Force Majeure Event, any law, rule, regulation, order, code, standard or requirement which requires any change hereunder shall entitle Seller to an equitable adjustment in the price and time of performance.

6. Force Majeure Event. Neither Buyer nor Seller shall have any liability for any breach or delay (except for breach of payment obligations) caused by a Force Majeure Event. If a Force Majeure Event exceeds six (6) months in duration, the Seller shall have the right to terminate the Agreement without liability, upon fifteen (15) days written notice to Buyer, and shall be entitled to payment for work performed prior to the date of termination. "Force Majeure Event" shall mean events or circumstances that are beyond the affected party's control and could not reasonably have been easily avoided or overcome by the affected party and are not substantially attributable to the other party. Force Majeure Event may include, but is not limited to, the following circumstances or events: war, act of foreign enemies, terrorism, riot, strike, or lockout by persons other than by Seller or its subsuppliers, natural catastrophes or (with respect to on-site work), unusual weather conditions.

7. Warranty. Subject to the following sentence, Seller warrants to Buyer that the (i) Work shall materially conform to the description in Seller's Documentation and shall be free from defects in material and workmanship and (ii) the Services shall be performed in a timely and workmanlike manner. Determination of suitability of treated water for any use by Buyer shall be the sole and exclusive responsibility of Buyer. The foregoing warranty shall not apply to any Work that is specified or otherwise demanded by Buyer and is not manufactured or selected by Seller, as to which (i) Seller hereby assigns to Buyer, to the extent assignable, any warranties made to Seller and (ii) Seller shall have no other liability to Buyer under warranty, tort or any other legal theory. The Seller warrants the Work, or any components thereof, through the earlier of (i) eighteen (18) months from delivery of the Work or (ii) twelve (12) months from initial operation of the Work or ninety (90) days from the performance of services (the "Warranty Period"). If Buyer gives Seller prompt written notice of breach of this warranty within the Warranty Period, Seller shall, at its sole option and as Buyer's sole and exclusive remedy, repair or replace the subject parts, reperform the Service or refund the purchase price. Unless otherwise agreed to in writing by Seller, (i) Buyer shall be responsible for any labor required to gain access to the Work so that Seller can assess the available remedies and (ii) Buyer shall be responsible for all costs of installation of repaired or replaced Work. If Seller determines that any claimed breach is not, in fact, covered by this warranty, Buyer shall pay Seller its then customary charges for any repair or replacement made by Seller. Seller's warranty is conditioned on Buyer's (a) operating and maintaining the Work in accordance with Seller's instructions, (b) not making any unauthorized repairs or alterations, and (c) not being in default of any payment obligation to Seller. Seller's warranty does not cover (i) damage caused by chemical action or abrasive material, misuse or improper installation (unless installed by Seller) and (iii) media goods (such as, but not limited to, resin, membranes, or granular activated carbon media) once media goods are installed. THE WARRANTIES SET FORTH IN THIS SECTION 7 ARE THE SELLER'S SOLE AND EXCLUSIVE WARRANTIES AND ARE SUBJECT TO THE LIMITATION OF LIABILITY PROVISION BELOW. SELLER MAKES NO OTHER WARRANTIES OF ANY KIND, EXPRESS OR IMPLIED, INCLUDING WITHOUT LIMITATION, ANY WARRANTY OF MERCHANTABILITY OR FITNESS FOR PURPOSE.

8. Indemnity. Seller shall indemnify, defend and hold Buyer harmless from any claim, cause of action or liability incurred by Buyer as a result of third party claims for personal injury, death or damage to tangible property, to the extent caused by Seller's negligence. Seller shall have the sole authority to direct the defense of and settle any indemnified claim. Seller's indemnification is conditioned on Buyer (a) promptly, within the Warranty Period, notifying Seller of any claim, and (b) providing reasonable cooperation in the defense of any claim.

9. Assignment. Neither party may assign this Agreement, in whole or in part, nor any rights or obligations hereunder without the prior written consent of the other party; provided, however, the Seller may assign its rights and obligations under these terms to its affiliates or in connection with the sale or transfer of the Seller's business and Seller may grant a security interest in the Agreement and/or assign proceeds of the agreement without Buyer's consent.

**10. Termination.** Either party may terminate this agreement, upon issuance of a written notice of breach and a thirty (30) day cure period, for a material breach (including but not limited to, filing of bankruptcy, or failure to fulfill the material obligations of this agreement). If Buyer suspends an order without a change order for ninety (90) or more days, Seller may thereafter terminate this Agreement without liability, upon fifteen (15) days written notice to Buyer, and shall be entitled to payment for work performed, whether delivered or undelivered, prior to the date of termination.

1-May-15



ETS - UV SYSTEM

11. Dispute Resolution. Seller and Buyer shall negotiate in good faith to resolve any dispute relating hereto. If, despite good faith efforts, the parties are unable to resolve a dispute or claim arising out of or relating to this Agreement or its breach, termination, enforcement, interpretation or validity, the parties will first seek to agree on a forum for mediation to be held in a mutually agreeable site. If the parties are unable to resolve the dispute through mediation, then any dispute, claim or controversy arising out of or relating to this Agreement or the breach, termination, enforcement, interpretation or validity thereof, including the determination of the scope or applicability of this agreement to arbitrate, shall be determined by arbitration in Pittsburgh, Pennsylvania before three arbitrators who are lawyers experienced in the discipline that is the subject of the dispute and shall be jointly selected by Seller and Buyer. The arbitration shall be administered by JAMS pursuant to its Comprehensive Arbitration Rules and Procedures. The Arbitrators shall issue a reasoned decision of a majority of the arbitrators, which shall be the decision of the panel. Judgment may be entered upon the arbitrators' decision in any court of competent jurisdiction. The substantially prevailing party as determined by the arbitrators shall be reimbursed by the other party for all costs, expenses and charges, including without limitation reasonable attorneys' fees, incurred by the prevailing party in connection with the arbitration. For any order shipped outside of the United States, any dispute shall be referred to and finally determined by the International Center for Dispute Resolution in accordance with the provisions of its International Arbitration Rules, enforceable under the New York Convention (Convention on the Recognition and Enforcement of Foreign Arbitral Awards) and the governing language shall be English.

12. Export Compliance. Buyer acknowledges that Seller is required to comply with applicable export laws and regulations relating to the sale, exportation, transfer, assignment, disposal and usage of the Work provided under this Agreement, including any export license requirements. Buyer agrees that such Work shall not at any time directly or indirectly be used, exported, sold, transferred, assigned or otherwise disposed of in a manner which will result in non-compliance with such applicable export laws and regulations. It shall be a condition of the continuing performance by Seller of its obligations hereunder that compliance with such export laws and regulations be maintained at all times. BUYER AGREES TO INDEMNIFY AND HOLD SELLER HARMLESS FROM ANY AND ALL COSTS, LIABILITIES, PENALTIES, SANCTIONS AND FINES RELATED TO NON-COMPLIANCE WITH APPLICABLE EXPORT LAWS AND REGULATIONS.

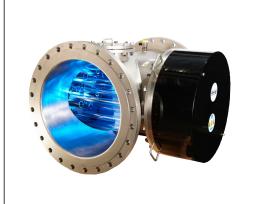
13. LIMITATION OF LIABILITY. NOTWITHSTANDING ANYTHING ELSE TO THE CONTRARY, SELLER SHALL NOT BE LIABLE FOR ANY CONSEQUENTIAL, INCIDENTAL, SPECIAL, PUNITIVE OR OTHER INDIRECT DAMAGES, AND SELLER'S TOTAL LIABILITY ARISING AT ANY TIME FROM THE SALE OR USE OF THE WORK, INCLUDING WITHOUT LIMITATION ANY LIABILITY FOR MECHANICAL WARRANTY CLAIMS OR FOR ANY BREACH OR FAILURE TO PERFORM ANY OBLIGATION UNDER THE CONTRACT, SHALL NOT EXCEED THE PURCHASE PRICE PAID FOR THE WORK. THESE LIMITATIONS APPLY WHETHER THE LIABILITY IS BASED ON CONTRACT, TORT, STRICT LIABILITY OR ANY OTHER THEORY.

14. Rental Equipment / Services. Any leased or rented equipment ("Leased Equipment") provided by Seller shall at all times be the property of Seller with the exception of certain miscellaneous installation materials purchased by the Buyer, and no right or property interest is transferred to the Buyer, except the right to use any such Leased Equipment as provided herein. Buyer agrees that it shall not pledge, lend, or create a security interest in, part with possession of, or relocate the Leased Equipment. Buyer shall be responsible to maintain the Leased Equipment in good and efficient working order. At the end of the initial term specified in the order, the terms shall automatically renew for the identical period unless canceled in writing by Buyer or Seller not sooner than three (3) months nor later than one (1) month from termination of the initial order or any renewal terms. Upon any renewal, Seller shall have the right to issue notice of increased pricing which shall be effective for any renewed terms unless Buyer objects in writing within fifteen (15) days of issuance of said notice. If Buyer timely cancels service in writing prior to the end of the initial or any renewal term this shall not relieve Buyer of its obligations under the order for the monthly rental service charge which shall continue to be due and owing. Upon the expiration or termination of this Agreement, Buyer shall promptly make any Leased Equipment available to Seller for removal. Buyer hereby agrees that it shall grant Seller access to the Leased Equipment location and shall permit Seller to take possession of and remove the Leased Equipment without resort to legal process and hereby releases Seller from any claim or right of action for trespass or damages caused by reason of such entry and removal.

15. Miscellaneous. These terms, together with any Contract Documents issued or signed by the Seller, comprise the complete and exclusive statement of the agreement between the parties (the "Agreement") and supersede any terms contained in Buyer's documents, unless separately signed by Seller. No part of the Agreement may be changed or cancelled except by a written document signed by Seller and Buyer. No course of dealing or performance, usage of trade or failure to enforce any term shall be used to modify the Agreement. To the extent the Agreement is considered a subcontract under Buyer's prime contract with an agency of the United States government, in case of Federal Acquisition Regulations (FARs) flow down terms, Seller will be in compliance with Section 44.403 of the FAR relating to commercial items and those additional clauses as specifically listed in 52.244-6, Subcontracts for Commercial Items (OCT 2014). If any of these terms is unenforceable, such term shall be limited only to the extent necessary to make it enforceable, and all other terms shall remain in full force and effect. The Agreement shall be governed by the laws of the Commonwealth of Pennsylvania without regard to its conflict of laws provisions. Both Buyer and Seller reject the applicability of the United Nations Convention on Contracts for the international sales of goods to the relationship between the parties and to all transactions arising from said relationship.

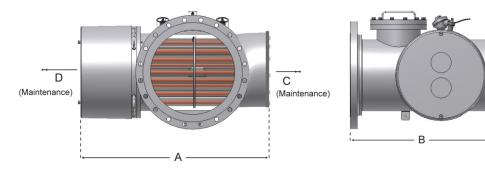


# SPECIFICATION SHEET SW-835-14 Electronic Ballasts





Model	Connection	# of	Lamp Power	Dim	Dimensions (inc		
		Lamps	Per Lamp	А	В	С	C D
SW-835-14	14 inches	8	3.5 kW	37	30	4	14



Drawings for illustration purposes only, use specific GA drawings for accuracy



SPECTRA Touch Control Panel		
Height	52 in	
Width	30 in	
Depth	26 in	
Voltage	480 V	
Frequency	60 Hz	
Phase	3	

# Multi-lamp, medium pressure UV systems for wastewaterapplications

#### **CHAMBER**

316L SS ANSI 150# flanged connections Install inline horizontally or vertically *Features:* Twist lock lamp connections Variable power lamps Dry UV intensity monitors High purity quartz sleeves Low voltage automatic wiper Access hatch One piece wiper ring Temperature sensor Drain and vent ports

#### **CONTROL SYSTEM**

NEMA 12 enclosure Epoxy coated mild steel enclosure Operational 32-113°F, RH <90% *Features:* 7" HMI touch screen Microprocessor system for basic control to full plant system integration MODBUS over Ethernet communication Internet monitoring capability Data logging capability with remote access Multiple warning and alarms Additional Information: Spectra Touch spec sheet

#### **SYSTEM OPTIONS**

304 or 316 NEMA 4X enclosures Certified explosion-proof design Skid mounted Containerized Internal/external polish or electropolish

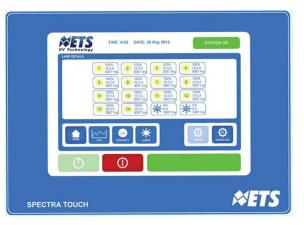
#### **INSTALLATION NOTES**

Provide necessary maintenance space Intstall in a dry area Provide floor dain or sump Lamps submerged at all times Minimum of two conduits required Chamber must be grounded



# SPECIFICATION SHEET SPECTRA TOUCH

ETS UV Technology microprocessor control system offers multiple levels of operation from basic controls to full plant system integration. Available on all UV systems. Existing systems can be upgraded to include a TOUCH control panel.





### SIMPLE CONTROLS AND DISPLAY

- 7" resistive touch screen human machine interface (HMI)
- Glare free operation
- On screen trending
- STOP soft touch push buttons
- RESET soft touch push buttons
- Simple operation for any level of technical experience and expertise
- All alarm functions have a simple text message display

### **INTERFACE CONTROLS**

- Ethernet connectivity/WiFi capability
- Selectable custom input and outputs
- Local and remote operation
- Process interrupt (valves, flow meters or pressure switches)
- Low UV alarm and shutdown
- Bleed temperature
- Flow meter input
- Automatic restart
- Variable power dosing
- Duty/Standby automatic changeover

### **ADVANCED DISPLAY FEATURES**

- Improved noise resistance
- Distributed I/O possible
- On/Off control
- Lamp running indication/lamp current
- Power on indication
- Elapsed hours meter
- Lamp failed contact (volt free)
- UV intensity & UV dose mJ/cm2
- Flow rate (accepts a 4-20ma signal from a flow meter)
- Temperature, low UV alarm
- System spares listing
- Ground fault
- Wiper fault

#### **ADVANCED DISPLAY FEATURES**

The Touch has a built in data logging facility (retrievable by users on a standard PC or laptop). The parameters logged are:

- UV intensity required (set point)
- UV intensity measured
- Lamp current
- Temperature
- Flow (if flow meter connected)
- Time and date
- Alarms generated: restrike timer, low intensity, low dose, high temperature, PSU temperature, lamp fault and ground fault



## ETS - UV SYSTEM

Project Name:	Centerville WWTP
Project Location:	Centerville, MD
Proposal No.:	23 UV 37 PB1
Proposal Date:	25-Apr-2023
Proposal Expires:	24-Jul-2023
Applications Engineer:	Martin Smith
Sales Manager:	Joe Ciurlino
Manufacturers Rep:	Envirep/TLC
Contact:	Dwight Swan
Phone:	(717) 503-4639
Email:	dswan@envirep.com
Consultant:	Whitman, Requardt & Associates, LLP
Consultant:	David R. Nixson, P.E.
Phone:	(443) 824-1620
Email:	dnixson@wrallp.com

## SCOPE OF SUPPLY

Qty	Description
- acty	DESIGN CONSIDERATIONS
	Peak flowrate: 3.3 MGD
	Transmittance (1 cm light cell): 65%
	TSS: <10 mg/l
	Iron concentration: <0.1 mg/l
	Manganese concentration: <0.1 mg/l
	Influent e. coli: <40,000 MPN/100 ml
	Effluent e. coli: <116 MPN/100 ml
	Configuration: 2 parallel UVLW-20800-20 (100% redundancy)
	Comgutation. 2 parallel 5 v 2000-20 (100 % redundancy)
	UVLW-20800-20
	UV Chamber
2	ETS-UV UVLW-20800-20 UV system complete with:
	16" ANSI flange connections, 316L SS
	(20) 800 W low pressure high output UV lamps parallel to flow
	(20) Quartz thimbles
	Temperature sensor
-	Automatic/Mechanical cleaning
	Access hatch
	(1) UV intensity sensor
	(1) Operation and maintenance manual
	Power/Control Cabinet
2	Free standing power/control cabinet, epoxy coated painted steel, complete with:
	Electronic ballast lamp drive
	Spectra 3, 7" touch screen
	Dimensions: H 79 x W 62 x D 24-in
	Power supply: 480V, 3ø, 4-Wire + GND (Wye), 60Hz
	NEMA12 enclosure
	Cables - UV chamber to power/control cabinet
2	30 ft cable set
	UVT Monitor
1	Online UVT Transmittance Monitor + Real Clean System
	-
	Supplied Spares
4	UV lamps
2	Electronic ballasts
3	Quartz thimbles
3	Thimble seals
20	Wiper rings
1	Wiper flap for UV intensity monitor



#### ETS - UV SYSTEM

#### SCOPE OF ENGINEERING

.

The following documentation shall be provided by Evoqua:

- Shop Drawing Submittal
  - Detailed Scope of Supply
  - Comments & Clarifications
  - Project Schedule
  - Technical Information / Equipment / Drawings
     Catalog Cutsheets
     Dimensional Drawings / General Assembly Drawings
     Functional Schematics / Piping and Instrumentation Diagrams (when applicable)
     Electrical Schematics (when applicable)
    - Control Panel Layouts, Ladder Logic Diagrams (when applicable)
    - Receiving, Handling and Storage
    - Warranty Statement

- Operation and Maintenance Manuals

- Ordering Information
- Warranty Statement
- Introduction
- Safety Precautions
- Preventive Maintenance General Information
- Maintenance Record Card
- Regional Offices
- Technical Data
- Installation
- Operation
- Service
- Illustrations
- Preventive Maintenance Kits and Spare Parts List
- Additional Literature

<u>NOTE</u> - In an effort to be environmentally responsible, one (1) hard copy of the submittal and O+M will be supplied and up to eight (8) copies will be supplied on flash drive(s). Additional hardcopies of the submittal and O+M can be supplied at a cost of \$50.00 each.

### **CLARIFICATIONS & EXCEPTIONS**

Section	Part	Description
NOTICE		The scope of supply and pricing are based on Evoqua's standard equipment selection, standard terms of sale and warranty terms. Any variations from these standards may affect this quotation.

#### **ITEMS NOT INCLUDED IN SCOPE**

- Mechanical and electrical installation labor
- Civil work including supply of anchor bolts
- Interconnecting piping
- Interconnecting wiring (unless detailed above)
- Valves, fittings, appurtenances not specifically listed above
- Installation supervision
- All taxes, fees, lien waivers, certificates, bonds and licenses
- Room ventilation, air conditioning, or lighting
- Videotaping (unless a videotape agreement is signed)



#### **ETS - UV SYSTEM**

### COMMERCIAL OFFERING

Payment Terms:	<ul> <li>30% Due on Approval of Submittals</li> <li>60% Due on Shipment of Equipment</li> <li>10% Due on Startup (not to exceed 90 days after Equipment Shipment)</li> <li>All payments are due 30 days from date of invoice and are not subject to retention</li> </ul>		
EXW:	Factory		
Freight to Job Site:	Included		
Submittal:	4-6 weeks after receipt and approval of purchase order		
Shipment:	16-20 weeks after receipt of full information and approved drawings (when required)		
Startup:	5 On-site day(s) included over 2 Trip(s)		
Training:	Concurrent with startup		
Extended Warranty:	Not Included		
Price:	<u>\$357,448</u>		

#### **Other Conditions:**

- Evoqua Water Technolgies, LLC (Evoqua) proposes to furnish materials, and/or equipment for the project identified at the beginning of this proposal. Any items not shown above as detailed under (i) 'SCOPE OF SUPPLY', (ii) 'SCOPE OF ENGINEERING', or (iii) other attachments to this proposal, are EXCLUDED. In addition:
  - a. Evoqua' price will be held valid for a period of 90 days from the date of this proposal ("Proposal Date"); provided, however, in the event (A) Evoqua receives an order from Buyer within 90 days from the Proposal Date and the percentage change in the U.S. Department of Labor Consumer's Price Index (all items) (the "Index") as it existed two months prior to the Proposal Date and the Index as it existed two months preceding the month in which Evoqua receives Buyer's order is greater than 10%, then Evoqua shall have the right to reprice this proposal or (B) Buyer's order is received more than 90 days beyond the Proposal Date, then Evoqua shall have the right to reprice this proposal.
  - b. Prices are in US Dollars.
  - c. Local or state taxes are not included in this proposal.
- 2) This proposal by Evoqua is contingent upon: (i) Evoqua' written acceptance of the purchase order or other contractual document issued in response to this proposal; and (ii) Evoqua' satisfactory completion of an anti-corruption due diligence review, as applicable; and (iii) the enclosed terms and conditions contained in the following page(s) of this proposal, such terms to take precedence in the event of conflict with any other terms or documents incorporated into the contract arising out of this proposal unless otherwise agreed in writing.
- 3) All of the information supplied by Evoqua in connection with this proposal (including drawings, designs and specifications) (the "Information") is confidential and/or proprietary and has been prepared for your use solely in evaluating the purchase of the equipment and/or services described herein. Transmission of all or any part of the Information to others, or use by you for any purpose other than such evaluation, is expressly prohibited without Evoqua' prior written consent.
- 4) Please address & send your purchase order to:

Neptune Benson Inc. 334 Knight St Ste 3100 Warwick, RI 02886-1286 Attn: Martin Smith ph: 401.262.4731 fax: 401.821.7129 email: martin.smith@evoqua.com



**ETS - UV SYSTEM** 

#### Standard Terms & Conditions of Sale

1. Applicable Terms. These terms govern the purchase and sale of equipment, products, related services, leased products, and media goods if any (collectively herein "Work"), referred to in Seller's proposal ("Seller's Documentation"). Whether these terms are included in an offer or an acceptance by Seller, such offer or acceptance is expressly conditioned on Buyer's assent to these terms. Seller rejects all additional or different terms in any of Buyer's forms or documents.

2. Payment. Buyer shall pay Seller the full purchase price as set forth in Seller's Documentation. Unless Seller's Documentation specifically provides otherwise, freight, storage, insurance and all taxes, levies, duties, tariffs, permits or license fees or other governmental charges relating to the Work or any incremental increases thereto shall be paid by Buyer. If Seller is required to pay any such charges, Buyer shall immediately reimburse Seller. If Buyer claims a tax or other exemption or direct payment permit, it shall provide Seller with a valid exemption certificate or permit and indemnify, defend and hold Seller harmless from any taxes, costs and penalties arising out of same. All payments are due within 30 days after receipt of invoice. Buyer shall be charged the lower of 1 ½% interest per month or the maximum legal rate on all amounts not received by the due date and shall pay all of Seller's reasonable costs (including attorneys' fees) of collecting amounts due but unpaid. All orders are subject to credit approval by Seller. Back charges without Seller's prior written approval shall not be accepted.

3. Delivery. Delivery of the Work shall be in material compliance with the schedule in Seller's Documentation. Unless Seller's Documentation provides otherwise, delivery terms are ExWorks Seller's factory (Incoterms 2010). Title to all Work shall pass upon receipt of payment for the Work under the respective invoice. Unless otherwise agreed to in writing by Seller, shipping dates are approximate only and Seller shall not be liable for any loss or expense (consequential or otherwise) incurred by Buyer or Buyer's customer if Seller fails to meet the specified delivery schedule.

4. Ownership of Materials and Licenses. All devices, designs (including drawings, plans and specifications), estimates, prices, notes, electronic data, software and other documents or information prepared or disclosed by Seller, and all related intellectual property rights, shall remain Seller's property. Seller grants Buyer a non-exclusive, non-transferable license to use any such material solely for Buyer's use of the Work. Buyer shall not disclose any such material to third parties without Seller's prior written consent. Buyer grants Seller a non-exclusive, non-transferable license to use Buyer's name and logo for marketing purposes, including but not limited to, press releases, marketing and promotional materials, and web site content.

5. Changes. Neither party shall implement any changes in the scope of Work described in Seller's Documentation without a mutually agreed upon change order. Any change to the scope of the Work, delivery schedule for the Work, any Force Majeure Event, any law, rule, regulation, order, code, standard or requirement which requires any change hereunder shall entitle Seller to an equitable adjustment in the price and time of performance.

6. Force Majeure Event. Neither Buyer nor Seller shall have any liability for any breach or delay (except for breach of payment obligations) caused by a Force Majeure Event. If a Force Majeure Event exceeds six (6) months in duration, the Seller shall have the right to terminate the Agreement without liability, upon fifteen (15) days written notice to Buyer, and shall be entitled to payment for work performed prior to the date of termination. "Force Majeure Event" shall mean events or circumstances that are beyond the affected party's control and could not reasonably have been easily avoided or overcome by the affected party and are not substantially attributable to the other party. Force Majeure Event may include, but is not limited to, the following circumstances or events: war, act of foreign enemies, terrorism, riot, strike, or lockout by persons other than by Seller or its subsuppliers, natural catastrophes or (with respect to on-site work), unusual weather conditions.

7. Warranty. Subject to the following sentence, Seller warrants to Buyer that the (i) Work shall materially conform to the description in Seller's Documentation and shall be free from defects in material and workmanship and (ii) the Services shall be performed in a timely and workmanlike manner. Determination of suitability of treated water for any use by Buyer shall be the sole and exclusive responsibility of Buyer. The foregoing warranty shall not apply to any Work that is specified or otherwise demanded by Buyer and is not manufactured or selected by Seller, as to which (i) Seller hereby assigns to Buyer, to the extent assignable, any warranties made to Seller and (ii) Seller shall have no other liability to Buyer under warranty, tort or any other legal theory. The Seller warrants the Work, or any components thereof, through the earlier of (i) eighteen (18) months from delivery of the Work or (ii) twelve (12) months from initial operation of the Work or ninety (90) days from the performance of services (the "Warranty Period"). If Buyer gives Seller prompt written notice of breach of this warranty within the Warranty Period, Seller shall, at its sole option and as Buyer's sole and exclusive remedy, repair or replace the subject parts, reperform the Service or refund the purchase price. Unless otherwise agreed to in writing by Seller, (i) Buyer shall be responsible for any labor required to gain access to the Work so that Seller can assess the available remedies and (ii) Buyer shall be responsible for all costs of installation of repaired or replaced Work. If Seller determines that any claimed breach is not, in fact, covered by this warranty, Buyer shall pay Seller its then customary charges for any repair or replacement made by Seller. Seller's warranty is conditioned on Buyer's (a) operating and maintaining the Work in accordance with Seller's instructions, (b) not making any unauthorized repairs or alterations, and (c) not being in default of any payment obligation to Seller. Seller's warranty does not cover (i) damage caused by chemical action or abrasive material, misuse or improper installation (unless installed by Seller) and (ii) media goods (such as, but not limited to, resin, membranes, or granular activated carbon media) once media goods are installed. THE WARRANTIES SET FORTH IN THIS SECTION 7 ARE THE SELLER'S SOLE AND EXCLUSIVE WARRANTIES AND ARE SUBJECT TO THE LIMITATION OF LIABILITY PROVISION BELOW. SELLER MAKES NO OTHER WARRANTIES OF ANY KIND, EXPRESS OR IMPLIED, INCLUDING WITHOUT LIMITATION, ANY WARRANTY OF MERCHANTABILITY OR FITNESS FOR PURPOSE.

8. Indemnity. Seller shall indemnify, defend and hold Buyer harmless from any claim, cause of action or liability incurred by Buyer as a result of third party claims for personal injury, death or damage to tangible property, to the extent caused by Seller's negligence. Seller shall have the sole authority to direct the defense of and settle any indemnified claim. Seller's indemnification is conditioned on Buyer (a) promptly, within the Warranty Period, notifying Seller of any claim, and (b) providing reasonable cooperation in the defense of any claim.

9. Assignment. Neither party may assign this Agreement, in whole or in part, nor any rights or obligations hereunder without the prior written consent of the other party; provided, however, the Seller may assign its rights and obligations under these terms to its affiliates or in connection with the sale or transfer of the Seller's business and Seller may grant a security interest in the Agreement and/or assign proceeds of the agreement without Buyer's consent.

**10. Termination.** Either party may terminate this agreement, upon issuance of a written notice of breach and a thirty (30) day cure period, for a material breach (including but not limited to, filing of bankruptcy, or failure to fulfill the material obligations of this agreement). If Buyer suspends an order without a change order for ninety (90) or more days, Seller may thereafter terminate this Agreement without liability, upon fifteen (15) days written notice to Buyer, and shall be entitled to payment for work performed, whether delivered or undelivered, prior to the date of termination.

1-May-15



ETS - UV SYSTEM

11. Dispute Resolution. Seller and Buyer shall negotiate in good faith to resolve any dispute relating hereto. If, despite good faith efforts, the parties are unable to resolve a dispute or claim arising out of or relating to this Agreement or its breach, termination, enforcement, interpretation or validity, the parties will first seek to agree on a forum for mediation to be held in a mutually agreeable site. If the parties are unable to resolve the dispute through mediation, then any dispute, claim or controversy arising out of or relating to this Agreement or the breach, termination, enforcement, interpretation or validity thereof, including the determination of the scope or applicability of this agreement to arbitrate, shall be determined by arbitration in Pittsburgh, Pennsylvania before three arbitrators who are lawyers experienced in the discipline that is the subject of the dispute and shall be jointly selected by Seller and Buyer. The arbitration shall be administered by JAMS pursuant to its Comprehensive Arbitration Rules and Procedures. The Arbitrators shall issue a reasoned decision of a majority of the arbitrators, which shall be the decision of the panel. Judgment may be entered upon the arbitrators' decision in any court of competent jurisdiction. The substantially prevailing party as determined by the arbitrators shall be reimbursed by the other party for all costs, expenses and charges, including without limitation reasonable attorneys' fees, incurred by the prevailing party in connection with the arbitration. For any order shipped outside of the United States, any dispute shall be referred to and finally determined by the International Center for Dispute Resolution in accordance with the provisions of its International Arbitration Rules, enforceable under the New York Convention (Convention on the Recognition and Enforcement of Foreign Arbitral Awards) and the governing language shall be English.

12. Export Compliance. Buyer acknowledges that Seller is required to comply with applicable export laws and regulations relating to the sale, exportation, transfer, assignment, disposal and usage of the Work provided under this Agreement, including any export license requirements. Buyer agrees that such Work shall not at any time directly or indirectly be used, exported, sold, transferred, assigned or otherwise disposed of in a manner which will result in non-compliance with such applicable export laws and regulations. It shall be a condition of the continuing performance by Seller of its obligations hereunder that compliance with such export laws and regulations be maintained at all times. BUYER AGREES TO INDEMNIFY AND HOLD SELLER HARMLESS FROM ANY AND ALL COSTS, LIABILITIES, PENALTIES, SANCTIONS AND FINES RELATED TO NON-COMPLIANCE WITH APPLICABLE EXPORT LAWS AND REGULATIONS.

13. LIMITATION OF LIABILITY. NOTWITHSTANDING ANYTHING ELSE TO THE CONTRARY, SELLER SHALL NOT BE LIABLE FOR ANY CONSEQUENTIAL, INCIDENTAL, SPECIAL, PUNITIVE OR OTHER INDIRECT DAMAGES, AND SELLER'S TOTAL LIABILITY ARISING AT ANY TIME FROM THE SALE OR USE OF THE WORK, INCLUDING WITHOUT LIMITATION ANY LIABILITY FOR MECHANICAL WARRANTY CLAIMS OR FOR ANY BREACH OR FAILURE TO PERFORM ANY OBLIGATION UNDER THE CONTRACT, SHALL NOT EXCEED THE PURCHASE PRICE PAID FOR THE WORK. THESE LIMITATIONS APPLY WHETHER THE LIABILITY IS BASED ON CONTRACT, TORT, STRICT LIABILITY OR ANY OTHER THEORY.

14. Rental Equipment / Services. Any leased or rented equipment ("Leased Equipment") provided by Seller shall at all times be the property of Seller with the exception of certain miscellaneous installation materials purchased by the Buyer, and no right or property interest is transferred to the Buyer, except the right to use any such Leased Equipment as provided herein. Buyer agrees that it shall not pledge, lend, or create a security interest in, part with possession of, or relocate the Leased Equipment. Buyer shall be responsible to maintain the Leased Equipment in good and efficient working order. At the end of the initial term specified in the order, the terms shall automatically renew for the identical period unless canceled in writing by Buyer or Seller not sooner than three (3) months nor later than one (1) month from termination of the initial order or any renewal terms. Upon any renewal, Seller shall have the right to issue notice of increased pricing which shall be effective for any renewed terms unless Buyer objects in writing within fifteen (15) days of issuance of said notice. If Buyer timely cancels service in writing prior to the end of the initial or any renewal term this shall not relieve Buyer of its obligations under the order for the monthly rental service charge which shall continue to be due and owing. Upon the expiration or termination of this Agreement, Buyer shall promptly make any Leased Equipment available to Seller for removal. Buyer hereby agrees that it shall grant Seller access to the Leased Equipment location and shall permit Seller to take possession of and remove the Leased Equipment without resort to legal process and hereby releases Seller from any claim or right of action for trespass or damages caused by reason of such entry and removal.

15. Miscellaneous. These terms, together with any Contract Documents issued or signed by the Seller, comprise the complete and exclusive statement of the agreement between the parties (the "Agreement") and supersede any terms contained in Buyer's documents, unless separately signed by Seller. No part of the Agreement may be changed or cancelled except by a written document signed by Seller and Buyer. No course of dealing or performance, usage of trade or failure to enforce any term shall be used to modify the Agreement. To the extent the Agreement is considered a subcontract under Buyer's prime contract with an agency of the United States government, in case of Federal Acquisition Regulations (FARs) flow down terms, Seller will be in compliance with Section 44.403 of the FAR relating to commercial items and those additional clauses as specifically listed in 52.244-6, Subcontracts for Commercial Items (OCT 2014). If any of these terms is unenforceable, such term shall be limited only to the extent necessary to make it enforceable, and all other terms shall remain in full force and effect. The Agreement shall be governed by the laws of the Commonwealth of Pennsylvania without regard to its conflict of laws provisions. Both Buyer and Seller reject the applicability of the United Nations Convention on Contracts for the international sales of goods to the relationship between the parties and to all transactions arising from said relationship.

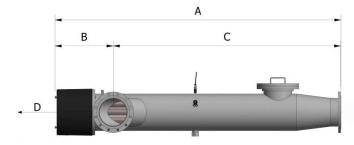


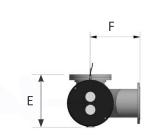
# SPECIFICATION SHEET UVLW RANGE





# of Dimensions Dimensions Lamps Model (800W) W ULVW-6800-10 UVLW-6800-14 UVLW-8800-14 UVLW-16800-20 UVLW-20800-20 UVLW-22800-24 UVLW-30800-24 UVLW-30800-30 UVLW-45800-30 





The UVLW is a range of 800W low pressure, high output amalgam UV systems that are validated to the 2003 and 2012 NWRI Reuse Guidelines

#### **CHAMBER**

316L SS ANSI 150# flanged connections Install inline, horizontally or vertically Features: Acess Hatch Twist lock lamp connections Dry UV intensity monitor High purity quartz thimbles Low voltage automatic wiper One piece wiper ring Temperature sensor Drain and vent ports

#### **CONTROL SYSTEM**

NEMA 12 epoxy coated mild steel enclosure Operational 32-113°F, RH <90% Features: 7" HMI Spectra II control system MODBUS Multiple warnings and alarms Variable power lamps 480V/3-phase

#### **SYSTEM OPTIONS**

304 or 316 NEMA 4X enclosures Effluent flange location Skid mounted Containerized Internal/external polish or electropolish

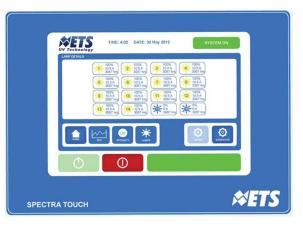
#### **INSTALLATION NOTES**

Provide necessary maintenance space Install in a dry area Provide floor drain or sump Lamps submerged at all times Minimum of two conduits required Chamber must be grounded



# SPECIFICATION SHEET SPECTRA TOUCH

ETS UV Technology microprocessor control system offers multiple levels of operation from basic controls to full plant system integration. Available on all UV systems. Existing systems can be upgraded to include a TOUCH control panel.





### SIMPLE CONTROLS AND DISPLAY

- 7" resistive touch screen human machine interface (HMI)
- Glare free operation
- On screen trending
- STOP soft touch push buttons
- RESET soft touch push buttons
- Simple operation for any level of technical experience and expertise
- All alarm functions have a simple text message display

### **INTERFACE CONTROLS**

- Ethernet connectivity/WiFi capability
- Selectable custom input and outputs
- Local and remote operation
- Process interrupt (valves, flow meters or pressure switches)
- Low UV alarm and shutdown
- Bleed temperature
- Flow meter input
- Automatic restart
- Variable power dosing
- Duty/Standby automatic changeover

### **ADVANCED DISPLAY FEATURES**

- Improved noise resistance
- Distributed I/O possible
- On/Off control
- Lamp running indication/lamp current
- Power on indication
- Elapsed hours meter
- Lamp failed contact (volt free)
- UV intensity & UV dose mJ/cm2
- Flow rate (accepts a 4-20ma signal from a flow meter)
- Temperature, low UV alarm
- System spares listing
- Ground fault
- Wiper fault

### **ADVANCED DISPLAY FEATURES**

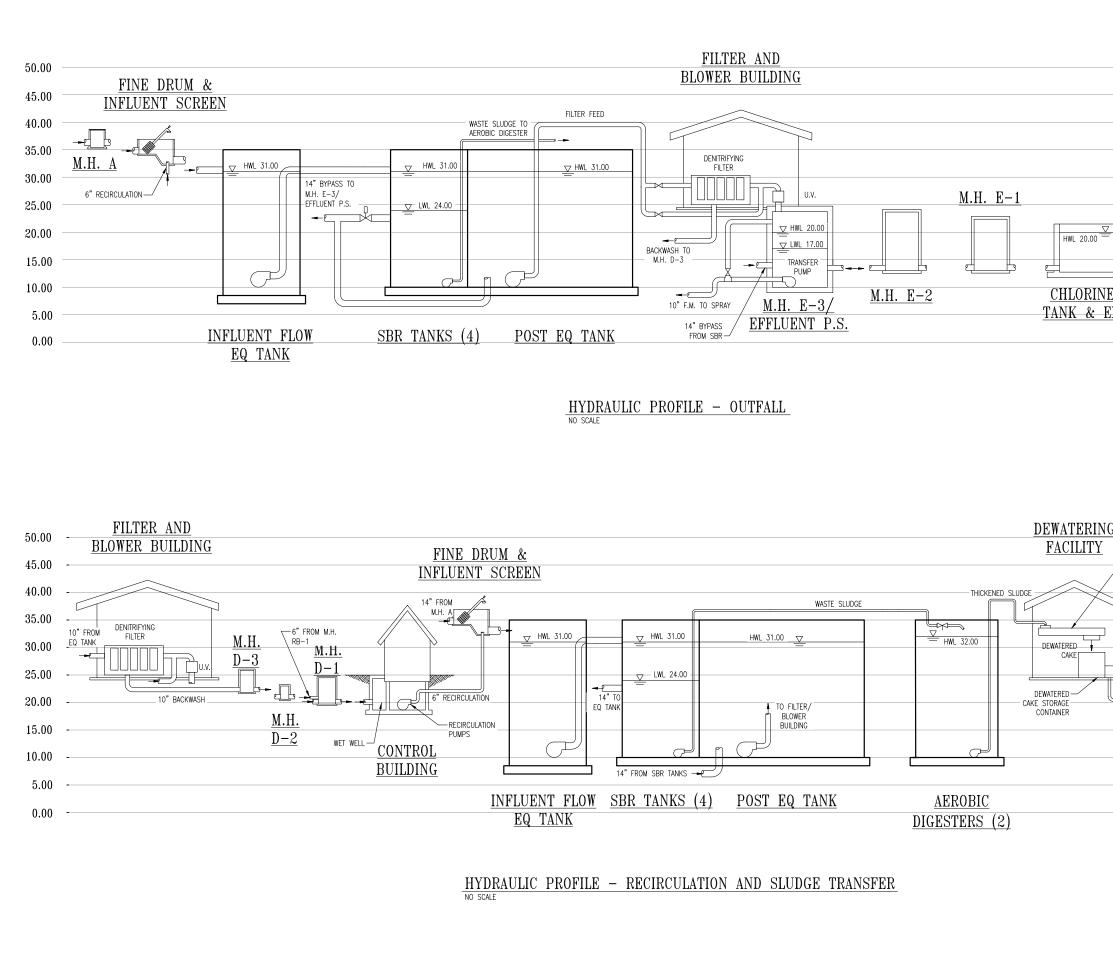
The Touch has a built in data logging facility (retrievable by users on a standard PC or laptop). The parameters logged are:

- UV intensity required (set point)
- UV intensity measured
- Lamp current
- Temperature
- Flow (if flow meter connected)
- Time and date
- Alarms generated: restrike timer, low intensity, low dose, high temperature, PSU temperature, lamp fault and ground fault

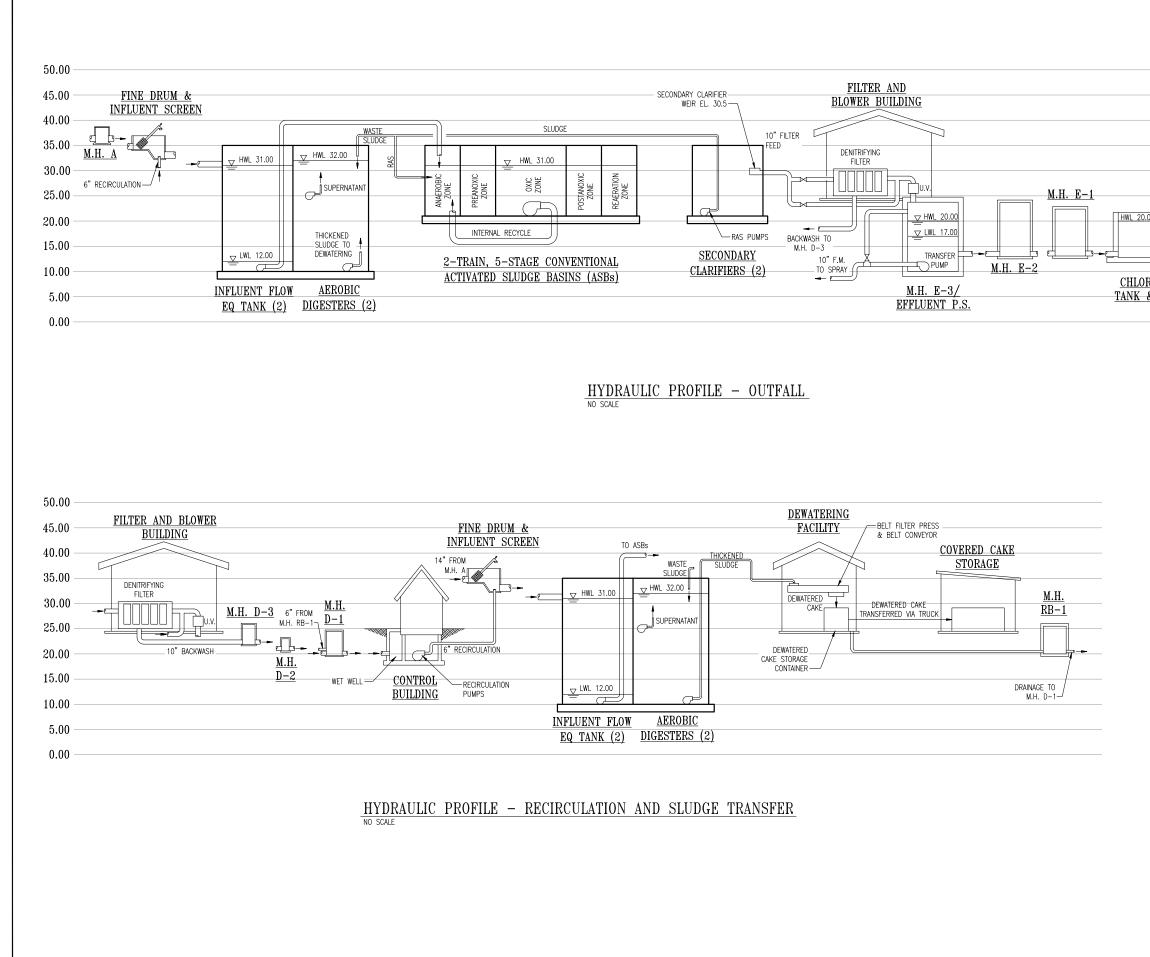


Appendix E

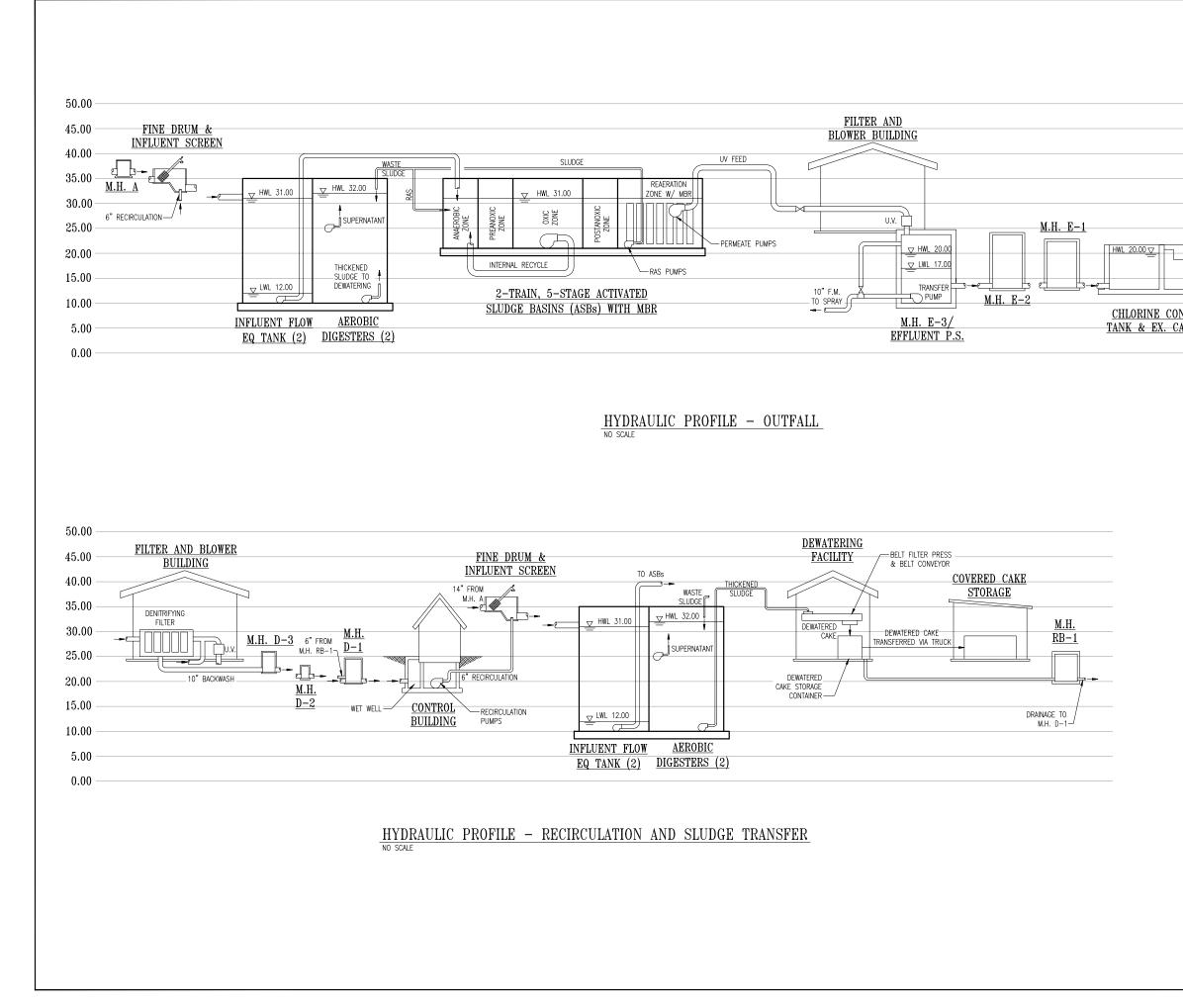
# Preliminary Hydraulic Profiles



	CLIENT INFORMATION
	TOWN OF CENTERVILLE CENTERVILLE, MD
14" OUTFALL	WWTP UPGRADE
E CONTACT	KEY PLAN
X. CASCADE	KEY PLAN
	GRAPHIC SCALES
	SIGNATURE
- BELT FILTER PRESS & BELT CONVEYOR	
COVERED CAKE	PRELIMINARY DESIGN
<u>STORAGE</u>	NOT FOR
	CONSTRUCTION
M.H.	
RB-1	PROFESSIONAL CERTIFICATION: I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND
	THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF
	MARYLAND,
DEWATERED CAKE 6" DRAINAGE	LICENSE NO EXPIRATION DATE:
TRANSFERRED VIA TO M.H. D-1	_
	Whitman, Requardt & Associates, LLP
	801 South Caroline Street, Baltimore, Maryland 21231
	[]
	ALTERNATIVE 1 HYDRAULIC PROFILE
	FIGURE NO.
	SCALE: NONE
	DATE: DECEMBER 2023 SHEET XX OF XX
	DES: KWS DRAWN: KWS CHECK: DRN



	CLIENT INFORMATION
	TOWN OF CENTERVILLE
	CENTERVILLE, MD
14" OUTFALL	
	WWTP UPGRADE
RINE CONTACT & EX. CASCADE	
<u>K EA. CASCADE</u>	KEY PLAN
	GRAPHIC SCALES
	SIGNATURE
	SIGNATORE
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	PRELIMINARY DESIGN
	NOT FOR
	CONSTRUCTION
	PROFESSIONAL CERTIFICATION:
	I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND
	THAT I AM A DULY LICENSED PROFESSIONAL
	ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND,
	LICENSE NO EXPIRATION DATE:
	Whitman, Requardt & Associates, LLP
	801 South Caroline Street, Baltimore, Maryland 21231
	ALTERNATIVE 2 HYDRAULIC PROFILE
	FIGURE NO.
	9
	$\sim$
	SCALE: NONE
	SCALE: NONE DATE: DECEMBER 2023 SHEET XX OF XX
	DATE: DECEMBER 2023 SHEET XX OF XX DES: LMA DRAWN: LMA CHECK: DRN
	DEG. MAR DIGHT, MAR OTEON, DRN



	CLIENT INFORMATION
	TOWN OF CENTERVILLE
	CENTERVILLE, MD
	CENTERVILLE, MD
14" OUTFALL	
	WWTP UPGRADE
<u>VTACT</u> ASCADE	
	KEY PLAN
	GRAPHIC SCALES
	SIGNATURE
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	PRELIMINARY DESIGN
	NOT FOR
	CONSTRUCTION
	PROFESSIONAL CERTIFICATION:
	I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND
	THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF
	MARYLAND, LICENSE NO.
	EXPIRATION DATE:
	Whitman, Requardt & Associates, LLP 801 South Caroline Street, Baltimore, Maryland 21231
	ALTERNATIVE 3
	HYDRAULIC PROFILE
	FIGURE NO.
	J J
	SCALE: NONE
	DATE: DECEMBER 2023 SHEET XX OF XX
	DES: LMA DRAWN: LMA CHECK: DRN



Appendix F

## Electrical Service Sizing and Single-Line Diagrams

	E	LECTRICAL SERVICE SIZING (EXIS	TING)			DATE:	November	27, 2023	
PROJECT:		Centerville WWTP							
LOCATION	:	Queen Anne's County				DESIGNE	DBY: SG	CHECKE	DBY: KK
ARCHITECT	<b>F ENGINEE</b>	R: Dhillon Engineering, Inc.							
Bldg.	S. NO.	DESCRIPTION	HP	KW	KVA	VOLTAGE	PHASE	AMPS	REMARKS
	1	SBR Sludge Transfer Pump #1	2		2.8	460	3	3.4	
	2	DDM Mixer #1	20		21.6	460	3	27	
	3	Exhaust Fan EF-1	7.5		9.2	460	3	3	
	4	Filtered Feed Pump #1	7.5		9.2	480	3	11	
	5	SBR Blower #1	50		54.1	480	3	65	
	6	Digester Aeration #1	30		31.9	460	3	40	
	7	SBR Sludge Transfer Pump #2	2		2.8	460	3	3.4	
Filter	8	DDM Mixer #2	20		21.6	460	3	27	
	9	Filtered Feed Pump #2	7.5		9.2	480	3	11	
Building	10	Exhaust Fan EF-2	0.5		0.9	460	3	1.1	
	11	Digester DDM Mixer	10		11.2	460	3	14	
	12	Digester Supernate Pump	3		3.9	460	3	4.8	
	13	SBR Blower #2	50		54.1	480	3	65	
	14	Digester Sludge Pump	2		2.9	480	3	3.4	
	15	SBR Blower #3	50		54.1	480	3	65	
	16	Digester Aeration #2	30		31.9	460	3	40	
	17	Panelboard DP			116	480	3		
Lab									
Building	1	Panelboard PC			25	208	3		
Control									
Bldg	1	Panelboard PD			20	208	3	77	
		Total kVA =			482.4				

Thus, the total connected load = 482 kVA

Based on the email correspondence received, the plant has seen a historical demand of 101 kW in January 2022. This corresponds to a demand of 126 kVA at 0.8 power factor.

We will assume this existing load for new service sizing calculations for all the proposed options.

Amps at 480V =	$\frac{475.6}{1.732 \ x \ 0.48} =$	580.25	Amps
Service Size =	572.07 x 1.25	=	725.32 Amps

	ELECT	RICAL SERVICE SIZING (ALTERN	IATIVE 1	)		DATE:	November	27, 2023		
PROJECT:		Centerville WWTP								
LOCATION	:	Queen Anne's County				DESIGNED BY: SG CHE			CKED BY: KK	
ARCHITEC	T ENGINEER:	Dhillon Engineering, Inc.								
Bldg.	S. NO.	DESCRIPTION	HP	KW	KVA	VOLTAGE	PHASE	AMPS	REMARKS	
	1	Existing Loads			126	480	3			
	2	SBR Blower #4	50		51.8	460	3	65		
	3	SBR Blower #5	50		51.8	460	3	65		
	4	Non-Potable Pump #1	20		22.5	480	3	27		
	5	Non-Potable Pump #2	20		22.5	480	3	27		
	6	Influent Screen	10		11.2	460	3	14		
	7	DNF Air Compressor #1	25		27.1	460	3	34		
	8	DNF Air Compressor #2	25		27.1	460	3	34		
	9	UV System			16	480	3			
	10	Digester Aeration #1	20		22.5	480	3	27		
	11	Digester Aeration #2	20		22.5	480	3	27		
	12	Digester Aeration #3	20		22.5	480	3	27		
	13	Dewatering Facility		20	25	460	3			
	14	Effluent Pump #1	100		98.8	460	3	124		
	15	Effluent Pump #2	100		98.8	460	3	124		
	16	Effluent Pump #3 (Standby)	100			460	3			
	17	HVAC Loads		10	12.5	480	3			
	18	Lighting		5	5	120	3			
		Total kVA =			663.6				<u> </u>	

Amps at 480V =

663

= 1.732 x 0.48

798.21 Amps

Taking 25% spare capacity and contingency and assuming a demand factor of 0.5, we get demand amps 498.88 Amps =

Service Size = 498.88 x 1.25 = 623.60 Amps

Thus, the existing service size is sufficient enough to handle this proposed option.

Taking 90% max loading and 0.8 power factor on a	a 500 kW g	enerator, we g	et availa	ble
capacity on the generator	=	500*0.9/0.8	=	562 kVA
			=	676 Amps

The existing 500 kW generator is also adequate to handle the proposed loads.

Centerville WWTP Queen Anne's County ER: Dhillon Engineering, Inc. DESCRIPTION Existing Loads Aeration Blower #1 Aeration Blower #2 Non-Potable Pump #1	HP 50	KW	KVA	DESIGNE	DBY: SG	СНЕСКЕ	<b>D BY:</b> KK
ER: Dhillon Engineering, Inc. DESCRIPTION Existing Loads Aeration Blower #1 Aeration Blower #2	HP	KW	KVA			CHECKE	DBY: KK
DESCRIPTION         Existing Loads         Aeration Blower #1         Aeration Blower #2	HP	KW	KVA	VOLTAGE	<b></b>		
Existing Loads Aeration Blower #1 Aeration Blower #2		KW	KVA	VOLTAGE			
Aeration Blower #1 Aeration Blower #2	50				PHASE	AMPS	REMARKS
Aeration Blower #2	50		126	480	3		
	1		51.8	460	3	65	
Non-Potable Pump #1	50		51.8	460	3	65	
	20		22.5	480	3	27	
Non-Potable Pump #2	20		22.5	480	3	27	
Digester Aeration #1	20		22.5	480	3	27	
Digester Aeration #2	20		22.5	480	3	27	
Digester Aeration #3	20		22.5	480	3	27	
Digester Aeration #4	20		22.5	480	3	27	
Digester Aeration #5	20		22.5	480	3	27	
Digester Aeration #6	20		22.5	480	3	27	
Anoxic Zone Mixers	5		6.1	460	3	7.6	
Internal Recycle Pump #1	10		11.2	460	3	14	
Internal Recycle Pump #2	10		11.2	460	3	14	
Return Sludge Pump #1	10		11.2	460	3	14	
Return Sludge Pump #2	10		11.2	460	3	14	
Influent Screen	10		11.2	460	3	14	
UV System			16	480	3		
Dewatering Facility		20	25	460	3		
DNF Air Compressor #1	25		27.1	460	3	34	
DNF Air Compressor #2	25		27.1	460	3	34	
Effluent Pump #1	100		98.8	460	3	124	
Effluent Pump #2	100		98.8	460	3	124	
Effluent Pump #3 (Standby)	101			460	3		
HVAC Loads		10	12.5	480	3		
Lighting		5	5	120	3		
 Total kVA =			782			<u> </u>	<u> </u>
-	DNF Air Compressor #2 Effluent Pump #1 Effluent Pump #2 Effluent Pump #3 (Standby) HVAC Loads Lighting	DNF Air Compressor #225Effluent Pump #1100Effluent Pump #2100Effluent Pump #3 (Standby)101HVAC Loads101	DNF Air Compressor #225Effluent Pump #1100Effluent Pump #2100Effluent Pump #3 (Standby)101HVAC Loads10Lighting5	DNF Air Compressor #2         25         27.1           Effluent Pump #1         100         98.8           Effluent Pump #2         100         98.8           Effluent Pump #3 (Standby)         101         10           HVAC Loads         10         12.5           Lighting         5         5	DNF Air Compressor #2       25       27.1       460         Effluent Pump #1       100       98.8       460         Effluent Pump #2       100       98.8       460         Effluent Pump #3 (Standby)       101       460         HVAC Loads       10       12.5       480         Lighting       5       5       120	DNF Air Compressor #2       25       27.1       460       3         Effluent Pump #1       100       98.8       460       3         Effluent Pump #2       100       98.8       460       3         Effluent Pump #3 (Standby)       101       460       3         HVAC Loads       10       12.5       480       3         Lighting       5       5       120       3	DNF Air Compressor #2       25       27.1       460       3       34         Effluent Pump #1       100       98.8       460       3       124         Effluent Pump #2       100       98.8       460       3       124         Effluent Pump #3 (Standby)       101       460       3       124         HVAC Loads       10       12.5       480       3       10         Lighting       5       5       120       3       10

Amps at 480V =

 $\frac{782}{1.732 \times 0.48}$ 

782 = 940.63 Amps

Taking 25% spare capacity and contingency and assuming a demand factor of 0.5,we get demand amps=587.89 Amps

Service Size = 587.89 x 1.25 = 734.87 Amps

Thus, the existing service size is sufficient enough to handle this proposed option.

Taking 90% max loading and 0.8 power factor on	a 500 kW	/ generator, we get availa	able
capacity on the generator	=	500*0.9/0.8 =	562 kVA
		=	676 Amps

The existing 500 kW generator is also adequate to handle the proposed loads.

ELECTRICAL SERVICE SIZING (ALTERNATIVE 3)				DATE:	November	27, 2023				
PROJECT:		Centerville WWTP								
LOCATION	l:	Queen Anne's County				DESIGNE	DBY: SG	CHECKE	DBY: KK	
ARCHITEC	T ENGINEER:	: Dhillon Engineering, Inc.								
Bldg.	S. NO.	DESCRIPTION	HP	KW	KVA	VOLTAGE	PHASE	AMPS	REMARKS	
	1	Existing Loads			126	480	3			
	2	Aeration Blower #1	50		51.8	460	3	65		
	3	Aeration Blower #2	50		51.8	460	3	65		
	4	Non-Potable Pump #1	20		22.5	480	3	27		
	5	Non-Potable Pump #2	20		22.5	480	3	27		
	6	Digester Aeration #1	20		22.5	480	3	27		
	7	Digester Aeration #2	20		22.5	480	3	27		
	8	Digester Aeration #3	20		22.5	480	3	27		
	9	Digester Aeration #4	20		22.5	480	3	27		
	10	Digester Aeration #5	20		22.5	480	3	27		
	11	Digester Aeration #6	20		22.5	480	3	27		
	12	Anoxic Zone Mixers	5		6.1	460	3	7.6		
	13	Return Sludge Pump #1	10		11.2	460	3	14		
	14	Return Sludge Pump #2	10		11.2	460	3	14		
	15	Influent Screen	10		11.2	460	3	14		
	16	UV System			16	480	3			
	17	Dewatering Facility		20	25	460	3			
	18	DNF Air Compressor #1	25		27.1	460	3	34		
	19	DNF Air Compressor #2	25		27.1	460	3	34		
	20	Effluent Pump #1	100		98.8	460	3	124		
	21	Effluent Pump #2	100		98.8	460	3	124		
	22	Effluent Pump #3 (Standby)	101			460	3			
	23	Influent Pump #1	25		27.1	460	3	34		
	24	Influent Pump #2	25		27.1	460	3	34		
	25	Permeate Pump #1	25		27.1	460	3	34		
	26	HVAC Loads		10	12.5	480	3			
	27	Lighting		5	5	120	3			
L		<b>T</b> .1.1114			040.0					
		Total kVA =			840.9					

Amps at 480V =

840.9 =

1011.48 Amps

1.732 x 0.48

Taking 25% spare capacity and contingency and assuming a demand factor of 0.5, we get demand amps 632.17 Amps =

Service Size =

632.17 x 1.25 =

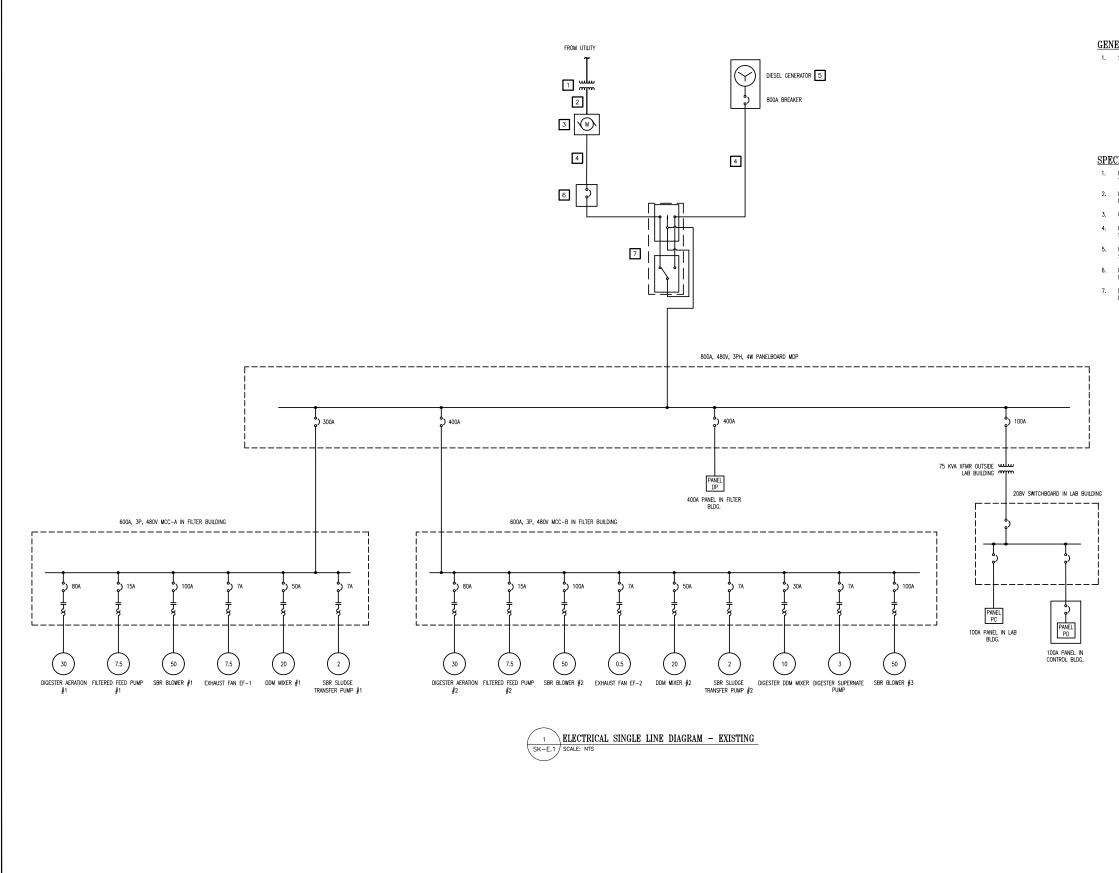
790.21 Amps

Thus, the existing service size is sufficient enough to handle this proposed option.

Taking 90% max loading and 0.8 power factor on	n a 500 kW	V generator, we ge	et availa	able
capacity on the generator	=	500*0.9/0.8	=	562 kVA
			=	676 Amps

ELECTRICAL SERVICE SIZING (ALTERNATIVE 3)			DATE:	November	27, 2023			
PROJECT:	Centerville WWTP							
LOCATION:	Queen Anne's County				DESIGNED BY: SG CHECKED BY			DBY: KK
ARCHITECT ENGINEER	Dhillon Engineering, Inc.							
Bldg. S. NO.	DESCRIPTION	HP	KW	KVA	VOLTAGE	PHASE	AMPS	REMARKS

The existing 500 kW generator is also adequate to handle the proposed loads.



ZNPRDJECTSNWRANG TOWN OF CENTERVILLE WATP ENR UPGRADENOS DRAVINGSNPRODUCTION/CENTERVILLE WATP EXISTING DNE L

REVISIONS

#### GENERAL SHEET NOTES:

1. SPARES AND SPACES ARE NOT SHOWN ON THIS SKETCH.

#### <u>SPECIFIC NOTES $\times$ :</u>

1. PAD MOUNTED DELMERVA POWER COMPANY EXISTING MEDIUM VOLTAGE TRANSFORMER.

 EXISTING UNDERGROUND DUCTBANK FOR UTILITY COMPANY PRIMARY FEEDER.
 UTILITY COMPANY METERING.

 EXISTING UNDERGROUND DUCTBANK FOR UTILITY COMPANY SECONDARY FEEDER.

5. EXISTING 500KW, 480/277V, 3PH, 4W DIESEL GENERATOR WITH SUB BASE TANK IN OUTDOOR WEATHERPROOF ENCLOSURE.

 EXISTING 800A, 480V, 3P ENCLOSED CIRCUIT BREAKER IN NEMA-1 ENCLOSURE, SUITABLE FOR SERVICE.

 EXISTING 800A, 480V, 3P AUTOMATIC TRANSFER SWITCH WITH BYPASS ISOLATION SWITCH IN NEMA-1 ENCLOSURE. CLIENT INFORMATION

## TOWN OF CENTERVILLE CENTERVILLE

## XXXX

KEY PLAN

GRAPHIC SCALES

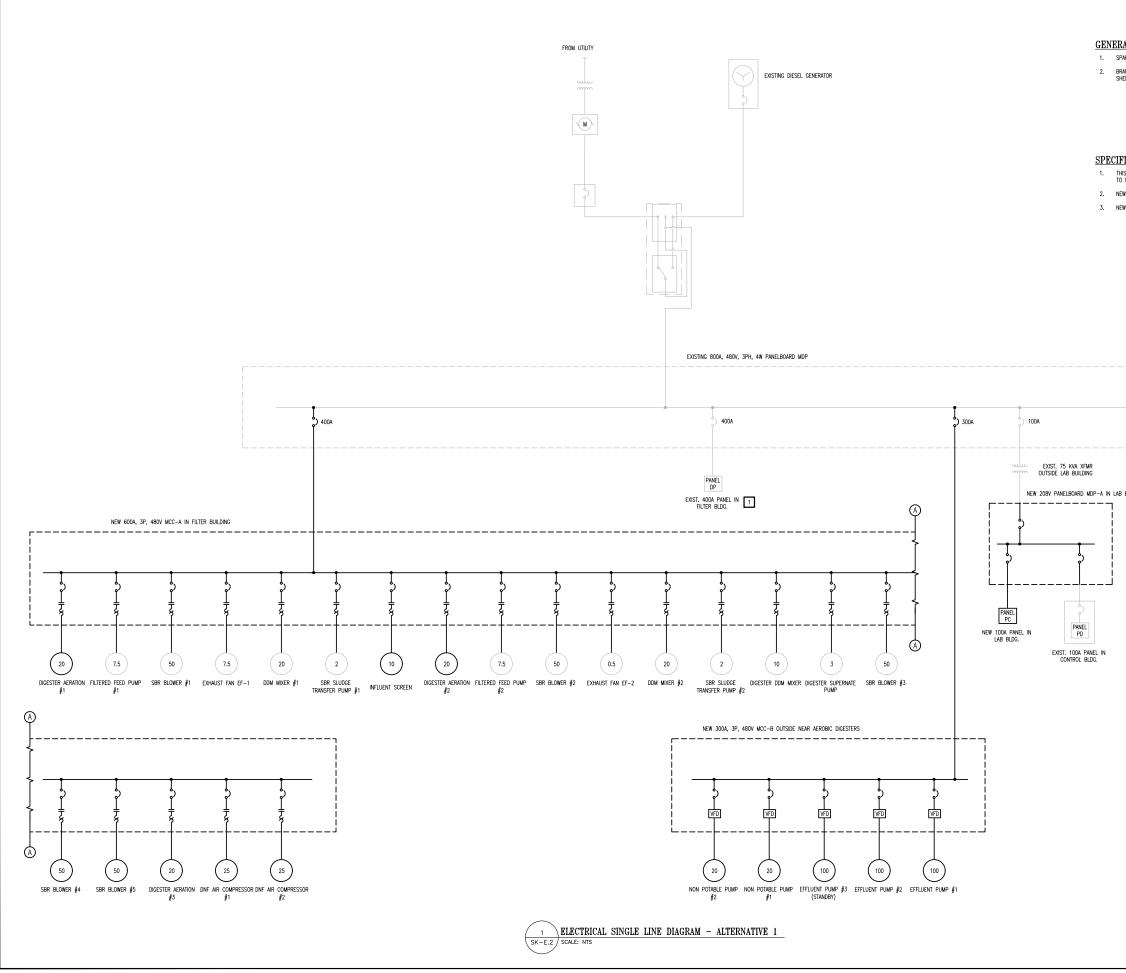
SIGNATURE



Whitman, Requardt & Associates, LLP 1500 Market Street, Suite W2750, Philadelphia, Pennsylvania 19102

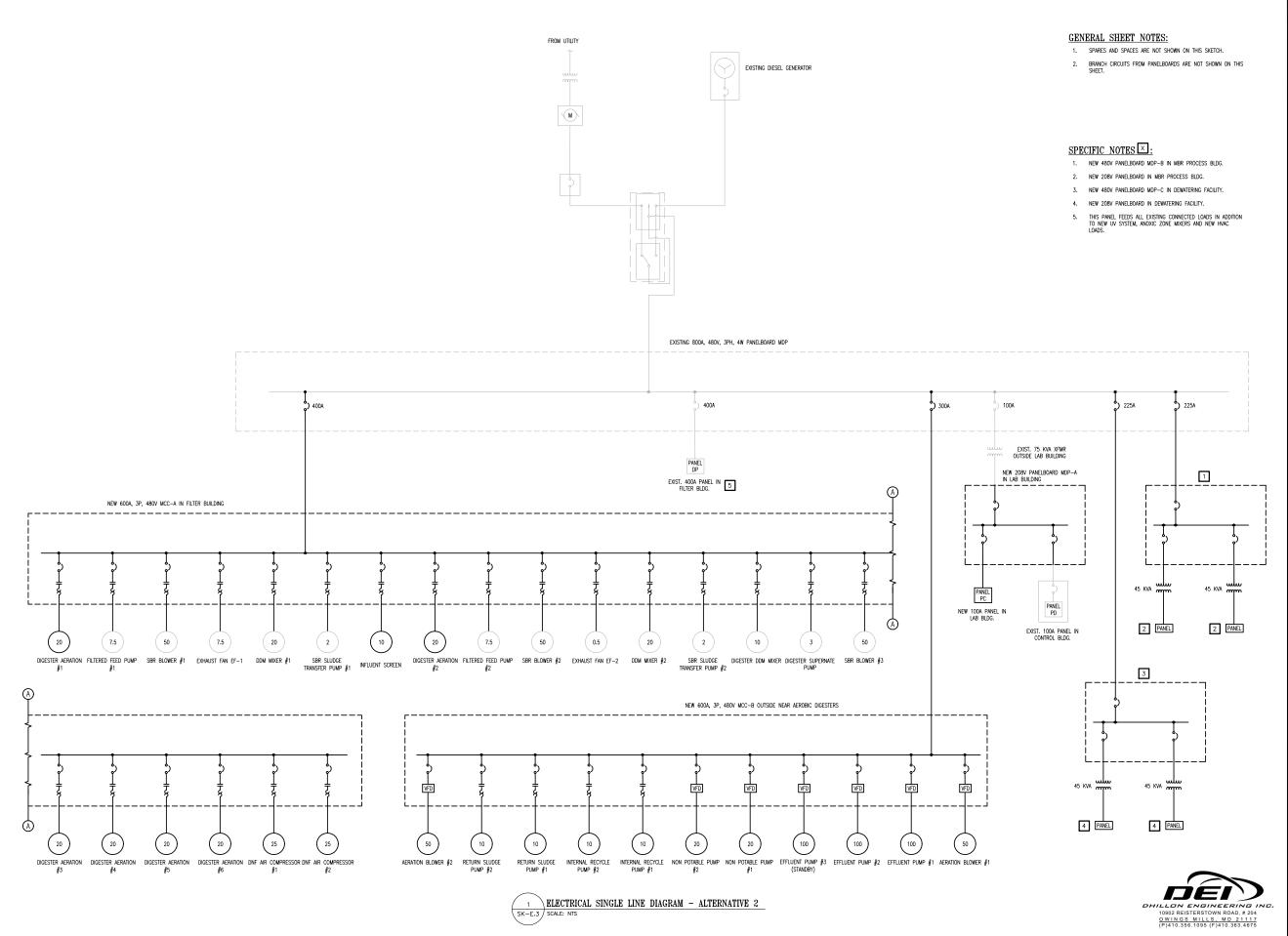
SINGLE LINE DIAGRAM EXISTING CONDITIONS				
DRAWING NO.				
SK-E.1				
SCALE: NONE				
DATE: NOV 2023		SHEET	0F	
DES: SG	DRAWN:	SG	CHECK:	KK
			PER	REPORT





CRAL SHEET NOTES:	
SPARES AND SPACES ARE NOT SHOWN ON THIS SKETCH. BRANCH CIRCUITS FROM PANELBOARDS ARE NOT SHOWN ON THIS SHEET.	CLIENT INFORMATION
IFIC NOTES ⊠:	TOWN OF CENTERVILLE CENTERVILLE
THIS PANEL FEEDS ALL EXISTING CONNECTED LOADS IN ADDITION TO NEW UV SYSTEM AND NEW HVAC LOADS.	
NEW 480V PANELBOARD MDP-C IN DEWATERING FACILITY.	
NEW 208V PANELBOARD IN DEWATERING FACILITY.	
	XXXX
	KEY PLAN
2254	GRAPHIC SCALES
<b>2</b> <i>L L n</i>	UVAT HIC SCALLS
	SIGNATURE
i l i	
ulu ulu	
45 KVA mm 45 KVA mm	
3 PANEL 3 PANEL	
	Whitman, Requardt & Associates, LLP 1500 Market Street, Suite W2750, Philadelphia, Pennsylvania 19102
	SINGLE LINE DIAGRAM
	ALTERNATIVE 1
	SK-E.2
	SCALE: NONE
10902 REISTERSTOWN ROAD, # 204 O WIN GS MILLS, MD 21117 (P)410.356.1095 (F)410.363.4675	DATE: NOV 2023 SHEET OF
	DES: SG DRAWN: SG CHECK: KK

PER REPORT



REVISIONS	

CLIENT INFORMATION

## TOWN OF CENTERVILLE CENTERVILLE

### XXXX

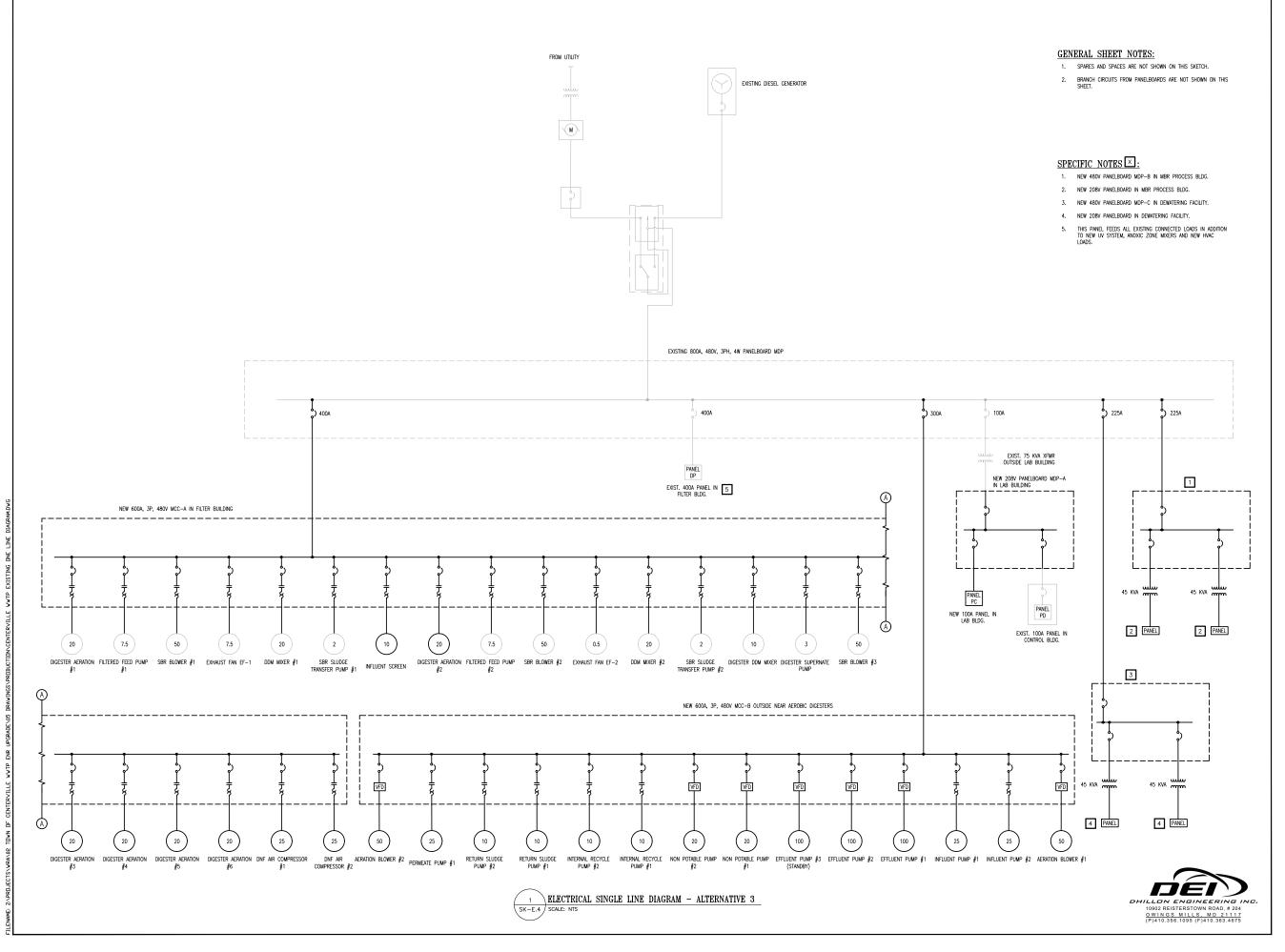
KEY PLAN

GRAPHIC SCALES

SIGNATURE



SINGLE LINE DIAGRAM ALTERNATIVE 2				
DRAWING NO.				
SK-E.3				
SCALE: NONE				
DATE: NOV 2023		SHEET	OF	
DES: SG	DRAWN:	SG	CHECK:	KK
			PER	REPORT



CLIENT	INFORMATION

## TOWN OF CENTERVILLE CENTERVILLE

#### XXXX

KEY PLAN

GRAPHIC SCALES

SIGNATURE



Wnitman, Requarat & ASSOCIATES, LLP 1500 Market Street, Suite W2750, Philadelphia, Pennsylvania 19102

SINGLE LINE DIAGRAM ALTERNATIVE 3				
DRAWING NO.				
SK-E.4				
SCALE: NONE				
DATE: NOV 2023		SHEET	OF	
DES: SG	DRAWN:	SG	CHECK:	KK
			PER	REPORT

